

Improving Concrete Overlay Construction

Final Report
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16. Abstract Several road construction projects involving concrete overlays at the state and county levels in Iowa in 2009 were studied for construction techniques and methods. The projects that were evaluated consisted of sites in four Iowa counties: Osceola, Worth, Poweshiek, and Johnson counties. The construction techniques and methods that were studied included concrete overlays and material usage. By evaluating these methods, highway agencies can explore different ways of making road construction less costly and can minimize the amount of time that the traveling public is exposed to road construction.					
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IMPROVING CONCRETE OVERLAY CONSTRUCTION

**Final Report
March 2010**

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TABLE OF CONTENTS

ACKNOWLEDGMENTS	XI
INTRODUCTION	1
Background.....	1
RESEARCH OBJECTIVES	2
Research Program Activity #1	2
Research Program Activity #2	2
Research Program Activity #3	2
Research Program Activity #4	2
Research Program Activity #5	2
Research Program Activity #6	2
Research Program Activity #7	3
Technical Advisory Committee	3
RESEARCH PLAN	4
Project Selection	4
RESEARCH GOAL RELATIONSHIP	8
Research Program Activity #1	8
Research Program Activity #2	10
Research Program Activity #3	13
Research Program Activity #4	14
Research Program Activity #5	15
Research Program Activity #6	17
Research Program Activity #7	18
Added Research Program Activities	20
OVERLAY CONSTRUCTION.....	21
Iowa Highway 9 in Osceola County	21
US 65 in Worth County	24
V-18 in Poweshiek County	31
County Road W-62 in Johnson County	40
DATA COLLECTION	45
Concrete Strength.....	45
Weather Data	46
Deflection Data	46
Roadway Surface Mapping.....	46
DATA ANALYSIS.....	50
Roadway Surface Mapping.....	50
Concrete Quantity Development.....	52
Geotextile Bond Breaker:	53
Maturity Values	54

Traffic Control Methods and Results.....	56
Deflection Data Analysis	57
Environmental Relationships	62
Knife Application Data	62
RESEARCH RESULTS	65
GPS-Controlled Longitudinal Joint Formation.....	65
GPS Pavement Surface Mapping: Pros, Cons, and Limitations	67
Slipform Paver Machine Control	67
Overlay and Stringless Paving Demonstration Openhouse	69
Geotextile Bond Breaker Placement.....	70
Concrete Opening Strength Requirement for Local Traffic Use	71
Traffic Control for One- and Two-Lane Overlay Construction.....	71
Deflection Data Analysis Results	72
Overlay Construction Operation Timing	72
Environmental Relationships	74
Knife Joint Former Results	74
RESEARCH CONCLUSIONS.....	75
Longitudinal Joint Formation	75
GPS Mapping of Pavement Surfaces for Concrete Surface Profile and Quantity Calculations.....	75
Milling.....	75
Slipform Paver Machine Control	75
Geotextile Bond Breakers	76
Concrete Opening Strength Requirement for Local Traffic Use	77
Traffic Control for One- and Two-Lane Overlay Construction.....	77
Overlay Construction Operation Timing	77
FWD Testing.....	78
RECOMMENDATIONS.....	79
Longitudinal Joint Formation with GPS-Controlled Saws	79
GPS Pavement Surface Mapping.....	79
Milling.....	79
Slipform Paver Machine Control	80
Geotextile Bond Breaker.....	80
Concrete Opening Strength Requirement for Local Traffic Use	80
Traffic Control for One- and Two-Lane Overlay Construction.....	81
Overlay Construction Operation Timing	81
FWD Testing.....	81
FUTURE RESEARCH	82
Longitudinal Joint Formation for Bonded Overlays.....	82
Surface Mapping.....	82
Surface Milling	82
Machine Control	82
Opening Strength	82

REFERENCES:	83
APPENDIX A. GEOTEXTILE SPECIFICATIONS AND FASTENING PLAN	A-1
Fabric Interlayer Construction Practices.....	A-2
APPENDIX B. JOHNSON COUNTY CROSS-SECTION PLANS.....	B-1
APPENDIX C. CONCRETE CORES	C-1
APPENDIX D. MATURITY TTF STATISTICS	D-1
Osceola County.....	D-1
Worth County.....	D-5
Poweshiek County	D-11
Johnson County.....	D-13
APPENDIX E. FWD DATA	E-1
APPENDIX F. KNIFE PHOTOS AND PRELIMINARY DESIGN.....	F-1

LIST OF FIGURES

Figure 1. Osceola County typical construction cross-section and overlay	5
Figure 2. Worth County typical construction overlay cross-section.....	6
Figure 3. Worth County overlay jointing pattern.....	6
Figure 4. Poweshiek County typical overlay construction cross-section	7
Figure 5. Johnson County typical overlay construction cross-section.....	7
Figure 6. Osceola County paving train	21
Figure 7. Osceola County tie bars.....	22
Figure 8. Osceola County finished pavement.....	22
Figure 9. Osceola County original pavement	23
Figure 10. Mainline paving.....	25
Figure 11. Worth County finished pavement.....	25
Figure 12. Worth County bar layout.....	26
Figure 13. Worth County before milling	28
Figure 14. Worth County existing condition after milling	28
Figure 15. A 12-foot-wide milling machine	29
Figure 16. Hand tooling of the center line	30
Figure 17. Gomaco 2800 Commander slipform paver	32
Figure 18. Gomaco GSI profiler	33
Figure 19. Poweshiek final slab.....	33
Figure 20. The gap in the HMA bond breaker where the geotextile material was placed.....	34
Figure 21. Fabric roll placement.....	35
Figure 22. Nailing pattern for the geotextile materials to the existing pavement.....	35
Figure 23. Unrolling and nailing the material to the existing pavement	36
Figure 24. Streetpads	37
Figure 25. Non fabric-covered lane	37
Figure 26. Fabric and concrete placement.....	38
Figure 27. Poweshiek existing pavement.....	39
Figure 28. Rehabilitation technique for damaged transverse joints.....	39
Figure 29. The project paving train.....	41
Figure 30. Johnson County finished pavement.....	42
Figure 31. Johnson County existing condition	43
Figure 32. A John Deere Gator equipped with a GPS receiver	47
Figure 33. GPS receiver and laser profiler located on the top of the John Deere Gator.....	47
Figure 34. An ATV equipped with a GPS	49
Figure 35. “Knife” mounted on the bottom of the slipform paver pan.....	63
Figure 36. “Knife” held in place with clamps.....	63
Figure 37. The “knife”	64
Figure 38. Saw equipped with a GPS unit	65
Figure 39. GPS receiver and screen.....	66
Figure 40. Concrete cores	70

LIST OF TABLES

Table 1. 0.1 mile pre/post Do comparison (deflection in mils)	58
Table 2. 0.2 mile pre/post Do comparison (deflection in mils)	59
Table 3. 0.5 mile pre/post Do comparison (deflection in mils)	60
Table 4. 1.0 mile pre/post Do comparison (deflection in mils)	61

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INTRODUCTION

Background

As our highway system ages and available funding shrinks, agencies are looking for efficient methods to preserve and extend the service life of existing pavements. In addition, the highway agencies are being asked to minimize the time of exposure to construction and inconvenience to the traveling public.

The movement to preservation activities calls for the use of overlays to strengthen the pavement and/or add a better driving surface. High oil prices have brought highway designers to look at concrete overlays as a viable alternative. Construction techniques for concrete overlays need to be improved in order to provide efficient answers to the preservation problems. Critical construction elements include developing means to address concerns about concrete yield, minimizing pavement train width, traffic control, opening-strength staging to meet traffic and concrete delivery demands, including cost-effective approaches, and surface preparation for the existing pavement. In addition, the use of innovative materials as bond breakers on unbonded overlays will be considered.

Despite the completion of hundreds of concrete overlay projects, some highway agencies are reluctant to use concrete overlays. Some agencies believe that concrete overlays are expensive, difficult to build and remove, and only to be used in limited applications. Because of these beliefs, it is important that efficient construction methods be developed to meet the public's need for mobility, safety, and access to property.

The Iowa Department of Transportation (Iowa DOT) and Iowa counties and cities have successfully completed a multitude of projects that range from thin bonded overlays (i.e., 2–4 inches) to thicker unbonded overlays (greater than 4 inches). About 1,000 miles of overlay are in use throughout Iowa.

RESEARCH OBJECTIVES

Research Program Activity #1

- Evaluate machine control systems to minimize paving train width
- Reduce quantity overrun concerns with global positioning system (GPS) mapping of the proposed project
- Reduce construction survey time with GPS mapping and evaluate GPS and 3D construction equipment control (e.g., milling machine, slipform paver, and cure cart)
- Develop ways to establish the profile grades and machine control before or immediately after the contract letting by the highway agency so that construction is not impacted

Research Program Activity #2

- Develop innovative ways to guide the longitudinal joint forming operation to match the underlying joint alignment
- Evaluate the use of GPS to control longitudinal joint sawing

Research Program Activity #3

- Determine the best way to establish the level of need and timing of milling for existing asphalt surface preparation
- Evaluate milling by the standard practice of stringline control and by GPS control

Research Program Activity #4

- Evaluate the use of innovative materials, such as geotextile layers, for use as bond separator layers

Research Program Activity #5

- Determine innovative ways of handling traffic control for the construction of single-lane overlays as part of a multi-lane overlay
- Evaluate the impact of haul road selection on road opening time

Research Program Activity #6

- Investigate potential solutions of using both existing and new paving train components to minimize the time of construction operation

Research Program Activity #7

- Determine the appropriate opening strength that is required of the concrete for local traffic use and through trucks and construction traffic, for depths of concrete of less than 6 inches.

In order to address the concerns noted above, in Iowa and across the country, a project was undertaken by the National Concrete Pavement Technology Center at Iowa State University to develop a guide document to assist engineers in concrete overlay design and construction. The original guide was published in January of 2007, and in September of 2008 the *Guide to Concrete Overlays* was completed.

Several more detailed elements related to concrete overlays are needed in the *Guide* to enhance concrete overlay design and construction and alleviate the concerns over the use of concrete overlays.

The research activities included in this project were established to address some of the gaps in the guide's information and to provide additional guidance to designers and contractors through additions to the guide. The overall goal is to make implementation of concrete overlay projects as straight forward and economical as possible.

Technical Advisory Committee

The research team identified a technical advisory committee (TAC) to provide review and guidance of the research from plan to execution in each of the construction projects. The advisory committee was made up of Brian Keierleber, Buchanan County Engineer; John Goode, Monroe County Engineer; Lyle Brehm, Tama-Poweshiek County Engineer; Chris Brakke, Iowa DOT Pavement Design Engineer; Kevin Merryman, Iowa DOT Portland Cement Concrete (PCC) Construction Engineer; Todd Hanson, Iowa DOT PCC Materials Engineer; Shane Tymkowicz, Iowa DOT Assistant District Engineer; Roy Gelhaus, Iowa DOT Resident Construction Engineer; and Gordon Smith, President of the ICPA.

The TAC was involved in the development of the various sections of the research proposal and met to discuss the details of the work plan prior to any of the construction. Members of the TAC visited some of the projects during construction. This group also provided a review of the materials placed in this report.

RESEARCH PLAN

Project Selection

The projects selected for this research were identified through a matrix process. The matrix consisted of first identifying the research objectives and comparing those to available projects that were slated for overlay (i.e., asphalt or portland cement concrete) construction in 2009. Projects were sought for consideration from both state and local road projects.

Final candidate projects were subjected to both visual reviews and record searches to determine their condition and to ensure that the project would provide a suitable base for a PCC overlay. State and local officials in charge of the rehabilitation decisions for each considered pavement were contacted to obtain their concurrence with the research that was being considered.

The first phase of the selection process was a visual evaluation of each pavement. Joints were assessed to ensure that transverse joints were turned down, indicating a relatively sound underlying joint with minor deterioration and only potential faulting.

Second, historical project files were used to determine the materials that were included in the existing layers to assess their long-term durability under an overlay. Projects with underlying pavements that exhibited materials-related distress were not considered to be a wise choice for thin PCC overlays. These materials would continue to show surface distress regardless of the choice of overlay materials. The materials are best rehabilitated through total reconstruction of the pavement structure.

Historical “as-built” plans and associated files were consulted to determine the materials used in each of the existing pavement layers and the type of binders and construction methods employed. One must understand the existing pavement characteristics prior to the design and construction of the new overlay for best performance results.

The selection process resulted in the identification of four active overlay projects that were already under development by the Iowa DOT and Osceola, Poweshiek Worth, and Johnson counties. The projects include the following:

Iowa Highway 9 in Osceola County—from relocated Iowa Highway 60 east 8.8 miles to the east junction of County Road L-58

The existing pavement consisted of a composite section of 4.5 inches of hot-mix asphalt (HMA) and an 18-foot-wide (10-7-10 section) PCC pavement with a 3-foot by 10-inch PCC widening unit. The Iowa DOT designed a 5.5-inch PCC overlay of the existing 24-foot-wide pavement and added a 9.5-inch-deep by 2-foot widening unit on each side of the existing pavement. The jointing of the overlay consisted of longitudinal joints at 4.5 and 9.0 feet from the centerline and transverse joints at 5 foot intervals. Reinforcement was to be placed over both widening joints and a single joint sawed at the original widening joint between the original 18-foot pavement and

the first 3-foot widening. The project was let in February 2009. The road was built under closed road conditions, with through traffic using a detour. The typical construction cross-section and overlay jointing notes for the Osceola County project are shown in Figure 1.

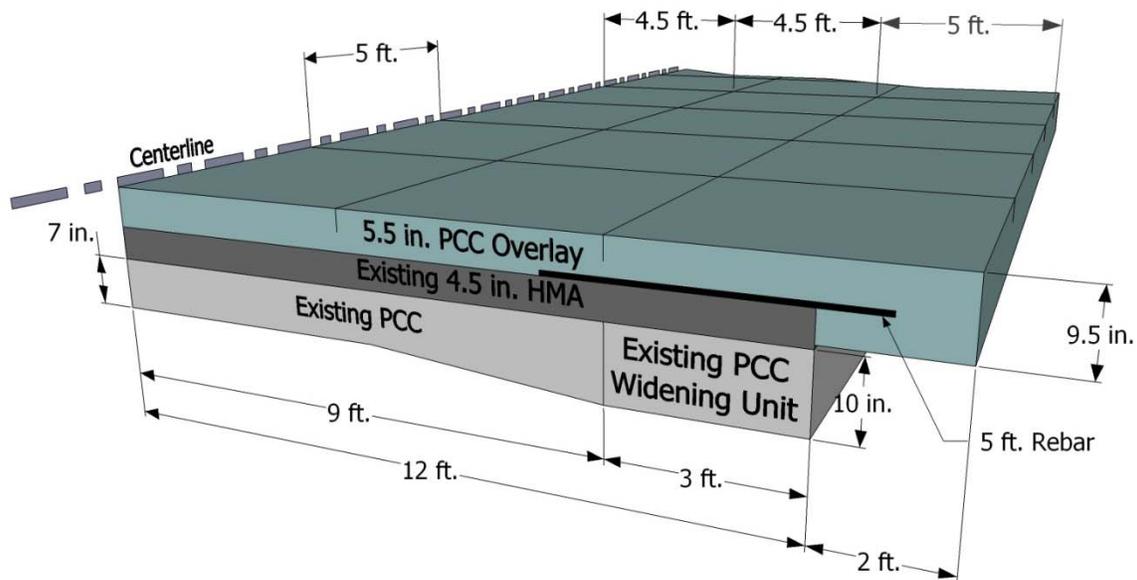


Figure 1. Osceola County typical construction cross-section and overlay

US 65 in Worth County—from Manly north 10.51 miles to the southern corporate limits of Northwood, Iowa

The existing pavement consisted of a composite section that had 5 inches of HMA over 7.5 inches PCC equivalent (10-7-10 section) with a 3-foot by 10-inch-deep PCC widening unit. The Iowa DOT designed a 5.0 inch PCC overlay of the existing 24-foot-wide pavement and the addition of an 8-inch deep by 4-foot widening unit on each side of the existing pavement. The jointing of the overlay consisted of longitudinal joints at 4.5 and 9.0 feet from the centerline, transverse joints at 5.0 foot intervals in the mainline, and 10 foot intervals in the turn lane areas. Reinforcement was to be placed over both widening joints. The project was let in January 2009. The road was to be built under closed road conditions with a detour for through traffic. The typical overlay construction cross-section and overlay jointing pattern for the Worth County project are shown in Figures 2 and 3. The length of tie-bar shown in Figure 1 was field-modified from 60 to 72 inches in length to allow for 18 inches of embedment into the new widening unit and onto the existing 18-foot pavement section.

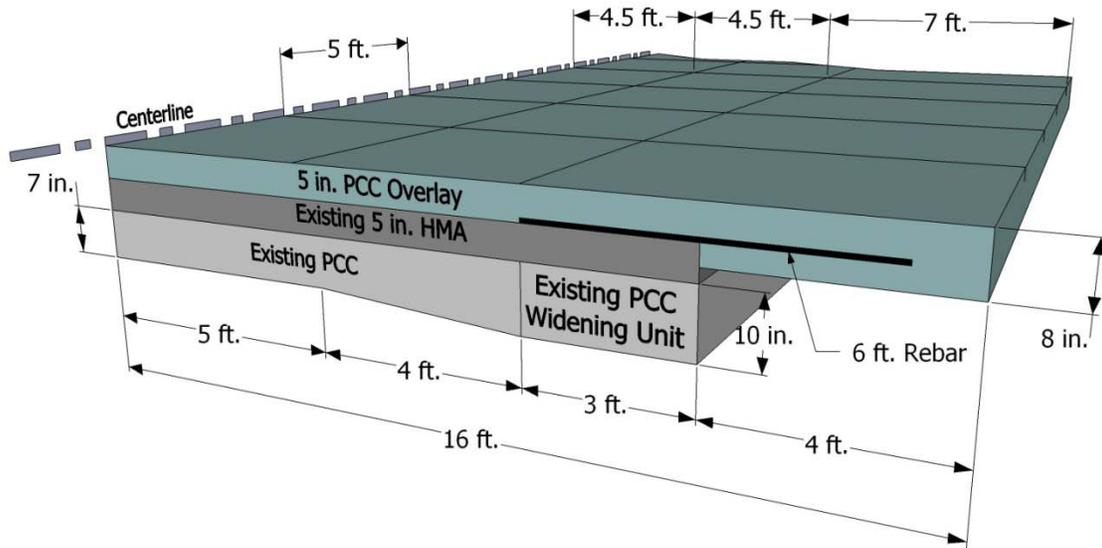


Figure 2. Worth County typical construction overlay cross-section

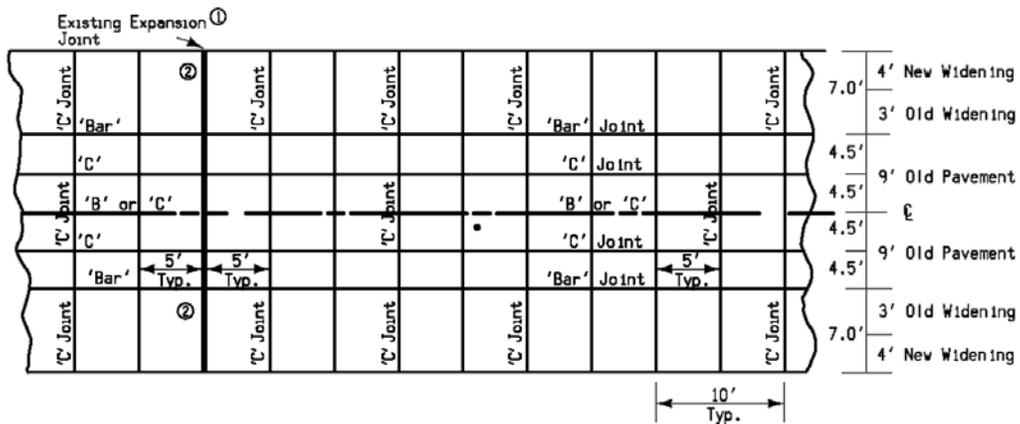


Figure 3. Worth County overlay jointing pattern

County Road V18 in Poweshiek County—from Iowa 85 north 9.58 miles to the southern corporate limits of Brooklyn, Iowa

The existing pavement was a 7-inch by 22-foot PCC pavement, built 30–35 years ago and was experiencing some joint deterioration. A 6-inch PCC unbonded overlay was planned for this project. Transverse joints were designed for 12-foot joint spacings. The project was let in November 2008. It was built with detour and full-width paving. The existing road was closed to through traffic, and a detour was used for through traffic. The typical overlay construction cross-section for the Poweshiek County project is shown in Figure 4.

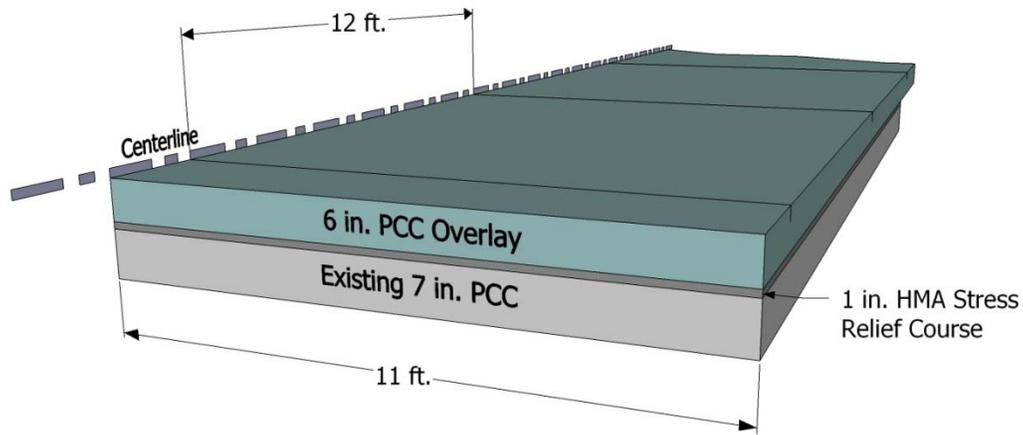


Figure 4. Poweshiek County typical overlay construction cross-section

County Road W-62 (Oak Crest Hill Road) in Johnson County—from a point one-half mile south of the town of Hills, north 4.69 miles to the Iowa Highway 921 connection with US 218 (Avenue of the Saints)

The existing pavement consisted of an 18-foot-wide, 1929 PCC pavement with lip curbs. The pavement was resurfaced with HMA to fill the height of the curbs and extend a 3-inch depth of HMA to a width of 24 feet. The county designed a 5.5-inch PCC overlay of the existing 18-foot pavement and an 8-inch by 8-foot PCC widening unit. The new pavement contained longitudinal joints 9 feet from the centerline and transverse joints at 12 foot intervals. All this construction was contained in an existing 66-foot right of way. The project was let in June of 2009. The roadway was closed to through traffic, and local traffic was allowed to use the second lane in only one direction. The typical overlay construction cross-section for the Johnson County project is shown in Figure 5.

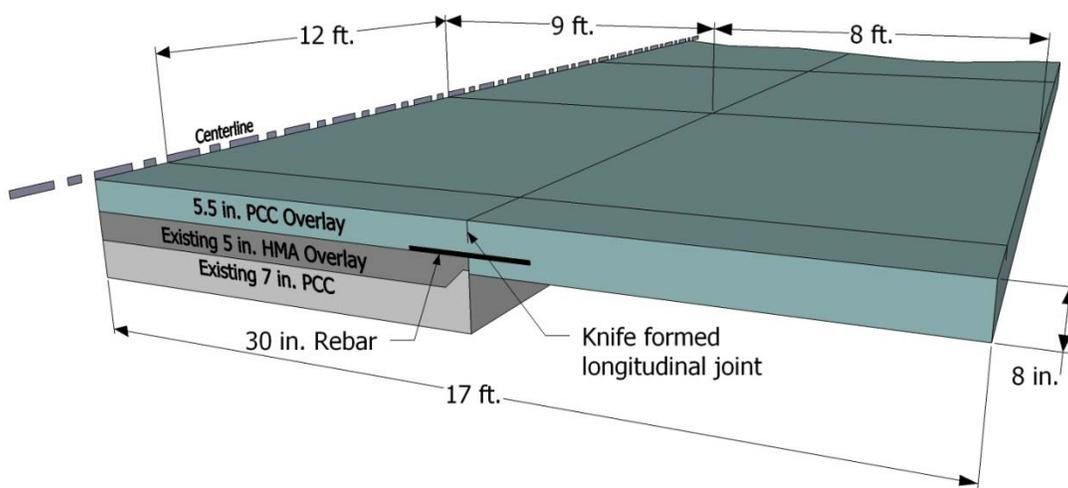


Figure 5. Johnson County typical overlay construction cross-section

RESEARCH GOAL RELATIONSHIP

It was not possible to attain results from each of the seven research objectives within these four projects. The project team was able to evaluate various parts of the individual goals as shown specifically below for the selected projects.

The workplan for each activity was broken into two parts: proposed and actual. The “proposed” workplan identifies what the research team originally set out to accomplish on each project. The projects only represent part of the larger national activity points. The “actual” workplan elements for each activity area represent what the research team was able to accomplish within the budget limits and through negotiation with the contracting agencies and contractors for each project. These items were not contract bid items and therefore could not be enforced by the research team for any of the projects. The project budget for maturity meters was based on two projects that were being simultaneously constructed and not three, as was true in 2009.

Research Program Activity #1

Two goals were to develop ways to establish the profile grades and machine control before or immediately after the contract letting from the highway agency and to evaluate machine control systems to minimize paving train width. The project team used the following techniques:

- Used GPS to plan the grades with various runs at edges, wheel paths, quarter points and centerline, prior to or after letting and prior to construction to determine profile grades and the expected concrete yield
- Used machine control systems (i.e., GPS or 3D) to evaluate the ability to construct a quality project without stringline control

Proposed Workplan

Iowa Highway 9 in Osceola County

The workplan for the Iowa Highway 9 project was to utilize a GPS-equipped all-terrain vehicle (ATV) to profile the edges, wheel paths, lane quarter points, and centerline in fall 2008 or spring 2009 just prior to construction. This technique would be used to establish final pavement grades, consider milling depths, and estimate the quantities of concrete to be used. This project was to be built with stringline control.

US 65 in Worth County

The project proposed utilizing a GPS-equipped ATV to profile the edges, wheel paths, lane quarter points, and centerline in fall 2008 or spring 2009 just prior to construction. This technique would be used to establish final pavement grades, consider milling depths, and estimate the quantities of concrete to be used. In this case, the quantities and grades could be compared to the current control work being done by conventional survey methods in terms of

time, cost, and changes over the winter months. Machine control system information on paving train components was to be provided to eliminate the need for stringline control. It was recommended that the contractor utilize machine control of his choosing to guide the slipform paver and associated equipment.

County Road V-18 in Poweshiek County

The project proposed utilizing a GPS-equipped ATV to profile the edges, wheel paths, lane quarter points, and centerline in fall 2008 or spring 2009 just prior to construction of the bond breaker, after the bond breaker placement, and after concrete placement. This technique was to be used to establish final pavement grades, verify bond breaker placement depths, estimate the quantities of concrete to be used, and verify pavement overlay depths versus coring. Machine control system information on paving train components was to be provided to eliminate the need for stringline control. It was recommended that the contractor utilize machine control of his choosing to guide the slipform paver and associated equipment.

County Road W-62 in Johnson County

No GPS profiling was done on this project. Conventional and topographic surveys were used to identify the existing pavement surface elevations and conditions prior to construction.

Actual Workplan

Iowa Highway 9 in Osceola County

A GPS-equipped ATV was equipped with a laser profiler and GPS rover unit to profile the edges, wheel paths, lane quarter points, and centerline in fall 2008 and spring 2009 just prior to construction. An after-overlay construction GPS/laser survey of the project was suspended due to GPS equipment problems.

Information from spring 2009 was provided to the contract surveyor to aid in the establishment of paving grades and quantities. No milling was done on the project and conventional stringline paving was used for construction. The data were used in a very limited manner by the contract surveyor to check for potential thin concrete depths between the gradeline and the existing pavement surface.

US 65 in Worth County

A GPS-equipped ATV was equipped with a laser profiler and GPS rover unit to profile the edges, wheel paths, lane quarter points, and centerline in fall 2008 and spring 2009 just prior to construction. An after-overlay construction GPS/laser survey of the project was suspended due to GPS equipment problems.

Another run was made on only the edges and centerline of the roadway. This was done after the HMA milling and before the PCC overlay construction. This run was made at 5 mph, and data were obtained at 25-foot intervals. The GPS data were supplemented by the use of a manual survey with total stations to collect data at the same locations and intervals. Both the GPS and total-station survey were used to assist the contract surveyor in establishing the final pavement grades, considering milling depths, and estimating the quantities of concrete to be used.

In this case, the quantities and grades were compared to the current control work being done with conventional survey methods in terms of time, cost, and changes over the winter months. Machine control of the slipform paver and associated equipment was required on this project.

County Road V-18 in Poweshiek County

The same GPS-equipped ATV was used to profile the edges, wheel paths, lane quarter points, and centerline in fall 2008 and spring 2009 just prior to construction of the bond breaker and after the bond breaker placement. An after-construction GPS/laser survey of the project was suspended due to GPS equipment problems.

The after-bond breaker placement data were used to assist the contract surveyor in establishing the final pavement grades, verifying bond breaker placement depths, and estimating the quantities of concrete to be used. The data were intended for use in verifying pavement overlay depths versus coring but were not accurate enough for this work. Machine control of the slipform paver was required on this project.

County Road W-62 in Johnson County

No GPS profiling was done on this project. Conventional topographic surveys were used to identify the existing pavement surface elevations and conditions prior to construction.

Research Program Activity #2

One of the goals for activity 2 was to develop innovative ways to guide the longitudinal joint forming operation to match the underlying joint alignment. The project team used the following techniques:

- Used GPS to locate the existing centerline locations and changes in direction and attached the GPS to the saw to recreate that line in the finished surface
- Considered using physical measurements from the edge of the pavement to the centerline prior to and after the overlay to establish the centerline joint locations
- Considered using the "knife" for the centerline again for a portion of the project. Representatives of the National CP Tech Center are credited with the development of a device referred to as the "knife," which was used to develop longitudinal joints in plastic concrete. Its goal was to eliminate the need for sawing of longitudinal joints. It was used on some 60+ projects in Iowa and adopted by other states as an alternative

- joint forming method. Premature cracking in some Iowa projects has caused the knife to be banned in Iowa at this time.
- Used cores at the centerline to measure the location of the old versus the new centerline
 - Considered using a wire in the joint of the underlying pavement that could be located after the overlay was placed

Proposed Workplan

Iowa Highway 9 in Osceola County

The plans called for sawing the longitudinal joint after pavement construction by splitting the slab width. The slab-splitting centerline location for all paving methods was to be used. The distance from the paving line hub to the centerline prior to paving was also to be measured. The location of the final longitudinal joints under conventional saw control in the paving with nine random centerline cores, GPS measurements, and measurements from the hubs was also to be verified.

US 65 in Worth County

The plans called for sawing the longitudinal joint after pavement construction by splitting the slab width. The slab-splitting centerline location for all paving was to be used. The location of the final longitudinal joints (under GPS saw control) in the paving with three cores and measurements from the hubs was to be verified. The district office was interested in considering using the “knife” in one day’s operation at the centerline only.

County Road V-18 in Poweshiek County

It was suggested that the county consider the use of the GPS-controlled centerline saw for at least one-third of the project with cores used to verify the accuracy of the operation. The county did not desire to use the “knife” at this time.

County Road W-62 in Johnson County

A project involving a bonded overlay was not identified at the local or state level for this project. The widening proposed for the project allowed for the use of the “knife” for the formation of the joint between the existing pavement and the paved shoulders.

Actual Workplan

Iowa Highway 9 in Osceola County

The longitudinal joint location was manually identified after pavement construction by splitting the slab width. This joint location was identified by the location of the original pavement design centerline, which was offset to a hub line for paver guidance and a reference back from the hub to the edge of the overlay and centerline. Three random centerline cores were drilled instead of the nine suggested to verify the conventional method of centerline development. GPS data and hub measurements were not verified due to the loss of the hubs immediately after paving.

US 65 in Worth County

The plans called for sawing the longitudinal joint after pavement construction by splitting the slab width. The distance from the paving line hub to the centerline prior to paving was also to be measured. The location of the final longitudinal joints under the GPS saw control in the paving was to be verified with three cores and measurements from the hubs. The district office also considered using the “knife” in one day’s operation at the centerline only. The decision to allow one-lane paving negated the GPS sawing and coring activity on this objective.

County Road V-18 in Poweshiek County

Construction and saw manufacturer schedules did not support the use of a GPS-controlled centerline saw for at least one-third of the project. Instead, three 500-foot areas were established for this purpose. The first area utilized the GPS centerline surveys at 10-foot intervals to map the existing centerline prior to the PCC overlay. Those points were re-established by the GPS rover unit after the overlay construction and after a conventional saw moved along the line created by these points. Three cores were used to verify the accuracy of the operation. Cores were also used here to verify geotextile–bond breaker interaction.

In the second case, the GPS file was collected in the same manner as at the first site. When the data failed to replicate after the overlay construction, an adjoining section employing slab splitting was used as a default value to test the GPS ability to saw a given line. The underlying pavement centerline could not be traced in the surface. Three cores were obtained from an adjacent area immediately preceding area number two. By splitting the slab width and taking the cores, the research team was able to get a default value associated with conventional means of matching the underlying and overlay centerline joint.

A third area was cored at three centerline locations to both measure the centerline location and verify geotextile–bond breaker interaction.

County Road W-62 in Johnson County

This project was designed as an unbounded overlay. The widening that was proposed for the project allowed for the use of the “knife” for the formation of the joint between the existing pavement and the paved shoulders. One-lane construction did not allow for an analysis of the centerline location in the various layers.

Research Program Activity #3

One of the goals for this activity was to determine the best way to establish the level of need and timing of milling for existing asphalt surface preparation. The following are techniques for establishing the level of need and timing for this preparation:

- Mill half of the project length for cross-slope only (pave one-quarter with stringline and one-quarter with machine control)
- Do not mill any of the other half of the project length (pave one-quarter with stringline and one-quarter with machine control)
- Compare the predicted yield versus the actual yield on concrete in each of the quarter-length sections
- Compare shear bond strength in the four portions of the project
- Compare the finished profile elevation to the planned profile in the four sections

Proposed Workplan

Iowa Highway 9 in Osceola County

The district office considered automated machine control of the miller/slipform paver. The office chose not to mill any of the existing surface and chose to use conventional stringline control of the slipform paver.

US 65 in Worth County

The district office chose to mill a half-inch nominal and to make some profile corrections in the original pavement. The contract did not include milling to a given profile.

County Road V-18 in Poweshiek County

No milling was required on this project.

County Road W-62 in Johnson County

No milling was required on this project.

Actual Workplan

Iowa Highway 9 in Osceola County

No changes were made to the proposed workplan.

US 65 in Worth County

The district office chose to mill a half-inch nominal depth at the centerline and provide for a 2% cross-slope in the milling. The contract did not include milling to a given profile.

County Road V-18 in Poweshiek County

No changes were made to the proposed workplan.

County Road W-62 in Johnson County

No changes were made to the proposed workplan.

Research Program Activity #4

The goal for this program was to evaluate the use of innovative materials, such as geotextile layers, for use as bond separator layers. The following describes techniques that were used for the evaluation of such materials:

- Use geotextile bond breakers at intersection locations where the existing asphalt must be totally removed to maintain crossroad grades
- Use geotextile bond breakers over concrete surface patches

Proposed Workplan

Iowa Highway 9 in Osceola County

The asphalt bond breaker layer was in place in the as-built condition. The patched surfaces were to be sealed with emulsion and covered with sand.

US 65 in Worth County

The asphalt bond breaker layer was in place in the as-built condition. The patched surfaces were to be sealed with emulsion and covered with sand.

County Road V-18 in Poweshiek County

The existing bare pavement was to be overlaid with an asphalt bond breaker. The research team suggested that the county consider using a short stretch (i.e., 300 or 600 feet) of the geotextile bond breaker in three locations on the project. The research team provided eight rolls of one brand of geotextile material and the nails to fasten the material to the pavement. Each roll was 15 feet by 300 feet in length. The contractor was asked to install the material. No patching was required on this project.

County Road W-62 in Johnson County

The asphalt bond breaker layer was in place in the as-built condition.

Actual Workplan

Iowa Highway 9 in Osceola County

No changes were made to the proposed workplan.

US 65 in Worth County

No changes were made to the proposed workplan.

County Road V-18 in Poweshiek County

Four sites were identified for using geotextile in place of the 1-inch nominal depth HMA. Three sites were identified prior to the construction of the HMA layer, and one was selected to correct a construction problem. The research team provided eight rolls of geotextile material and fasteners for use on Sites 1, 2, and 4. A separate geotextile manufacturer provided the materials for Site 3. The contractor applied the materials at each of the four sites.

County Road W-62 in Johnson County

No changes were made to the proposed workplan.

Research Program Activity #5

One of the goals for this program was to determine innovative ways of handling traffic control for the construction of single-lane overlays as part of a two-lane or multi-lane overlay. The following were considerations made by the project team to determine these innovative techniques:

- Consider the leap-frog operation that uses one slipform and cure machine and maintains two days of run separation from the end of the first slab to the beginning of the second lane slab
- Consider two plants and two pavers operating from opposite directions and lanes
- Consider two plants and two pavers operating in the same direction and gapping two-mile segments between placements to maintain full operation
- Consider lateral minimum clearance pavers to maintain traffic on a shoulder and opposing or completed lane
- Evaluate worker success and response to working on the shoulder side of the pavement

Proposed Workplan

Iowa Highway 9 in Osceola County

The use of detours and full-width paving would not require the inclusion of special equipment and methods of placement or one-lane paving.

US 65 in Worth County

The use of detours and full-width paving would not require the inclusion of special equipment and methods of placement or one-lane paving.

County Road V-18 in Poweshiek County

The use of detours and full-width paving would not require the inclusion of special equipment and methods of placement or one-lane paving.

County Road W-62 in Johnson County

The project location was selected to provide for one lane of paving and different elements of traffic control, paver control, and paving train length.

Actual Workplan

Iowa Highway 9 in Osceola County

No changes were made to the proposed workplan.

US 65 in Worth County

The contractor's decision to use one-lane paving under closed road/detour conditions changed the construction procedures for this project. Through traffic was discouraged by the removal of core-outs at each end of the project and the replacement of bridge approaches at three bridges along the project length. Persons living along the project were allowed to use the nonpaved lane during each work phase, and their home access was usually disturbed for less than 30 hours during pavement strength gain periods. Traffic was allowed to cross the project at three locations throughout the construction period.

County Road V-18 in Poweshiek County

No changes were made to the proposed workplan.

County Road W-62 in Johnson County

No changes were made to the proposed workplan.

Research Program Activity #6

The program goal was to investigate potential ways of using both existing and new paving train components so that the length of time of the construction operation was minimized. The following includes techniques of using these components to minimize operation time:

- Consider ways to reduce the time between the texture machine/joint sawing and the completion of the shouldering operation and striping
- Consider alternate haul roads to speed up shoulder construction and opening time

Proposed Workplan

Iowa Highway 9 in Osceola County

The combination of the maturity measurements and the haul road selection relative to paving direction and contractor/shoulder subcontractor coordination would be monitored to determine the best way to reduce the time from the curing machine passage to striping time and opening. A pre-job conference or notification to the contractor to use a haul road plan that would fit the shouldering operation directly behind the paver and not after the project was complete was to be utilized. Shouldering would proceed when construction traffic strength was attained on the slab.

US 65 in Worth County

The combination of the maturity measurements and the haul road selection relative to paving direction and contractor/shoulder subcontractor coordination would be monitored to determine the best way to reduce the time from the curing machine passage to striping time and opening. A pre-job conference or notification to the contractor to use a haul road plan that would fit the

shouldering operation directly behind the paver and not after the project was complete was to be utilized. Shouldering would proceed when construction traffic strength was attained on the slab.

County Road V-18 in Poweshiek County

The combination of the maturity measurements and the haul road selection relative to paving direction and contractor/shoulder subcontractor coordination would be monitored to determine the best way to reduce the time from the curing machine passage to striping time and opening. The county would consider utilizing a pre-job conference or notification to the contractor to use a haul road plan that would fit the shouldering operation directly behind the paver and not after the project was complete. Shouldering would proceed when construction traffic strength was attained on the slab.

County Road W-62 in Johnson County

The research team would monitor the various activities in the paving train to look at ways to reduce the time of road closure for construction of one-lane paving

Actual Workplan

No changes were made to any of the proposed workplans for this activity.

Research Program Activity #7

The goal for this program was to determine the appropriate opening strength that is required of the concrete for use by local traffic and through truck and construction traffic for depths of concrete of less than six inches. The following includes techniques for determining the appropriate opening strength:

- Use maturity probes at 500-foot or closer intervals
- Use shear strength tests, pull-off tests, or other tests to determine the bond strength at opening
- Use engineering judgement and shoulder construction equipment to determine how closely one can operate to the paving operation versus the potential for cracking (e.g., corner, longitudinal, or transverse) and limit cracking to 1 % of the slabs for the day
- Evaluate saw cutting to determine if a relationship to opening strength can be established

Proposed Workplan

Iowa Highway 9 in Osceola County

The research team suggested that the contractor employ a pickup truck at the beginning of each day and at midday to determine the point at which the tires begin to indent the concrete to a depth of a quarter-inch and correlate it to the estimated maturity of the concrete strength at that location. Testing would be done in each lane every one-half day in a longitudinal location that straddled the centerline so as not to impact the profile measurements. A method to tie the sawing window to strength would be evaluated.

US 65 in Worth County

The research team suggested that the contractor employ a pickup truck at the beginning of each day and at midday to determine the point at which the tires begin to indent the concrete to a depth of a quarter-inch and correlate it to the estimated maturity of the concrete strength at that location. Testing would be done in each lane every one-half day in a longitudinal location that straddled the centerline so as not to impact the profile measurements. A method to tie the sawing window to strength would be evaluated.

County Road V-18 in Poweshiek County

Maturity measurements were to be made to each 500-foot section of pavement and correlated to the contractor/county judgment on how close to bring the construction equipment for shoulder and joint sealing work. The documentation of the strength at the time of joint sawing would be noted.

County Road W-62 in Johnson County

Maturity measurements were to be made to each 500-foot section of pavement and correlated to the contractor/county judgment on how close to bring the construction equipment for shoulder and joint sealing work. The documentation of the strength at the time of joint sawing would be noted.

Actual Workplan

All work plans were followed in terms of the identified activities, with two exceptions. One exception was that no contracting authorities would allow the pickup truck test method for determining the traffic opening strength. This had to be done in correlation to other construction activities such as maturity testing.

The other exception was that, because three of the four projects were under construction at the same time instead of the two projects that were expected in the budgeting, the maturity device installation interval was extended to 1,000 feet from the original 500 feet.

Added Research Program Activities

The research team has been working to develop a design program for use by highway agencies in the development of trial and design overlay thicknesses. The program is based on the work done by American Concrete Pavement Association (ACPA), CTL Group, and the Colorado Department of Transportation (Cable et. al, 2005). Those groups developed the thickness design methodologies but let the user develop much of the inputs. One of those inputs is the measurement of the structural adequacy of the existing pavement layers. The design program looks to reduce deflections in the final surface and extend the life of the pavement. Currently, it is centered around the single existing layer of pavement theory. Previous graduate work at Iowa State University (ISU) has provided a framework for handling multi-layer or composite pavements and for states to use their falling weight deflectometer (FWD) to provide the necessary overlay design information on pavement structural adequacy.

FWD testing was conducted on Iowa Highway 9 in Osceola County, US 65 in Worth County, and County Road V-18 in Poweshiek County before and after PCC overlay construction. The work utilized the ISU KUAB trailer-mounted FWD. Testing was accomplished at 0.1 mile increments in both lanes for the length of each project.

OVERLAY CONSTRUCTION

Iowa Highway 9 in Osceola County

Mainline Paving

Mainline paving for Iowa Highway 9 in Osceola County began on July 2, 2009 and ended August 4, 2009. Handpours for the turn lanes, bridge approaches, and gaps at US 59 were completed August 22, 2009. Mainline paving was accomplished with use of a Guntert & Zimmerman slipform paver and burlap drag, work-bridge with burlap drag, and a Gomaco cure/texture machine, shown in Figure 6. Hand-floating and mopping of the surface was accomplished between the slipform and the work-bridge.



Figure 6. Osceola County paving train

The slipform was fed by a fleet of dump trucks and a central drum mix plant located approximately 3 miles southeast of the end of the project (EOP) or 12 miles from the beginning of the project (BOP).

Workers also installed deformed, epoxy-coated Number 4 rebars on 30-inch centers across the old and new widening joints. These bars were 60 inches in length and were nailed to the existing asphalt surface with no less than three nails for each bar. It was important to know that the orientation of the bar nails must be such to resist movement of the roll of concrete in front of the slipform paver, as shown in Figure 7. The finished overlay product for the Osceola County project can be seen in Figure 8.



Figure 7. Osceola County tie bars



Figure 8. Osceola County finished pavement

Shoulder Construction

Due to the lack of quality stone materials in this area, the material that was removed to accommodate the 2-foot widening was stockpiled at the concrete plant site for application to the new shoulders after paving was complete. A 4-foot-wide milling head machine was used to remove a portion of the existing shoulders to a depth of 3 inches below the existing pavement surface.

Shoulder construction consisted of two-lift construction, and both lifts were placed between September 8 and 11, 2009. The stone was hauled from pits owned by the shouldering contractor

in Minnesota. The materials were stockpiled in an Iowa DOT stockpile site near the US 59/Iowa Highway 9 intersection by semi-bottom dump trucks. End dump trucks were later used to move the materials to the construction areas. Due to an exceptionally heavy rain and the contractor's method of finishing the stone shoulder surface, a third lift was required for additional stone in driveway fillets and storm-damaged areas. This work was completed on September 22. Haul distance was a critical factor in this shouldering effort.

Special Construction Concerns

A large number of longitudinal subdrains were added to this project but accounted for less than one week of construction prior to the paving. In addition, a number of full-depth concrete patches were placed prior to the paving. Some contractor issues caused this activity to take multiple weeks, when only one week was required to complete the task. The project required 1,379 square yards of full-depth concrete patches and 153 surfaces patches of asphalt to repair transverse joint locations in the existing pavement. The general condition of the surface was good due to the maintenance practices of the local Iowa DOT authorities, as shown in Figure 9.



Figure 9. Osceola County original pavement

The slipform paver was guided by stringlines on this project. This work required a concentrated effort on the part of the contract surveyor to map the existing surface, develop the new overlay profiles, and do the field staking at 25-foot intervals. Multiple survey crews were required to do these activities and stay ahead of the paving operation.

Throughout the construction process of Highway 9, it was observed that there were numerous “stops and starts” throughout each day due to the lack of continuous delivery of concrete trucks. This was affected by the number of available hauling units and the planned haul routes. This type of occurrence was controlled by the contractor and his overall plan for execution of the project and the associated smoothness of the completed surface. Some slipform pavers such as the

Guntert & Zimmerman used on this project resulted in only minor changes in ride due to stops during paving. Concrete delivery route selection, such as eliminating city street routes where possible, can aid in reduced travel time and increased production on sites such as this one.

The cure/texture machine followed the slipform at various distances throughout the project. At some points, the cart was approximately 400 to 500 feet back from the paver; at other times, the cart was immediately behind the burlap. The timing of texturing and curing is an art, and it was determined by a hand touch test to assure that the surface was dry enough to obtain the correct tining depth. Some difficulty was experienced in keeping the curing nozzles clean and applying an adequate amount of cure on the sides and top of the slab. The operator corrected this as the project proceeded east.

Random transverse cracking outside of the transverse joints occurred between Stations 304+25 and 342+75. This happened on the same day that the pavement construction reached its maximum length on the entire project. There were approximately 30–40 cracks that were angled from the joints on about 3,850 feet of the project. A meeting was held with the research team, Iowa DOT representatives, and supervisors from the contractor. It was suggested and agreed upon that the cracks were the result of late sawing of the individual joints. The contractor agreed to saw the initial relief joints at every fifth rather than every tenth joint, and the saw crew was encouraged to stay closer to the paving so that the concrete was more “green” when it was sawed on days when the paving moved rapidly and when the temperature swing from morning to noon to evening, coupled with wind, caused the thin overlay to rapidly set. No further cracking of this type was evidenced on the project.

Contract Administration and Traffic Control

This contract called for construction of an overlay on the existing 24-foot-wide pavement and widening to 28 feet to be accomplished, under closed road conditions. The road was closed only after patching and installation of additional subdrains on June 15, 2009 and reopened to the public on September 29, 2009.

A total of 100 working days were allowed for the contract, and 93 working days were charged for the work. Rain was the main contributor to the length of time that was required for the project.

US 65 in Worth County

Mainline Paving

Mainline paving for US Highway 65 in Worth County began on August 27, 2009 and ended October 22, 2009. Handpours for the turn lanes, bridge approaches, and medians at two channelized intersections were completed October 30, 2009. Mainline paving was accomplished with the use of a Guntert & Zimmerman slipform paver and burlap drag, work-bridge with

burlap drag, and a Gomaco cure/texture machine, shown in Figure 10. Hand-floating and mopping of the surface was accomplished between the slipform paver and the work-bridge.



Figure 10. Mainline paving

The slipform was fed by a fleet of dump trucks and a central drum mix plant located approximately 0.5 miles east of the project and 2 miles south of the EOP or approximately 8.5 miles from the BOP. Concrete was dumped directly onto the existing pavement in front of the slipform for both lanes. A view of the finished two lanes of the overlay in Worth County can be seen in Figure 11.



Figure 11. Worth County finished pavement

Workers also installed deformed, epoxy-coated number 4 rebars on 30-inch centers across the old and new widening joints. These bars were 72 inches in length and were nailed to the surface with no less than three nails for each bar. It was important to know that the orientation of the bar nails must be such to resist movement of the roll of concrete in front of the slipform paver. The layout of the bars for the Worth County work is shown in Figure 12.



Figure 12. Worth County bar layout

This project was let to be built under a closed road with a detour traffic control plan. At the preconstruction conference, the contractor requested and was allowed to build the roadway one lane at a time while the road remained closed to through traffic. He chose this method for the following reasons:

- One-lane paving allowed for the use of the other lane as part of a two-lane haul road for concrete.
- The method allowed for the use of one slipform setup of a 16-foot width to pave all of the mainline, including the two channelized intersections.
- The project called for the use of stringless paving control. One-lane construction allowed for remediation of the first lane surface for ride problems prior to the paving of the second lane, if the need arose.

The requirement for stringless paving called for two items not found in conventional stringline controlled operations. The first item was the establishment of vertical control points 250 feet along the roadway and on alternating sides of the roadway. This was provided by a contract surveyor to supplement the existing set of control points that were set by the State of Iowa at 1,000 foot intervals along the east side of the existing pavement on the foreslope. This kept the paving equipment (i.e., slipform and trucks) from interfering with the line of sight between the total stations and at least three of the known vertical control points at any given location along the roadway. The project location was relatively flat but contained several horizontal curves that

posed sight distance problems as crops reached maturity height along the roadway. This also contributed to the need for the 250-foot spacing of control points.

The second requirement for the stringless paving operation was the addition of the electronic/hydraulic controls on the slipform paver. This was accomplished during the week of August 24–27, 2009. Contractor employees went through a five-day course that covered the operation of the total stations, radios, and operation of the slipform paver appurtenances in the week prior to the field installation.

Construction time was longer than what would be expected for a project of this nature. The extended time was attributed to the weather during the second lane placement. The month of October included a majority of lost time due to the threat of rain.

The contractor staff, contract surveyor, and the research team worked together on this project to provide a better model of the existing pavement that aided in the development of the surface model and the guidance of the slipform paver. Once paving began, there were no lost days due to model construction and application.

Shoulder Construction

Due to the widening requirements for this project, a 4-foot-wide milling head machine was used to remove a portion of the existing shoulders to a depth of 3 inches below the existing pavement surface. The material was hauled to two places. The first portion of the material was sent to an Iowa DOT garage approximately 11 miles from the project to replace stone that had been placed on these shoulders the previous year. The remaining portion was stockpiled at the concrete plant site for application to new shoulders after paving was complete.

Shouldering operations began on September 30, 2009 on the northern 2 miles of the project with the application of the salvaged shoulder materials in the first lift. Materials were placed by a shouldering machine that was fed by contractor trucks that were not needed for the short concrete hauls at this stage of the project. When mainline placement of concrete was complete, additional trucks were used to place the shoulder stone on the remaining portions of the project. All material was placed in two lifts due to the depth of the shoulders, and virgin aggregate from the same quarry site that housed the concrete plant was used to finish the work on November 4, 2009. The salvaged materials and the virgin shoulder materials were obtained from the same location as the concrete plant. One-lane paving could have made shoulder placement available during the construction.

Special Construction Concerns

In general, the pavement surface was well maintained and in good condition, as shown in Figure 13. The existing surface friction course was exhibiting signs of localized delamination, and the Iowa DOT decided to remove it by milling. The final surface after milling is shown in Figure 14.



Figure 13. Worth County before milling



Figure 14. Worth County existing condition after milling

Full-depth repairs of the existing surface prior to overlay changed from a minimal number of joint repairs to over 400 transverse joints that were given a full-depth repair with Portland cement concrete techniques. This work served to basically remove any longitudinal stresses in the existing pavement and should enhance the performance of the overlay.

This work was done at the same time that the additional longitudinal subdrains were installed. Both operations were accomplished before the road was closed to through traffic. These operations seemed to work well together.

The Iowa DOT chose to mill the existing HMA surface to achieve a 2% cross-slope in each lane and reduce the amount of PCC overlay overrun due to wheel ruts. The Iowa DOT also wanted to remove much of a bituminous surface sealcoat that showed large areas of surface loss. There was a concern about the bonding capability of this layer. Milling was accomplished by using a 12-foot-wide milling machine, shown in Figure 15. Grade was established by setting the mill to scratch the surface at the centerline and mill to the 2% cross-slope across the driving lane in each direction. In super-elevated curves, the mill was set to follow the existing rate of the cross-slope.



Figure 15. A 12-foot-wide milling machine

Paving train length usually did not exceed 500 feet from the delivery point in front of the slipform paver to the cure/texture machine in the rear. This distance could be reduced to 250 feet on days with hot, winding conditions. It was controlled by the rate of set in the concrete. Cool, overcast days did not allow the pavement to set quickly for either cure/texturing or joint sawing.

Shoulder construction was slowed by a combination of factors that were present in this project. The first involved the decision to mill out only 4 feet of the 8 foot shoulder surface in preparation for the widening. This area had to be drained in some way to provide a good base for the widening concrete and stable base for the shoulder stone when it was applied. One could not assume there would be no rain between the removal and replacement of the shoulder stone. Second, the removed shoulder stone contained an excess of fine materials that did not compact well when dry or when dampened due to rain or pug-milling. If the shouldering had followed directly behind the paving, some of these problems could have been averted. Contractor decisions to use their own equipment for this process slowed the operations.

The centerline joint was hand tooled for both lanes of paving, shown in Figure 16. Three early entry saws were used to cut the transverse joints. The first saw cut every fifth joint to a depth of T/3, and the other two saws cut the intermediate joints to a depth of T/4. The paving of the 16-foot-width did not exceed three-quarters of a mile per day on this project. No premature cracking was noted on this project.



Figure 16. Hand tooling of the center line

Surface model building was enhanced on this project over the Poweshiek project in the following ways:

- Communication between the contracting authority, contractor, and model builder were used to identify each group's goals prior to the start of construction. A pre-pour type meeting greatly aided in this communication.
- The model base information for the existing surface was improved with more attention to data collection by the research team and the contract surveyor. This work was done between the time of surface milling and the time of overlay placement.
- The contract surveyor had the advantage of learning from the Poweshiek project what to consider in the model development.
- With a more proactive research team and contract surveyor, the model was completed in time for the owner and contractor to review the profiles for several days before paving. In the future, this should also allow all parties to identify minor deviations in alignment or profile days in advance of the questionable data and allow for corrections.

Contract Administration and Traffic Control

This contract called for the construction of an overlay for the existing 24-foot-wide pavement and a widening to 32 feet to be accomplished under closed road conditions. The road was closed only after patching and installation of additional subdrains on July 18, 2009 and reopened to the public on November 18, 2009.

One-hundred working days were allowed for the contract, and 96 working days were charged for the work as of November 30, 2009. Seeding of disturbed areas remains to be done in spring 2010 during the seeding period.

The project also had a secondary set of working days for a detour on the south portion of the project. Sixty working days were allowed for this portion of the paving, and 60 days were utilized.

This project also required the contractor to maintain traffic through at least one of the intersections in the City of Kensett at all times and not close two consecutive rural intersections at the same time.

Rain was the main contributor to the length of time that was required for the project. The application of one-lane paving did not seem to affect the rate of paving on this project. One is still limited by the ability to produce the material, haul and place material, and saw joints each day. The rural nature of this project did not create any access problems for landowners with the one-lane construction. It did create problems for the quarry operation and a wind farm materials equipment site. Two-lane construction could have reduced those locations' down time or detour usage.

V-18 in Poweshiek County

Mainline Paving

This contract called for the placement of a nominal 1 inch of HMA concrete. This was placed to serve as a bond breaker between the original PCC pavement and the PCC overlay. Three areas were chosen by the research team to substitute a geotextile fabric for the asphaltic concrete in an effort to measure the differences in cost and performance compared to the bond breaker. The road was closed prior to the placement of the bond breaker. The HMA layer was placed between May 19 and May 21, 2009. The asphalt was placed one lane at a time under closed road conditions.

After placing the bond breaker asphalt, the contractor chose to place a portion of the shoulder stone before the overlay construction. This served to develop a uniform pad line for the slipform paver operation on this route under the new stringless paving specification. The fabric bond breaker was placed immediately in front of the PCC paving operation.

The requirement for stringless paving called for two items not found on conventional stringline-controlled operations. The first was the establishment of vertical control points 250 feet along the roadway and on alternating sides of the roadway. Due to these control points, paving equipment would not interfere with the line of sight between the total stations and at least three of the known vertical control points at any given location along the roadway. Large elevation changes in the existing roadway profile also contributed to the need for the 250-foot spacing.

The second requirement for the stringless paving operation was the addition of electronic/hydraulic controls on the slipform paver. This was accomplished during the week of June 13–19, 2009. Contractor employees went through a five-day course that covered the

operation of the total stations, radios, and operation of the slipform paver appurtenances in the week prior to the field installation.

Mainline paving began June 26^t and was completed July 22. Handwork at intersections was completed July 23, 2009. The construction time was longer than would be expected for a project of this nature. The extended time included work by the contractor, contract surveyor, and the research team to understand the modeling process and relate it to the field surveys done prior to the modeling process. Other contractor-committed work also accounted for 2–3 days within that time frame.

The paving train consisted of a Gomaco 2800 Commander slipform paver with V float, hand finishers, burlap drag bridge, and Gomaco cure/texture cart, as shown in Figure 17. In this case, the contractor also chose to use the Gomaco GSI profiler, shown in Figure 18, to measure the profile at various points across the slab and continuously along the pavement for the length of the project. This was used as a research project for Gomaco and to help the contractor evaluate the operation of the stringless technology and the slipform paver, which was a new machine for the contractor.



Figure 17. Gomaco 2800 Commander slipform paver



Figure 18. Gomaco GSI profiler

This project proceeded well on paving days, with 10–19 transit mix trucks delivering the concrete from a plant that was located at the north end of the project. The plant was about 10 miles from the beginning of the project. The operation allowed constant and consistent movement of the paving train. Delivery was very good, and at times the contractor was able to achieve speeds of 10–20 feet per minute due to the discharge of two trucks that delivered the concrete side by side.

A view of the finished Poweshiek County pavement can be seen in Figure 19.



Figure 19. Poweshiek final slab

Geotextile Bond Breaker Construction

Geotextile bond breaker material was specified for three locations on this construction project. In each case, the material was placed on both lanes of the pavement surface. Those sites included the following locations: Stations 20+00 to 22+94 (flat or tangent grade), Stations 384+00 to 389+91 (negative grade), and Stations 36+00 to 38+94 (positive grade). The intent of the location selections was to evaluate the interaction between the paving equipment and concrete and the fabric placement and overlay constructability. Due to an overlay construction problem, a need for a fourth fabric site was identified at Stations 34+05 to 37+05 (entry to horizontal curve).

Two separate material suppliers were used for this project. The first material, HATE B 500-PP was supplied by Huesker of Charlotte, North Carolina. The alternative material, Tencate Mirafi, 1450 BB was supplied by Tencate Geosynthetics of North America. Both materials met the same material specifications outlined in a special report done by the Transtec Group (Rasmussen and Garber, 2009) and were supplied in rolls, 15 feet in width by 300 foot in length. The Huesker material was utilized on Sites 1, 2, and 4, while the Tencate Mirafi material was used on Site 3.

Installation instructions were the same for both materials. A copy of the installation instructions can be found in Appendix A. The materials were to be overlapped no more than 6 inches at centerline, and the excess width was to be laid flat on the road shoulder to act as a wick drain for any moisture between the pavement layers after construction, shown in Figure 20. The test areas were gapped during the placement of the 1-inch nominal HMA bond breaker. Fabric roll placement began 3 feet before the HMA gap to allow for overlap of the two materials and ended 3 feet on the HMA at the terminus of the test area, as shown in Figure 21.



Figure 20. The gap in the HMA bond breaker where the geotextile material was placed



Figure 21. Fabric roll placement

The materials were attached to the existing pavement with nails, shown in Figure 22, from Hilti ramset-type guns. They were nailed in an approximate 5.25-foot transversely by 5-foot longitudinal grid pattern with one row of nails 3 inches from each pavement edge, one at the centerline, and one at each midpanel location. The material was difficult to keep straight under this type of construction and developed small bubbles in the surface. These bubbles were folded forward, and an additional nail was added to place them in a horizontal position prior to concrete placement. The intent was to have no bubbles under the material and no more than three layers of fabric between the concrete layers. It was permissible to cut such a bubble and make a two-layer overlap but this technique was not employed on this project.



Figure 22. Nailing pattern for the geotextile materials to the existing pavement

At the first location, no prior instructions were given to the contractor as to how to place the material. The contractor chose to use hand labor to unroll two rolls of material at the same time. He accomplished this in the small area between the concrete delivery trucks and the slipform paver. The use of ready-mix concrete delivery trucks made this possible. The operation can be seen in Figure 23. It required that the material also be nailed to the existing surface in the workspace between the rolls and the concrete placement area. This required approximately four people to unroll the fabric, three people nailing the material to the existing surface, and close coordination with the person directing the concrete placement.



Figure 23. Unrolling and nailing the material to the existing pavement

Site 4 was the next placement area. In this case, the contractor placed the entire length of fabric in both lanes prior to any concrete placement. The same number of crew were utilized for placement. Concrete delivery trucks were directed to back into the slipform paver by using the middle of the roadway surface. This required some turning of the trucks due to the horizontal curve location. No particular problems were noted in this fabric or concrete placement. The use of “street pads” on the slipform paver and the control of the concrete “head” in front of the paver removed any problem with the slipform trying to pull the fabric toward the paving operation, as shown in Figure 24.



Figure 24. Streetpads

The downgrade nature of Site 2 resulted in a different approach to the material placement. In this case, the first roll was placed prior to any concrete placement. Delivery trucks backing up the grade used the non-fabric-covered lane until they got closer to the paving operation and then deposited concrete over the second lane roll, as shown in Figure 25. This allowed the trucks to not maneuver on the fabric.



Figure 25. Non fabric-covered lane

At Site 3, both rolls of fabric were placed prior to concrete placement, and all delivery trucks used the center of the fabric-covered slab to get to the paving operation, as shown in Figure 26. The up, or positive, grade at this location caused some small movements in the fabric due to truck front-braking systems. Again, no fabric ruptures were noted at any of the locations and

fabric bubbles were treated with extra nails to secure them to the surface prior to concrete placement.



Figure 26. Fabric and concrete placement

Shoulder Construction

Actual shoulder stone placement began July 22, 2009 south of Interstate 80 and was completed August 4, 2009. Due to the depth of the pavement overlay, the shoulder stone was placed in two lifts on top of the pre-paving materials. The shoulder stone was placed with a shouldering machine, and as many as ten trucks supplied the material. It was compacted with the use of pneumatic and steel rollers. The first lift was placed beginning at Interstate 80 moving south and then proceeded counterclockwise (working with traffic) around the project. The second lift was placed in the same manner.

The source of the shoulder stone was north and west of Malcom 1.5 miles and northwest of the project site. This provided direct access to US 63 and Iowa 85 as a haul road to the south end of the project while the paving proceeded north. The materials were hauled by a combination of private and company trucks, depending on availability.

Special Construction Concerns

The existing pavement on this project was in good condition. The surface was uneven at the joints from years of vertical movement during spring thaws and subgrade clays that did not drain well. There was some evidence of random joint faulting, but in general travel was not impacted by the amount or severity of faulting. A view of the existing pavement prior to the placement of the bond breaker layer is shown in Figure 27.



Figure 27. Poweshiek existing pavement

This project contained 37 existing transverse joints that were exhibiting vertical movement and damage from expansion of the pavement in a longitudinal direction. The contracting authority had experienced this type of problem on other county projects and had developed a working rehabilitation technique. The technique consists of using a rock saw to cut along the joint at a width of 6 inches. The void was filled with hot mix asphaltic concrete. This work was done before the overlay operation and allowed the concrete delivery trucks to compact the asphalt during paving operations, as shown in Figure 28.



Figure 28. Rehabilitation technique for damaged transverse joints

Available trucks were a problem on the shouldering operation. Contractor trucks were not available during much of the paving time period. This, coupled with lack of available manpower for the shouldering equipment, slowed the operation.

The contractor was able to use three early-entry saws to cut the transverse joints and one additional saw for the longitudinal joint on this project. The 6-inch overlay depth resulted in the contractor saw crew being able to start and stay within sight distance of the paving operation with the joint development.

The construction of the computer model for the paving proved to be a learning experience on this project. All parties involved in the work learned on-site because this type of construction was the first of its kind in Iowa. The Leica, Inc., personnel were on site to advise the contract surveyor, research team, and contractor about ways of improving the model construction and its application. This project identified the need for understanding the “as-built” plan survey information, accurate mapping of the existing surface, development of the model in advance of the paving, and checking of the surface to model fills during paving.

Contract Administration and Traffic Control

This contract called for construction of the overlay full width (i.e., 22 feet), under closed road conditions. The road was closed to begin the HMA bond breaker construction on May 19, 2009 and reopened to the public on July 17, 2009. The second lift of shoulder stone was applied under the “open road” conditions.

A total of 65 working days were allowed for the contract and 55 working days were charged for the work. Rain and stringless paving startup difficulties contributed to the length of time required.

County Road W-62 in Johnson County

Mainline Paving

This project was developed to be constructed one lane at a time, under closed road conditions, and in stages. The contractor requested and was granted the opportunity to change the staging plan and move to the end of the project to begin paving. Paving began July 24, 2009 at the EOP and consisted of curve widening on both sides of the approach to Highway 921. Paving resumed on August 3, in the northbound lane at Freund Road to serve two major traffic generators (i.e., sand pit and precast pipe companies and a trailer court). The northbound lane was completed August 3 and the southbound lane was constructed between August 17th and August 20. The time between lane paving was used to allow for strength gain on the northbound lane and to strengthen the southbound shoulder for support of the slipform paver and delivery trucks.

The paving train for this project, shown in Figure 29, consisted of a CMI slipform paver with burlap drag, hand finishers, work-bridge with burlap drag, and a Pavesaver cure/texture cart. The

train was usually less than 500 feet between the slipform and the cure/texture cart. Cool, cloudy weather limited the contractor's ability to shorten that distance.



Figure 29. The project paving train

In this case, the contractor employed a variety of trucks to deliver the concrete. These included conventional single and semi-trailer dump trucks and transit mixers. The contractor had the capability to load wet batches into the transit mixers for this work. The concrete plant was located approximately 5 miles from the site in an urban area, which, along with the one-lane paving process, made delivery erratic at times.

In this case, the contractor obtained two overlay projects for the same county at the same time. Both employed one-lane paving and the same width of lanes. The contractor moved from the W-62 project to the other project due to wet subgrade and the needs of both projects in terms of time constraints.

Paving of the mainline resumed on September 3 at the BOP and moved forward in the northbound lane through a major intersection (i.e., one-half mile north of BOP). The pavement was allowed to gain maturity opening strength, and then paving was reversed and proceeded south on September 8 through the BOP.

Mainline paving resumed in the northbound lane on September 11 and finished in the northbound lane on September 28. The paving train was moved back to a point approximately one-half mile north of the BOP and resumed paving on October 5 in a northbound direction in the southbound lane. Paving of the southbound lane was completed on October 15, 2009. The finished two-lane product for the Johnson County work can be seen in Figure 30.



Figure 30. Johnson County finished pavement

Shoulder Construction

Shoulder construction on this project involved the addition of a very narrow earth fill to allow for the 8-foot-wide paved shoulder and a 1–2 foot rock fillet along the edge of the paved shoulder. The existing right of way, 66 feet, made it very difficult to construct a 34-foot-wide pavement between the backslopes in some areas and on narrow fills in other areas. The rock was placed in one lift between August 10, 2009 and November 6, 2009. Shouldering stone was placed as the paving work was completed in the various areas and not in one continuous operation. The shoulder stone source was River Products, with quarries located along Interstate 80 in Iowa City. This created an approximate 12-mile haul to the north end of the project, or 16 miles to the south end of the project. The material was hauled by the contractor trucks. It was placed with the conventional shouldering machine. This was often done during times when paving was not in progress, which allowed the use of the contractor trucks for this purpose. The one-lane construction allowed trucks to enter from either end of the project during the construction.

Special Construction Concerns

The existing pavement was in fair to good condition for its traffic load and the age of the underlying concrete pavement. As seen in Figure 31, there were many areas of delamination between the existing asphalt layers and the original concrete in the form of potholes near the centerline and across the slab in various areas. The plans called for 690 square yards of full-depth patching with concrete, and one patch by count.



Figure 31. Johnson County existing condition

Joint development was accomplished with the use of early entry saws. The use of one-lane construction, the “knife” for development of the only longitudinal joint, and conventional slab sizes greatly reduced the need for saws on this project.

This project was built using conventional stringline control of the paving train. This created a problem with the use of the alternate lane and shoulder. The stringline used approximately one-half of the existing 9-foot-wide lane and forced vehicles onto the new shoulder, which was soft from the rains and construction process and needed in the narrow right of way.

This project would have benefited from a stringless application.

Contract Administration and Traffic Control

The plans for this project called for two types of construction staging. The first type dealt with the construction sequence from the south to the north, and the second dealt with the cross-section that was to be employed for each of the construction sequences. Those plans can be found in Appendix B.

The staging plan called for a south-end portion (approximately 1 mile) in the city of Hills to be constructed first, followed by the northbound lane from Hills to Iowa 921(EOP). The third phase was to build south from Iowa 921 to provide access to a trailer park and then continue south to the city of Hills and complete the project through one-lane construction.

Large amounts of rain and soft subgrades, in addition to project agreement problems between the city of Hills and Johnson County, changed the plan completely. All paving was done one lane at a time, as planned, but the sequence was changed to accommodate the weather, the City of Hills, and subgrade conditions. Overlay construction began at the EOP and proceeded south to

accommodate three major traffic generators. Second, the south end, or BOP area, was completed to allow access to Hills. Finally the center sections of the project were completed.

This contract called for the construction of an overlay for the existing 24-foot-wide pavement and a widening to 34 feet to be accomplished under closed road conditions. In addition, the plans called for the paving to be completed one lane at a time under local traffic. The traffic control consisted of a Closed Road sign and one-way traffic through the project at all times without a traffic control vehicle. No major problems were noted in the traffic control. There were some conflicts between the paving operation and property owners regarding the closing of access for the period needed for gaining pavement strength at three locations. These were gapped and closed later when traffic could be diverted to one side of the existing driveways. The road was closed only after patching and installation of additional subdrains on July 20, 2009 and reopened to the public on November 16, 2009.

A total of 60 working days were allowed for the contract, and 55 working days were charged for the work as of November 30, 2009. Seeding of disturbed areas remains to be completed in spring 2010 and will utilize most of the remaining 5 working days. Rain was the main contributor to the length of time required for this project. The construction of narrow, deep fills in areas with little or no drainage in a 66-foot-wide right of way was difficult even under dry conditions.

DATA COLLECTION

Concrete Strength

Maturity data were collected by placing thermocouples in the fresh concrete at overlay mid-depth 500 feet from the beginning and ending of the project and approximately every 1,000 feet along the project. The devices were placed approximately one foot from the edge of the pavement in the widening area. The thermocouples were placed in the concrete with a small wire taped to a 3/8-inch wooden dowel. The thermocouple wires plugged into Madgetech data recorders that recorded temperature readings inside and outside the concrete every 15 minutes. The time/temperature measurements were taken to estimate the strength gain rates of the concrete. The thermocouples were removed after approximately two to three days, or when the concrete reached an estimated 500 pounds per square inch (psi) flexural strength, as computed from a field lab test by the contractor. The resulting data were placed in an Excel file, where the time/temperature factor was calculated. The same procedure was used for each of the four construction projects.

The desired Poweshiek County maturity time/temperature factor (TTF) for the project was 1,875, which meant that a strength of 500 psi was achieved. Once the concrete reached the adequate TTF, the contractor and owner were notified that the road could be opened only for local traffic and construction traffic.

The desired maturity TTF for the Osceola County project varied over three separate maturity curves, from 978 to 996 and finally 1,354 for the remainder of the project. This meant that the strength of 500 psi was achieved. State specifications require that the contractor make a new curve any time the mix materials are changed. Once the concrete reached the adequate TTF, the contractor and owner were notified that the road could be opened only for local traffic and construction traffic.

Seven different maturity curves were employed during the Worth County paving process due to changes in materials. This created the need for alternate material sources and characteristics in the mix. The desired TTF for the project ranged from 565 to 807. Each value related a TTF to an estimated strength of 500 psi. The differences in the values came from material supply problems that caused six changes in the mix design over the course of the paving. Once the concrete reached the adequate TTF, the contractor and owner were notified that the road could be opened only for local traffic and construction traffic.

The desired TTF for the Johnson County project was 1,716, which meant that strength of 500 psi was achieved. A verification curve was developed in August that provided a TTF of 1,391 for use after that date. Once the concrete reached the adequate TTF, the contractor and owner were notified that the road could be opened only for local traffic and construction traffic.

Weather Data

Weather data in the form of air temperatures, relative humidity, and wind speed/direction were gathered in two-hour intervals with handheld gages to help explain the growth in concrete strengths under various weather conditions. The same procedure was used for each of the four construction projects. Its purpose was to back up the maturity data and explain any abnormalities in that data. Due to the mild summer, the maturity meters provided all the data needed for this project.

Deflection Data

Falling weight deflectometer (FWD) data was only collected on the Poweshiek, Osceola, and Worth County projects.

In the case of Poweshiek County, initial FWD deflection data was collected after the bond breaker asphalt layer was placed and before the PCC overlay was placed. The initial runs in Osceola and Worth counties were conducted on the existing asphalt surfaces prior to construction. Data was collected on 0.1 mile increments in both directions in the right wheelpath and midpanel locations. A second round of FWC data was collected on the 0.1 mile longitudinal increments in the right wheelpath and midpanel location after the PCC overlay was placed.

No deflection testing was done on the Johnson County project.

Roadway Surface Mapping

A John Deere Gator utility vehicle was used to map the existing surface of the roadway on the Poweshiek, Osceola, and Worth County projects. It was equipped with a GPS receiver mounted on the top of the cab, shown in Figure 32, directly over the laser profiler. Data were streamed from both units to a central computer for storage, as shown in Figure 33. Data were using the Iowa real time network (RTN) system and its ability to correct for location and elevation.



Figure 32. A John Deere Gator equipped with a GPS receiver



Figure 33. GPS receiver and laser profiler located on the top of the John Deere Gator

The intent of this data collection was to determine the ability of the GPS system to map the existing pavement surface to a vertical accuracy sufficient for pavement overlay quantity estimation and control. The number of profiles was selected to determine how many of these key locations across the pavement were needed to accurately map the surface for overlay construction.

The method of data collection involved the use of a NEMA string that provided x,y,z coordinates of each point in latitude, longitude, and elevation. The data were then translated into state plane coordinates and elevation from sea level in feet by the ISU/GIS laboratory staff.

The initial GPS mapping data collection interval for Poweshiek County was determined from the travel speed of 12 mph to be 3–4 feet between data points. The site was broken into two segments: the southern 6 miles of north-south roadway and the north section that contained a curve and break for Interstate 80. The data were collected along each of the pavement edges, four wheelpaths, two midpanel locations, and centerline. Data were collected two times during the construction: once prior to the construction of the bond breaker asphalt layer and once after the construction of that layer. In the second data collection, the travel speed was reduced to 5 mph and the sample interval to 25 feet in an effort to improve on the accuracy of the measurements. In this case, profiling was only done on the edges and centerline of the asphalt pavement surface.

The GPS data from the post-bond breaker placement/pre-overlay phase data collection were passed to the contract surveyor for use in the development of the final gradelines and the calculation of concrete quantities.

Early attempts to survey the Osceola County project at 12 mph and 3–4-foot data point intervals proved not to be satisfactory in terms of elevation control. The next GPS data collection interval was determined from the travel speed of 5 mph to attain a sample interval to 25 feet for accuracy of the measurements. The site was surveyed in one continuous operation from end to end. The data were collected along each of the pavement edges, four wheelpaths, two midpanel locations, and centerline. Data were collected prior to construction of the PCC overlay.

The GPS data from the pre-overlay phase data collection were passed to the contract surveyor for use in the development of the final gradelines and the calculation of concrete quantities. In this case, the surveyor had a set of hubs on each side of the pavement and cross-sections at 25-foot intervals of the pavement edges, quarter points, and centerline as a comparison set of data.

GPS data for the Worth County project were collected in the same manner as for the Poweshiek County project. This project required milling of the HMA surface to attain a 2% cross-slope in each lane and remove as much of the half-inch microsurfacing as possible. After milling was completed, data were collected prior to construction of the PCC overlay at 25-foot intervals for accuracy of the measurements. Data were acquired on the centerline and two pavement edge profiles.

The Worth County GPS data from the pre-overlay/post milling phase data collection were passed to the contract surveyor for use in the development of the final gradelines and the calculation of concrete quantities. In this case, the surveyor was providing a computer model to guide the slipform paver and used the data to check the required design depths of the overlay.

In an effort to learn more about the potential for GPS surface mapping, a second ATV was equipped with a different GPS mounting, shown in Figure 34. This vehicle allowed the GPS receiver to be mounted on a 2-meter pole and wheel that floats freely from the vehicle in a PVC tube. This unit has been found to work best at 5 mph and sampling rates of every 25 feet longitudinally along the road. Data were gathered in the same x,y,z coordinates as with the first device and along the centerline and two edges of the pavement. This unit worked best by

subdividing the project into subsections, setting a base station, and communicating by phone through the base station to the RTN closest station.



Figure 34. An ATV equipped with a GPS

GPS mapping of the original pavement surface was not done in Johnson County.

Data from the Worth County project was analyzed to look at the ability to plot in 3D and use that data for computation of the quantities of concrete, final profile review and depth assurance in the overlay. The team was successful in plotting the data in 3D format in Geopac 3D software that is used in Iowa.

DATA ANALYSIS

Roadway Surface Mapping

Using the current GPS system and the associated RTN in Iowa requires a large amount of survey and GPS training. This is not a system that can be easily and quickly learned by a contractor staff or research team. The learning begins by understanding the limitations of the system. The limitations affecting the research and potential construction of pavements include but are not limited to the following:

- A lack of telephone communication speed between the data collection or paving equipment and the RTN base stations affects the development of elevation corrections.
- Impact of dense foliage or large towers, such as water towers, near the project interfere with or block the radio signals and stop or reduce the accuracy of the data collected.
- Satellite configuration and number viewed at a given time limits the accuracy and construction time that could be employed each day.
- Still-positioning with GPS is best for the development of x,y,z coordinates, and if one is moving during data collection, slower is better.
- Establishing adequate vertical control on a paving project with GPS requires that additional control points be established on each side of the project approximately $\frac{1}{4}$ to 1 mile to the right and left of the project and at 1- to 2-mile intervals along the roadway to tied-down elevations across the pavement.

The research team began this project with the assumption that data could be collected at approximately 12 mph (i.e., the maximum vehicle-sustained speed) and stored in such a manner as to collect points approximately every 4–5 feet along a given profile route using a 5 Hz data collector. The data were collected at that speed and stored with data from a Roline laser (i.e., surface profile) on the Gator. Initial work in fall 2008 utilized the base stations that were set at known control points (x,y,z) along the project and broke the data collection into 24-mile segments.

Initial analysis of the data indicated that elevations were not being accurately calculated with this method. After conferring with other GPS users, it became apparent that the GPS system worked best for stationary measurements, occupying a given point for a given amount of time to allow the satellites to triangulate a best fix solution for a given point. This was unacceptable for the purpose of this research or the construction of a pavement.

The data collection in the spring of 2009 was done with a different approach. Based on work done by a local consultant in Iowa with the same goal in mind, the team recorded data from a machine moving 5 mph, and the associated sample rate was 25 feet plus or minus. The data was compared with static data taken manually with a total station and the Iowa DOT control network of points at 1,000-foot intervals along the project.

During the course of the research, it became apparent that it could be difficult to find easy-to-use software that Iowa highway agencies would own to plot the resulting 3D profiles and analyze against a design profile of the same area. Several attempts were made to plot profiles of the 3D data. It was eventually found that the data can be plotted in some 3D versions of AutoCAD and MicroStation. These happen to be programs that are available to many highway agencies and can be used to develop both existing and final road surface profiles along with cutting cross-sections to calculate concrete overlay quantities.

The number of profiles required for the mapping of the pavement surface depends on two things: the level of overlay material yield control the highway agency expects from their design and the surface condition of the existing pavement. At a minimum, the agency should require profiles of the two pavement edges and centerline. If the asphalt surface is rutted in the wheelpaths, the pavement quarter point elevations become important in the control of where the minimum depth of overlay will occur. The depth of the wheelpath ruts also becomes important in the quantity control. To expect good overlay yield results on the part of the owner and minimize risk on the part of the contractor, it is important to have nine profiles of the pavement.

GPS surveys of the final surfaces for the Osceola, Worth, and Poweshiek projects were planned but not carried out. Telephone connection problems and communication problems with the RTN system made it very difficult to complete. This part of the work was terminated by the field research team in lieu of working with the existing data gathered to date in the surface mapping of the Worth County project.

The research team was able to do the 3D plotting in Geopac 3D software. The consultant that designed the roadway used AutoCad CivilSoft 3D and the Leica staff used Carlson 3D software. Discussions with others involved with stringless paving outside Iowa indicated that the same plotting and design checks can be done with InRoads 3D software. Three dimensional software packages for highway are capable of both plotting the existing surface and proposed surface x,y,z coordinate data, but also can develop the final profiles and the executable file for the machine control.

During the course of the research, a system was identified that is capable of doing the intended mapping of the existing pavement surface. The AMW data collection system has been used since 1995 for highway work. It was developed for the primary purpose of mobile mapping applications. It does not use a surveyor-type collector; rather, the specialized software runs on a ruggedized PC with multiple data ports. The system commonly employs a GPS rover capable of updating on 10 hz to collect data at 10-foot intervals traveling at 35 mph. If speed or distance is an issue a 20 hz rover can be used.

AMW software also supports dual slope inclinometers for rod tip correction. The software also checks longitudinal slope by calculating slope base on elevation change in travel direction. All calculations are carried out in real time. The software also includes the ability to collect data from sonar, laser, and conductivity sensors while collecting data for topography. Practically any data source with analog or digital output can be used. This data is time stamped and added to the GPS data file. The data is then used to calculate a "best fit" center line profile for milling and

paving. Horizontal and vertical curvature can be corrected as well as superelevation when edge of metal (pavement) and shoulder point profiles are included.

The same software can also be used to set control point networks if none is available. All data is calculated for use with station and cross-section reports. The software is also used for right-of-way mapping and road work staking, including slope stakes.

Concrete Quantity Development

The theoretical concrete quantities for each of the projects were developed from a template that represented the design overlay thickness across the existing slab and an additional area for widening units that were designed for in the cases of the Osceola, Worth, and Johnson County projects. The cross-slopes applied to the design surface were 1.5 to 2.0% as designated by the project engineer in each case for tangent sections. Super-elevated curve cross-slopes were adjusted to the maximum of 6% while maintaining the minimum overlay depth across the total roadway. Longitudinal centerline and edge of new pavement grades were established by the contractor representative in the Poweshiek, Osceola, and Worth projects and were reviewed and approved by the state or county owner prior to construction. In these projects, the contractor representative was encouraged to verify the existing pavement elevation at the pavement edges, quarter points, and centerline at approximately 25-foot intervals to ensure that the minimum overlay thickness was achieved. Theoretical concrete yield was calculated using the end area method with the data.

On the Osceola Iowa Highway 9 project, the contractor representatives were required to stake each side of the pavement at 25-foot intervals for the use of the slipform stringline control. Elevations of the key points across the slab were recorded during the initial survey and hub installation. They were used to ensure the minimum depth of overlay through adjustment of the vertical grades. Concrete quantities varied from the minimum depth of overlay to account for high points in the existing pavement surface.

The overlay design consultant for the Johnson County project established the grades for the single-lane paving and the theoretical concrete quantities. In this case, quantities varied from the minimum due to the quality of the subgrade in the pavement widening area. The widening rests on a newly constructed, narrow fill area that, under the best of conditions, will subside and use more than the template quantity of concrete. It is important to note that this type of widening should be properly specified in the bidding to assure that the owner and contractor risks are minimized to provide both the owner and contractor with the best unit price, using the two bid items of square yard and cubic yard.

In the cases of the Poweshiek County V-18 and Worth County US 65 projects, the contractor consultant developed a final surface with profile values at edges and centerline of pavement. The consultant utilized the as-built information for alignment and two sources of information for the elevation of the existing pavement surface. One piece of information was the GPS survey profiles that were developed at 25-foot intervals at the pavement edges, wheelpaths, quarter points, and centerline on top of the asphalt surfaces. The consultant also did his own static GPS

shots at 25-foot intervals at the pavement edges, quarter points, and centerline. By comparing the three pieces of information, the consultant was able to develop a spreadsheet of quantity values that allowed for the minimum depth of overlay and corrected minor vertical curve problems in the existing alignment. This method allowed the contractor to raise or lower the profile surface model to change the concrete yield at any given location and maintain ride values on the surface. When this is done, there will most likely be a thin spot in the concrete at an isolated location.

Geotextile Bond Breaker:

The geotextile bond breakers were utilized on this project to investigate their ability to be a positive bond breaker between the pavement layers. The materials used in this project meet the suggested national interim specifications shown in Appendix A. They also offered a thinner bond breaker layer (0.1+/- inch) versus the HMA thickness of 1 inch to achieve the same long-term performance results. This can be a major factor in locations of vertical clearance in overlay projects. This can also offer the potential for lower bond breaker layer costs of construction.

The centerline cores taken on this project verified that the material acted as a bond breaker between the two layers of concrete. In each case, the geotextile adhered to the overlay concrete and did not adhere to the existing concrete in the original pavement. Photos of those cores are shown in the Results section for the GPS sawing experiment and Appendix C. This adherence would indicate that the fabric is of sufficient weave and thickness to absorb mortar from above but not let it pass through the fabric and cause bonding of the fabric to the lower concrete layer.

Long-term performance of the geotextile bond breakers cannot be determined in this study under the study timeline. Future review of the test site's concrete performance by the InTrans staff and the Poweshiek County Engineer will provide that long-term response.

The concerns of the highway industry over the use of this material center on performance and cost. The performance has been very good in Europe, and this is one of the first overlay projects in the U.S. to use the material. It has been used in Kansas for a two-lift paving project in 2009 on top of a cement-treated base in Oklahoma on Interstate 40 in 2008 and on Route D south of Kansas City, Missouri, in 2008.

The cost of constructing this type of bond breaker was estimated from the amount of labor used on the Poweshiek project. The crew consisted of seven laborers and one foreman who were usually found doing various tasks around the paving train. Since the test sites were short in length, no dedicated crew was assigned to laying out the bond breaker. In each of the four locations, four people were assigned to handle the unrolling of the material and situating to remove air bubbles from beneath the material. Three others were assigned to fasten or nail the material to the existing surface. One of those three people was retained to deal with any bubble problems that might develop during the concrete placement. The operation was supervised by the paving foreman.

In this case, the crew was able to place two 300-foot rolls and nail them down in 1.5 hours. The average cost was \$0.35 per square yard for laborers, foreman, and equipment. The cost for the material itself was \$1.84 per square yard for the material, including freight from North Carolina, and \$0.14 per square yard for the hardware to fasten it to the slabs. This provided for a total cost of \$2.33 per square yard, compared to the price of asphalt bond breaker on this project at \$7.05 per square yard. When applied to a 5–10-mile paving project, the savings of \$4.72 per square yard, 67% is considerable.

Maturity Values

Maturity measurements in this study were made with different goals in mind. One goal was to provide the owner with the time at which the concrete reached an estimated flexural strength of 500 psi. Current specifications allow the contractor to place construction equipment on the new surface when the concrete reaches this flexural strength. The question becomes how long it takes to achieve that strength with the project's surrounding materials and environment and how that can impact the overlay progress of the project.

Research by others (Cole and Okamoto, 1995) has indicated that construction equipment could be allowed to use the slab surface when the pavement has reached 350 psi flexural strength. The 350 psi value has been used in Iowa to allow water trucks to only use the center of the slab to feed water to concrete saws for joint development. Maturity gauges on this project were monitored through the 350 psi value and to an estimated strength of greater than 500 psi. The goal was to compare the times from pavement placement to the times of joint sawing and to the two target strengths of 350 and 500 psi.

Currently joint sawing crews use a calibrated hand or shoe test to determine when to begin sawing transverse joints in a way that will allow for the use of “early entry” saws and still not ravel the edges of the joint. Many crews begin to saw when the surface of the pavement will not scuff under a hand or movement of a shoe. The first in a series of saws is used to “skip” a number of joints and provide relief to the pavement, preventing premature transverse cracking. In conventional-depth pavements, the number of skipped joints may go as high as ten or as low as five, depending on the type of aggregate in the mix. In the case of overlays (i.e., 2–5 inches in depth), this research proved that five joints should be the maximum distance between joints sawed in the “skip” process.

The research first sought to show the contractors the relationship between the maturity value for strength and time of sawing. Second, it sought to relate the strength at sawing to that required for use of the surface by construction vehicles.

Maturity TTF values were obtained for each of the sensors placed in the four pavements. The sensor software provided the basic time and temperature readings at 15-minute intervals from the concrete placement until they were stopped by the research team (usually after the estimated strength reached 500 psi flexural). The spreadsheet for each sensor was then expanded to provide a TTF for each time period and an accumulated value column. Joint saw times at each sensor were obtained from data that the sawing crew provided in writing on the slab each day. Since

multiple maturity curves were used on each project, the date and time of sensor installation was referenced to a given maturity curve and TTF target values for 350 and 500 psi. By using all parts of this information, the research team was able to develop the tables shown in Appendix D that relate the sensor number and pavement location to time from paving to joint sawing, estimated 350 and 500 psi strengths, and the estimated flexural strength at joint sawing.

All recorded values were used in the development of these tables. In the case of Johnson, Worth, and Poweshiek counties, the development of these tables resulted in one or more values of very low time and strength values at joint sawing. It is the belief of the research team that these values represent erroneous interpretation of times written on the slab.

Maturity curve values for each of the projects became specific to the concrete mix cements and coarse aggregates along with the environmental conditions at the time of concrete placement. The data from each of the projects were reviewed in terms of average and median values, maximum and minimum values, and standard deviation in values. Each of these values can help describe the strength gain characteristics of the particular mix and location.

The Poweshiek County project, which used limestone aggregate, resulted in the shortest time to joint sawing with median and mean values of five to six hours and flexural strengths of 141 to 151 psi. The 350 psi strength level was reached in less than one day and 500 psi in two to two-and-a-half days. This concrete set up quickly in the middle of the summer with very little deviation in values.

The Worth County project had time to joint sawing median and mean values of six to seven hours and strengths of 230 to 245 psi flexural strength. The 350 psi strength took nine hours, and the 500 psi value required less than 16 hours. The mix on this project proved to be a very fast strength gain material. The larger standard deviation in the values for the project can be attributed to the large changes in weather over the course of the paving. This project utilized a gravel coarse aggregate.

The use of the quartzite coarse aggregate in the Osceola County project resulted in some interesting maturity relationships. Median and mean saw time values were at seven hours, and strengths were in the 287 to 290 psi flexural range. This mix also gained strength rapidly with times of only 10 hours to 350 psi and 33 hours to 500 psi flexural strength. The combination of rapid strength gain and very strong coarse aggregate made this project a test to stay ahead of premature cracking from the strength-gain rate and still not ravel joints during sawing. This project also proved the importance of understanding the project materials when purchasing saw blades to meet the needs of the project and minimize raveling.

Maturity values taken from the Johnson County project must be considered with knowledge of the project and its timeline. The data contained several times to joint sawing that exceeded nine hours. The values that are shown are verified; however, they represent a very rainy climate that influenced rate of gain on a majority of the paving days. The contractor also was working multiple jobs with the same crew in the Iowa City area at the same time and may not have been able to saw when the concrete was ready to saw. This shows up in the mean to median values for

saw times of 7.5 to 9.31 hours and associated strengths of 179 to 250 psi flexural values. This concrete mix set at a slower rate than for the other projects and required 13 to 17 hours to achieve 350 psi and 35 to 38 hours to get to 500 psi flexural strength.

Traffic Control Methods and Results

Traffic control for all of the construction projects in this study began with a closed road and used a detour for each project. In each case, the public announcement system (e.g., newspapers and radio) were used to alert local residents of the impending delays.

The detour utilized for the Poweshiek County V-18 project stretched several miles on state and county roads. In this area, the road pattern does not provide for close parallel routes. Residents living along the route utilized ATV equipment on the shoulders and ditches to get to and from the nearest county road access (i.e., less than 1 mile). They left their cars/pickups at this access for the two to four days required by the pavement to gain maturity and by the contractor to provide temporary access to the slab by granular fills. Farmers and agricultural suppliers who required special access for material supply were accommodated by adjusting the paving schedule at their site.

In the case of the Osceola County Iowa Highway 9 work, the detour was utilized for the entire construction period until the safety line painting and signing was complete and traffic could resume along the route. The contractor provided temporary access to the completed pavement for residents living along the pavement, but only after the concrete had reached maturity. For the time during and immediately after pavement construction (i.e., two to four days), residents were able to access their homes by using ATV equipment and cars/pickups on the remaining shoulder area. This project was entirely rural in nature. Farmers along the route were able to maintain animal sale deliveries and feed supply timetables by coordinating with the contractor's paving schedule near their location.

Single-lane construction under traffic and dual-lane construction with a detour had been discussed for the Worth County US 65 project during the planning phase of this project. The contractor requested and was granted the single lane with closed road and detour option. In this case, intermediate dates were established in an effort to reduce the total detour length and time. The contract also required that the contractor maintain cross traffic at three locations and deal with oversized vehicles at one site.

Johnson County officials requested that the contractor use a combination of closed road and detour for the through traffic on W-62 during construction. Single-lane construction was chosen for the paving. Due to the fact that there was a large mobile home park, two material supplier locations, and various homes along the route, continuous through movement by local traffic was also requested by the county. The contractor built and maintained One Way signing for the adjacent residents to use. Residents were asked to park across the road in most cases during the pavement construction and development of concrete strength (i.e., two to four days). The contractor stockpiled driveway stone at each access point prior to construction and thus made it easier to construct the access upon approval of the concrete strengths. Three businesses along the

pavement were granted a special gap in the paving for one-half of their access to allow continuous movement during construction. Those gaps were later paved and the other half of the drive was used for temporary access.

Deflection Data Analysis

The deflection data were gathered on the three projects to provide a test validation of work being done by the research team on an overlay design procedure. The basic procedure was one developed for the Colorado DOT in previous years. It required some characterization of the existing pavement condition and its structural capacity. Many transportation departments employ the FWD as the tool of choice to measure existing structural capacity of the highway under design. This method, in combination with other information on the physical characteristics of asphalt layers in a composite pavement, allows the researcher to use the FWD to measure existing structural capacity.

The research team has been working to use the FWD data and known asphalt layer information to ease the data entry for the Colorado design program. This research offered a way to look at two parts of that effort:

1. What frequency of FWD testing is necessary for existing pavement surface and to characterize the structural capacity? Can this frequency be done effectively at 0.1-, 0.2-, 0.5-, or 1.0-mile increments in each lane?
2. Can before- and after-overlay construction testing verify that the deflection reduction correlates to the impact of the overlay thickness selected in the pre-construction design procedure?

The data from the pre- and post overlay construction for each of the three projects were first corrected for temperature at the time of data collection. All of the deflections were normalized to 70 ° F. Historical data on the asphalt layers for each project were collected and input into the design program along with the FWD data. Examples of the FWD data can be found in Appendix E.

Test runs of the design program were conducted for FWD data collected at 0.1, 0.2, 0.5, and 1.0 mile increments in both directions for the pre-construction and post-construction for each project.

The before and after PCC overlay deflection data was summarized in terms of maximum, minimum, average, and mean values for each of the three highways that were tested. The summary was developed for 0.1, 0.2, 0.5, and 1.0 mile increments by selecting values from the data set at the determined frequency. The results of the analysis for each construction project are shown in Tables 1, 2, 3, and 4, shown below by direction of testing and the combination of both directions. The data is compared only on the maximum deflection value under the load cell. That is the deflection that most design programs use to calculate overlay depths of asphalt or concrete.

Table 1. 0.1 mile pre/post Do comparison (deflection in mils)

Route:	Iowa Highway 9 in Osceola County								
Direction:	Northbound			Southbound			Both		
Interval:	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	4.6	0.1	4.6	6.3	0.1	6.3	4.6	0.1	4.6
Maximum:	14.7	0.4	14.3	14.3	0.2	14.1	14.3	0.2	14.1
Mean:	10.0	0.1	9.8	10.0	0.1	9.9	10.0	0.1	9.8

Route:	US-65 in Worth County								
Direction:	Northbound			Southbound			Both		
Interval:	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	3.3	0.7	3.2	3.6	0.1	3.6	3.3	0.7	3.2
Maximum:	14.5	0.3	14.2	15.4	0.3	15.1	14.5	0.3	14.2
Mean:	8.0	0.1	7.9	7.9	0.1	7.8	7.9	0.1	7.9

Route:	V-18 in Poweshiek County								
Direction:	Northbound			Southbound			Both		
Interval:	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile	0.1 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	5.7	0.1	5.6	6.4	0.1	6.0	5.7	0.1	5.6
Maximum:	17.1	0.7	16.4	20.4	0.6	19.8	17.1	0.6	16.5
Mean:	7.8	0.2	7.6	9.0	0.1	8.8	7.8	0.1	7.6

Table 2. 0.2 mile pre/post Do comparison (deflection in mils)

Route:	Iowa Highway 9 in Osceola County								
Direction:	Northbound			Southbound			Both		
Interval:	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	4.6	0.1	4.6	6.3	0.1	6.3	4.6	0.1	4.6
Maximum:	14.7	0.4	14.3	14.3	0.2	14.2	14.3	0.2	14.2
Mean:	10.0	0.1	9.8	10.0	0.1	9.8	10.0	0.1	9.8

Route:	US-65 in Worth County								
Direction:	Northbound			Southbound			Both		
Interval:	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	3.3	0.1	3.2	3.6	0.1	3.6	3.3	0.1	3.2
Maximum:	14.5	0.2	14.3	15.4	0.3	15.1	14.5	0.2	14.3
Mean:	8.0	0.1	7.9	7.9	0.1	7.8	7.9	0.1	7.8

Route:	V-18 in Poweshiek County								
Direction:	Northbound			Southbound			Both		
Interval:	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile	0.2 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	5.7	0.1	5.6	6.4	0.1	6.0	5.7	0.1	5.6
Maximum:	17.1	0.3	16.8	20.4	0.6	19.8	17.1	0.3	16.8
Mean:	7.8	0.2	7.7	9.0	0.1	8.8	7.8	0.1	7.7

Table 3. 0.5 mile pre/post Do comparison (deflection in mils)

Route:	Iowa Highway 9 in Osceola County								
Direction:	Northbound			Southbound			Both		
Interval:	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	7.0	0.1	6.9	6.5	0.1	6.4	6.5	0.1	6.4
Maximum:	14.5	0.2	14.3	14.1	0.2	13.9	14.1	0.2	13.9
Mean:	10.5	0.1	10.3	10.6	0.1	10.4	10.5	0.1	10.3

Route:	US-65 in Worth County								
Direction:	Northbound			Southbound			Both		
Interval:	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	4.2	0.1	4.1	4.4	0.1	4.4	4.2	0.1	4.1
Maximum:	12.8	0.2	12.6	11.7	0.3	11.5	11.7	0.2	11.5
Mean:	7.9	0.1	7.9	8.2	0.1	8.1	7.9	0.1	7.9

Route:	V-18 in Poweshiek County								
Direction:	Northbound			Southbound			Both		
Interval:	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile	0.5 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	5.9	0.1	5.8	6.5	0.1	6.4	5.9	0.1	5.8
Maximum:	11.8	0.3	11.5	11.9	0.2	11.8	11.8	0.2	11.6
Mean:	7.8	0.1	7.6	8.5	0.1	8.3	7.8	0.1	7.6

Table 4. 1.0 mile pre/post Do comparison (deflection in mils)

Route:	Iowa Highway 9 in Osceola County								
Direction:	Northbound			Southbound			Both		
Interval:	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	8.3	0.1	8.2	8.0	0.1	7.9	8.0	0.1	7.9
Maximum:	13.8	0.4	13.5	14.1	0.2	13.9	13.8	0.2	13.6
Mean:	10.7	0.2	10.5	10.6	0.1	10.5	10.6	0.1	10.5

Route:	US-65 in Worth County								
Direction:	Northbound			Southbound			Both		
Interval:	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	4.2	0.1	4.1	6.2	0.1	6.1	4.2	0.1	4.1
Maximum:	12.8	0.2	12.6	11.3	0.3	11.1	11.3	0.2	11.1
Mean:	8.4	0.1	8.3	8.4	0.1	8.2	8.4	0.1	8.2

Route:	V-18 in Poweshiek County								
Direction:	Northbound			Southbound			Both		
Interval:	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile	1.0 mile
Overlay:	Pre	Post	Reduction	Pre	Post	Reduction	Pre	Post	Reduction
Minimum:	5.9	0.1	5.8	6.5	0.1	6.4	5.9	0.1	5.8
Maximum:	9.84	0.2	9.6	11.9	0.2	11.7	9.84	0.2	9.7
Mean:	7.5	0.1	7.3	8.4	0.1	8.2	7.5	0.1	7.3

Differences in deflection can be identified in terms of direction of survey. This can be attributed to the type of traffic using each direction of the roadway. Heavy amounts of loaded trucks in one direction versus empty trucks in the opposite direction can result in increased deterioration and deflections in the loaded truck direction. This data indicates that both directions should be tested and reviewed separately and in combination for the determination of additional pavement overlay depth to accommodate the anticipated loadings in the future.

The differences noted in the summary values relative to testing frequency do not indicate a difference between the various intervals from 0.1 to 1.0 miles. This data indicates that the highway agency could use any of the testing frequencies for development of an overlay design. In practicality, each of these roads tested was uniform in condition and did not exhibit isolated areas of moderate to severe distress. If this type of distress had been noted during FWD testing, additional tests could have been performed at closer intervals to isolate the area in the data set for additional consideration in the design.

The data also indicate that nearly all of the deflection measured before the overlay placement was eliminated during construction. This is a further indication that the design overlay depth was correct to make the pavement act as a rigid but composite pavement after construction. Small amounts of deflection are usually present in any pavement structure under FWD testing. The best result that can be expected from construction is the reduction of those near zero as was the case in these tests.

Environmental Relationships

During construction, several items of information were collected to aid with the analysis of the development of the pavement flexural strengths and ways of dealing with the paving train length. The data collected included

- Wind speed and direction in two-hour increments, and
- Air temperature and relative humidity in two-hour increments

Often during the construction season, especially in the early spring or late fall, wide changes in temperature take place during the day and night. This affects the rate of concrete strength gain. The data gathered on each of these projects were designed to account for those types of changes if they occurred.

Each of the projects was built during the parts of the summer when large temperature changes did not occur. Rain was present during parts of the Worth, Johnson, and Osceola County projects, but did not happen during the initial curing of concrete. Therefore, rain was not a factor in the strength gain of the concrete overlays. Maturity measurements provided an adequate measure of the conditions in the overlay for this work.

Knife Application Data

The “knife” was redesigned by its inventor Bob Steffes and in conjunction with the contractor on the Johnson County Road W62 project. It was mounted on the bottom of the slipform paver pan and is shown in Figures 35, 36, and 37. The “knife” was connected to the front and back edges of the pan with clamps that held it rigidly in place during concrete operations. This also allowed the contractor to utilize the slipform paver on other projects during this construction and remove the “knife” within minutes for that purpose.



Figure 35. “Knife” mounted on the bottom of the slipform paver pan



Figure 36. “Knife” held in place with clamps



Figure 37. The “knife”

Data collection for this effort consisted of driving the finished slab approximately two weeks after the completion of the last mainline paving. Visual observations and photos were taken to verify the presence or lack of presence of the crack caused by the “knife” operation. Example photos can be found in Appendix F. At the time of this survey, the air and pavement temperatures had greatly lowered and the pavement was in a state of contraction. In all cases, the longitudinal joint appeared as an open or hairline crack and followed the proposed line or the adjacent longitudinal tining groove. There were no locations found where the crack had deviated from the intended alignment.

RESEARCH RESULTS

GPS-Controlled Longitudinal Joint Formation

The research sought to look at ways of ensuring that the placement of longitudinal joints in bonded overlays was properly aligned. In the case of the Poweshiek County project, three centerline locations of 500 feet each were identified with a GPS 2-meter pole unit. The centerline in the first site was surveyed by GPS prior to and after the overlay at intervals of 10 feet that included each transverse joint and the midpoint of the slab length throughout the 500 feet. A conventional saw was used to follow the line created by connecting the points on the overlay surface. Three centerline concrete cores were extracted from this area to verify the results. Photos of the cores (Stations 20+50, 21+00, 22+00) for the Poweshiek County project can be found in Appendix C. The centerline in the overlay surface and the underlying pavement was identical in two of the three cores and varied by 0.75 inches in the third core.

In a second location, a saw was equipped with the GPS unit and allowed to locate a line to saw from a GPS file. This site was used to demonstrate the ability of the saw to follow the GPS file points. The exact location of the original centerline could not be established as a reference for coring on this site. We were able to demonstrate the use of GPS to control the saw location by the efforts of equipment company representatives. Problems with the original GPS file caused the research team to resort to using manual methods to develop a line for the saw to follow. The line on the surface was then mapped by GPS and used as an input file for the saw, shown in Figure 38. A GPS receiver and screen were mounted on the top of a conventional early entry saw, shown in Figure 39, which was normally used by the contractor. The saw operator was given a 15 minute training session and told to keep a target type “+” on the screen lined up with a vertical line. He was able to saw the established line with no additional assistance and produced an acceptable centerline. The research team was able to estimate the cost of such a system that included a \$20-25K saw and a \$40K +/- GPS receiver unit for a total of \$60K.



Figure 38. Saw equipped with a GPS unit



Figure 39. GPS receiver and screen

A third site included the conventional manual method of splitting the pavement surface width to establish the position of the centerline to saw. Centerline cores were taken near the second sawing demonstration location as a way of verifying centerline location using manual methods. Those cores (Stations 200+50, 372+00, and 373+00) are shown in Appendix C and yielded a distance between the overlay centerline and the original pavement centerline of 0 to 2 inches, which would be unacceptable in a bonded overlay.

Centerline concrete cores were taken from the first and third sites and one additional site (Stations 36+50, 37+00, 38+00) to measure the relative ability of each centerline identification method. In this case, the distance between the overlay centerline and the original pavement centerline varied from 1 to 1.75 inches, which would not be acceptable in a bonded overlay.

In the case of the Osceola County project, no GPS centerline identification work was done prior to the overlay placement. The overlay centerline was established by splitting the width of the new overlay slab. Centerline cores were taken (Stations 190+00, 235+00, and 340+00) and are shown in Appendix C. The distance between the overlay centerline and the original centerline varied from 0 inches to a distance greater than the core radius of 2 inches, which would not be acceptable in a bonded overlay.

GPS Pavement Surface Mapping: Pros, Cons, and Limitations

Mapping of the existing pavement surface to determine the elevations of key points across the pavement was a critical part of establishing the overlay surface design and verifying that the design would provide a minimum depth of overlay at all points. This research looked at multiple variations in GPS data collection and the use of manual surveys with total stations. Those efforts provided the following results:

- Telephone communication speed limitations between the all-data collection or paving equipment and the RTN base stations in the development of elevation corrections restricts the use of GPS technology for surface mapping or slipform paver operation.
- Dense foliage or large towers such as water towers near the project can interfere with or block the radio signals and stop or reduce the accuracy of the data collected. This would also impact the operations of the slipform paver.
- Satellite configuration changes during the day limit the accuracy of the paving product elevations.
- Still-positioning with GPS is best for the development of x,y,z coordinates, and if the operator is moving during data collection, it is best to move slowly. Moving data collection requires more equipment and training than would normally be found in highway agencies and is constrained by the phone systems.
- Establishing adequate vertical control on a paving project with GPS requires that additional control points be established on each side of the project, approximately ¼ to 1 mile to the right and left of the project and at 1- to 2- mile intervals along the roadway to tied-down elevations across the pavement.
- Data collection should be obtained at 25-foot intervals to meet ride and geometric design specifications of the pavement surface, regardless of the method used. GPS collector units should travel at less than 5 mph, and data collectors with greater than 5 Hz capability would be advised.
- The Ames Engineering Vehicle arrangement and the Gomaco smoothness indicator (GSI) device offered some potential in future mapping of the pavement surfaces for overlay designs.
- Profile mapping is possible with 3D computer software, such as AutoCAD and MicroStation, for the surface profiles of the existing road surface and the design model of the overlay surface. This information can be used to develop cross-sections, check minimum overlay depths, and calculate concrete quantities for theoretical yield purposes. This was verified with the Worth County materials.

Slipform Paver Machine Control

The special provisions for the two stringless paving projects allowed for any combination of total station, laser, and GPS control systems to be used to construct the pavement overlay to Iowa DOT standard tolerances. The results of this research centered on one stringless control system, two contractors, and two slipform paving machines.

Conflicts can arise in trying to meet Iowa DOT smoothness specifications, such as concrete yield expectations, existing or improved geometrics, and being within minimum or average overlay depth requirements.

- To meet the smoothness requirements for incentive pay, one must try to have model and machine control elevations in the 0.01–0.05-foot plus or minus range.
- Concrete yield can be achieved if the contractor can trim the existing surface and build the overlay to the same grade line and the vertical tolerances as shown above.
- Decisions must be made during design and communicated to the individual designing the overlay model profiles explaining what, if any, corrections are to be made to the existing pavement surface profiles. In some cases, the existing profiles do not meet current line, grade, and cross-slope standards for highway safety. Only the pavement owner can make the solution decisions for this type of problem, and it must be done prior to contract letting and final survey. This will also greatly affect ride and concrete yield.
- Many overlay design procedures are developed for a minimum overlay thickness throughout the entire project. Others indicate an average depth through contract concrete volume limitations. It is important that the person setting the overlay surface profile know the intent of the overlay depth when dealing with the first three criteria.
- Sensor selection for the paving equipment should be left to the discretion of the contractor based on the tolerances established in the contract documents for the concrete depth and quantities.
- Depressed areas over crossroad pipes, existing short vertical curves, and existing bridge approaches require some improved geometrics. If the design follows the existing surface with a nominal depth of concrete, a bump will be created due to the model shape. The same can happen when trying to transition into an existing bridge approach or super-elevated curve. In the event of stringline control, the string is likely adjusted by eye to achieve the desired smoothness. In the case of the model, the adjustment must be considered in the design. These are areas where minimums should be extended. Experience from the Poweshiek project with making this adjustment from 150- to 300-foot minimum length curves improved the ride and should result in the improved surface performance. Similar results were achieved in Worth County when this theory was applied.

Many of the conflicting goals shown above can be worked out if the highway owner develops the profiles prior to contract letting and asks the contractor to build the design. The success of this method is dependent on the accuracy of the data used by the owner in the design and the control (x,y,z accuracy) provided to the contractor during construction.

If the final surface profile design is left to the contractor or subcontractor, as it was in these projects, it is vital that that information get to the contractor representative as soon as the contract is signed prior to the construction. In these projects, the surveying subcontractor received the information less than two months before the anticipated construction. The contracts called for a “nominal” overlay depth and had no mention of any other limiting factors other than ride quality. Concrete for these projects was paid for by square yard for placing and by cubic

yard to account for irregularities in the existing surface that could cause overruns. This type of contract resulted in overrun quantities of concrete and questions about profile grade and concrete yield. Decisions were made “in the field, on the fly” regarding those results, but future planning could avert such questions.

It is important to note that the Leica, Inc., total station system was used for the Poweshiek and Worth County projects. This system was selected due to the tight ride quality specification that was used as the target for smoothness on the project. The “zero blanking band” measurement called for the tolerance provided by total station control in conjunction with known control points at 250-foot intervals along the pavement under construction.

Specified ride quality was achieved on both projects. The Worth County project illustrated that the yield overrun could be maintained in the 100%–110% range with tight vertical control. This project also incorporated minor geometric changes to remove small vertical sag curves in the existing alignment. Much of the overrun in the Poweshiek project resulted from geometric corrections over 50-foot intervals across settlement areas at cross-road pipes in the original pavement.

The projects illustrated that the Leica, Inc., software and electronic controls worked very well for both a Gomaco and Guntert-Zimmerman slipform paver. It is important to note that these are relatively new machines that have “constant flow” hydraulic systems and up-to-date electronic controls. It is safe to say that if any slipform paver of this type and age was able to provide superior ride values using a stringline control system, it should be able to do it with the Leica, Inc., stringless system. The system is not slipform paver manufacturer specific.

It is important to note that for any control systems there are some limitations. Listed below are some limitations that apply to the Leica, Inc., system using the total stations and prisms:

- Fog or excessive dust between or on the prisms can cause the total station to shut down from not being able to center on the target.
- Prisms are located at two different heights on the slipform paver to alleviate the potential for the total station to switch targets and cause the paver to malfunction.
- Care should be taken to assure that the line of sight between the total station and the prism is maintained at all times. This means trying not to cross the line of trucks or high equipment or persons near the slipform paver.

Overlay and Stringless Paving Demonstration Openhouse

On September 2, 2009, an openhouse sponsored by the ICPA was held for this project. The openhouse was directed to showcase several overlay projects in Worth County (i.e., county and state projects) that were being constructed in 2009. Approximately 118 people from 56 public and private entities in 12 states attended the event, including representatives from DOTs, the Federal Highway Administration, county engineers, consulting engineers, and equipment suppliers. Speakers from the Flynn, Corp., Leica, Inc., Guntert-Zimmerman Construction

Division, Inc., ICPA, National CP Tech Center, and the research team gave briefings to those who were present on the various projects in the county and the stringless concept being used on the US 65 overlay.

A bus tour followed the briefings, beginning at the US 65 project to observe the stringless paving in progress and then to observe the finished overlay projects on the county system. The stringless concept was well received by those present and a very good discussion on how to make this work on their projects took place during and after the bus trip. Based on the reaction of county officials and the attendees to the open house, the future of concrete overlays and stringless technology looks very promising.

Geotextile Bond Breaker Placement

Both of the products utilized on this project offered the same constructability and demonstrated no differences in handling or performance during paving operations. Cores were taken at centerline on Sites 1 and 3 to verify the immediate performance and assist in a joint experiment. The three cores that were taken can be seen in Appendix C. In each case, the materials performed as directed. The geotextiles adhered to the overlay concrete and not to the underlying concrete surface. No tack material was recommended or used between the existing concrete surface and the bond breaker, as would be the case with the use of the HMA.



Figure 40. Concrete cores

Three advantages were found with the use of the geotextile bond breaker. The first was the reduced cost of construction. The savings of \$4.72 (67%) per square yard on this project was of direct benefit to the contracting agency budget. A second benefit was that the cost would have continued to diminish if the material had been bought in large quantities and placed by machine with less labor involved. The last benefit was in the reduced thickness of the bond breaker for situations where overlay vertical clearance was a problem, such as overhead bridges or signs. The test results show that the material was acting properly as a bond breaker. The only remaining item to be monitored is its ability to drain and its long-term performance.

Concrete Opening Strength Requirement for Local Traffic Use

The goal of this research was to try to identify an opening strength for local traffic use that is separate from the specified strength for opening to normal highway traffic. Normally, the contractor might employ two or more maturity gauges per day in the overlay. For these projects, that number was increased to one maturity gauge per 1,000 feet of paving. This allowed the contractor, the owner, and the research team to monitor the differences that occurred due to mixes and weather during paving.

The results of this maturity monitoring effort on the four projects can be summarized as follows:

- The owner and contractor can use the maturity method to both manage the project activities and reduce the overall time of construction.
- Knowing the rate of strength gain for a given mix can assist the contractor in managing sawing operations to match pavement construction rates with saw crew size and method of sawing the transverse joints.
- In each of the research cases, 350 psi flexural strength was achieved in less than 24 hours after paving. This strength should allow for the construction of temporary access points for residents along the pavement one day after paving. The net result would be that residents are only inconvenienced on the day of paving instead of multiple days.
- Current mix designs represented in this study produced 500 psi concrete in less than 48 hours after construction. This can result in shoulder construction that is two days behind the paving.
- The strength at joint sawing time is related to the course aggregate characteristics and the cement utilized. It ranged from five to nine hours in the test data under current sawing methods. The use of material characteristics and saw blade materials can reduce this time to five hours or less without introducing raveling beyond specifications.

Traffic Control for One- and Two-Lane Overlay Construction

The rural nature of three of the projects in this research and the existing road system in Iowa allowed the contractors to utilize short-notice advance communications between contractor personnel and local residents to maintain adjacent access during construction. The mild weather and residents' access to ATV equipment also contributed to work moving smoothly without traffic control problems in Poweshiek, Osceola, and Worth counties.

The traffic control plan for one-lane, one-way traffic without a continuous pilot car worked in this situation. It worked primarily due to property owner patience and removal of the through traffic to a major route only one mile west of the project. A key element in this success was the well-executed coordination with local county law enforcement to ticket drivers who did not heed the local-traffic only and one-way traffic control. This also helped discourage through traffic users. The plan provided for some interesting traffic conflict points during both night and day, but there were no known reportable accidents.

The plan was well received by local residents in terms of maintaining access during construction and the contractor staff appreciated the reduction in traffic around the paving operation due to one-way-only traffic.

Deflection Data Analysis Results

Deflection testing results from testing frequencies of 0.1, 0.2, 0.5, and 1.0 miles in each direction provided very similar results in each of three county tests. This suggests that a minimum of 1.0 mile test frequencies is sufficient for overlay design surfaces if the pavement is relatively uniform. Data should be gathered in each lane or direction to account for differences in truck loadings and traffic levels. Smaller test frequencies should be considered in areas of severe pavement distress to isolated areas that require additional strengthening in the overlay design.

Overlay Construction Operation Timing

One of the goals of the national research effort was to look at ways to shorten the length of time of the paving operation. For purposes of this research, we define the construction operation time as the time in days from when the contractor first begins any project-associated work on-site, until the day all safety devices are in place and traffic has full use of the new pavement. Next, we need to understand what happens during that time-line and determine where changes might be considered.

The following list represents the research team's view of what usually happens during this time period in terms of work operations, assuming all warning signs are in place along the project length:

1. Drainage improvement outside the pavement area, such as pipe extensions or urban intake repair or replacement
2. Utility relocation, vertically or horizontally
3. Survey control establishment or review
4. Subdrain improvements along the shoulder
5. Pavement surface patching
6. Pavement surface milling
7. Bridge approach repair or replacements
8. Intersection or shoulder earthwork reconstruction or enhancements
9. Pavement widening trench and final shoulder preparation construction
10. Pavement overlay construction
11. Shoulder and access construction
12. Safety device installation (e.g., guardrail, signs, painting)

Steps 1–3 of this list describe work that is necessary to clear the way for pavement construction and ensure that it will meet specifications when completed. These steps should be done under traffic and prior to any official working day start time. Inattention to utility relocations at this time can result in large losses of time due to unexpected utility locations, low lines, or drainage facilities that cannot be relocated prior to paving. This requires an early pre-construction

conference to coordinate each of these activities and requires assignment from the contractor and contracting authority of a person to oversee that this work proceeds in a timely manner.

Special traffic control in the way of signs, flaggers, and/or signals may be required for steps 4– 8. These steps should remain in the contract work day period. The pavement patching and bridge approach work can be done simultaneously. The subdrain work moves at a quick pace and usually should remain a stand-alone operation. Pavement surface milling is a very quick moving operation that works best under closed or flagged traffic control situations. If intersection reconstruction is required, the pavement removal and grading operations fit well in this time period.

Shoulder widening construction (step 9) is a relatively new step to the PCC overlay process. It requires the trenching for the widening unit and gets involved with a discussion of the quantity, quality, and final location of the existing shoulder materials. The need to consider the type material, quality, quantity, and overall future use of the excavated materials during the project planning stages is associated with trenching operations. The 2009 projects called for the removal of the trench widening materials and their deposit in highway owner stockpiles or in the final shoulder product. Conventional removal methods with a milling machine produce an aggregate product that often contains excessive amounts of fines from aggregate shoulders. This material is very difficult to place and compact in the final shoulder. If the material is to be used as replacement owner stockpile shoulder materials, the same problem will occur.

The designer should also consider the use of the remaining shoulder surface materials during construction. The open trench can become a drainage problem if it is not drained often during construction. Drainage cuts can then become areas that affect the paving machine pad line and the overall pavement smoothness. It may become cost-effective to remove the entire width of shoulder surface by milling or rolling over the shoulder to widen it. This technique can be very helpful in the case of narrow existing shoulders where pavement widening is to be included in the contract. Cost of removal, timing of removal, pad line opportunity and stability, and final shoulder construction timing and cost are affected by this decision.

The pavement overlay construction and shoulder construction steps (steps 10 and 11) should go hand in hand as a way to reduce construction time. Current overlay paving operations account for approximately 0.75 miles of single- or dual-lane pavement per day. Maturity measurements allow for joint development within hours after construction and construction traffic within 2–3 days in most cases. In these projects, maturity measures would have allowed construction equipment on the slab within 30 hours.

Shoulder construction time is one area that can reduce the time of total project construction. Currently, shouldering is not started until the paving is completed or nearly done. This is often associated with the relationship of the shoulder stone source location and the haul routes selected to transport this material to the project. A second part of this operation involves the selection of who (i.e., contractor or subcontractor) will haul and place the materials for the shoulders. By considering this operation at the time of project bidding, the contractor and suppliers can develop a plan that allows the shouldering material haul routes to access the finished product from the

beginning of paving and follow the paving operation so that both operations end within days of each other.

This research effort did not identify any ways to reduce the overall time for installation of safety devices (e.g., guardrail and signing) or line painting due to the existing length of time required and the placement speed. The time saved in the shoulder stone placement can be reflected in how the highway agency or subcontractor can have the signing underway during the paving on projects of significant length.

Environmental Relationships

Rapid changes in temperature between day and night and the high degrees of humidity during the day can impact the paving that is done in the early spring or late fall in Iowa. The weather during this research remained relatively uniform and did not exhibit such swings in temperature. Weather data were used only as a backup for maturity measurements in this case.

Rain played a part in the Osceola and Worth County projects in terms of delaying construction but did not change any of the outcomes. Rain and fog do not help the stringless method used to control the slipform paver. Fog covering the prisms presents a distorted target and stops operations, as can large amounts of dust. During rain, the total stations must be covered to prevent damage.

Knife Joint Former Results

The redesigned “knife” was successful in forming the longitudinal joint between the edge of the driving lane and paved shoulder on the Johnson County W-62 project for the length of the project. In this case, the pavement was constructed one lane at a time, and this joint was near the centerline of the 17-foot width, with 9 feet on the driving lane side of the joint and 8 feet on shoulder side. Consideration was given to not overworking the joint during hand finishing operations. During the majority of the project, it was not possible to finish from both sides, and therefore finishing across the joint occurred.

RESEARCH CONCLUSIONS

This research resulted in the following conclusions:

Longitudinal Joint Formation

- A conventional saw can be guided by a GPS receiver and operator to provide a joint in the overlay that is within 1 inch horizontally from the existing joint in the underlying pavement and thus control centerline cracking in bonded concrete pavements.
- Off-the-shelf GPS equipment can be mounted on early-entry or water-cooled saws to form the longitudinal joint.
- The cost of GPS-controlled saws may be justified in the paving of bonded overlays (concrete to concrete) or other applications where matching the underlying joint is critical for crack control.
- The redesigned “knife” and its location on the slipform paver successfully formed the longitudinal joint.

GPS Mapping of Pavement Surfaces for Concrete Surface Profile and Quantity Calculations

- Moving GPS mapping can be used develop the x and y coordinates of the pavement surface. At the present time, the z accuracy is not adequate for slipform paving smoothness and quantity control when gathered with a moving device.
- Enhancements in GPS data collection equipment and associated phone systems could make this a viable system in the future for pavement construction.
- Stationary or very slow moving (i.e., less than 5 mph) GPS data collection does offer the potential for pavement surface mapping at 25-foot intervals.
- GPS data collection at speeds of up to 35 mph using proprietary software and on-site base stations has been demonstrated outside Iowa. This method has produced elevation data points within the accuracy levels required for paving machine control.

Milling

- A 12-foot-wide milling head with closely spaced teeth provided very constant cross-slope and minimized the concrete that was placed in the removed surface, thus improving the overlay concrete yield values.
- Over-width milling of the trench for the widening units can improve the drainage capabilities of the trench, allow passage of the slipform sideform, provide an improved paver pad line, and improve ride in the final overlay product.

Slipform Paver Machine Control

- The stringless paving control system performed to expectations in line, grade, and cross-slope control to match the designer model for the surface of the finished

- pavement overlay on each of the two test projects.
- Regardless of the brand, modern slipform pavers that have recent constant-flow hydraulic systems and up-to-date electronic controls can be outfitted with the stringless paving system from Leica, Inc.
 - Success of the stringless system rests on the slipform paver controls and the ability to set very tight vertical control for the guidance system on the ground.
 - The control system will replicate what the designer puts into the final surface model.
 - The system that was used in this research performed equally well on one- and two-lane paving situations. In one-lane situations, the outer edge is controlled by the model on the second pass, and the centerline of the first pass is “locked” to existing elevations of the first pass.
 - Designer knowledge of the existing surface alignment and surface elevations is critical to development of the final design model.
 - GPS- and GPS/laser-controlled slipform operations do exist but were not evaluated in this research due to the decisions made by contractors in control equipment selection.

Geotextile Bond Breakers

- Geotextile bond breakers provided a \$4.72 (67%) per square yard savings in cost over traditional asphalt bond breakers on the test project.
- Geotextile bond breakers provide a positive bond break between pavement layers.
- They are easy to place with minimal training for staff prior to construction.
- The nailing plan devised for this project worked successfully.
- Allow for approximately 25%–30% extra fasteners and powder charges to do the placement.
- Daily maintenance of the fastener propellant guns is vital to the continued placement of the geotextile materials on an extended-length project.
- Consider securing the outer edges to shoulder material in windy conditions.
- Geotextile can be a positive overlay attribute in cases of vertical clearance limitations.
- Placement can be manual or with machine, but it requires a certain level of manual labor to secure and maintain alignment of the materials
- Geotextiles can be placed prior to overlay construction or during construction.
- Geotextile bond breakers resist tearing due to normal construction traffic.
- The long-term performance of these materials was not part of this research effort.

Concrete Opening Strength Requirement for Local Traffic Use

- Maturity values can be used to manage joint sawing operations and reduce overall overlay construction timelines.
- Joint sawing times ranged from five to nine hours after construction and associated strengths that were directly associated to the coarse aggregate materials.
- Flexural strengths of 350 psi were achieved in less than 24 hours for each of the test sites.
- Flexural strengths of 500 psi were achieved in less than 48 hours for each project.
- Using maturity results, temporary access can be restored 24 hours after pavement construction, and shouldering can begin after 48 hours from this data.
- Maturity values are subject to the coarse aggregate and cement characteristics and the environment that they are subjected to.
- Maturity-measured sawing time and related concrete strengths are related to coarse aggregate characteristics and the type of saw blades utilized.

Traffic Control for One- and Two-Lane Overlay Construction

- Both single- and dual-lane pavement overlay construction provided adequate traffic control to achieve the construction objectives.
- Provided traffic detours for two-lane construction can result in shorter paving time periods over one-lane construction.
- Highway owner and contractor communications with the adjacent residents is essential to the success of either single- or dual-lane overlay construction.

Overlay Construction Operation Timing

The following list represents the research team's view of areas where construction time can be reduced through innovative thinking and the use of technology in each major part of the overlay construction sequence:

1. Consider the following areas for construction prior to the charging of work days or starting time of the project. Utilize earlier preconstruction meetings with the entities involved to effect this part of the construction process:
 - Drainage improvement outside the pavement area, such as pipe extensions or urban intake repair or replacement
 - Utility relocation, vertically or horizontally
 - Survey control establishment or review
2. Conduct the following work items as part of the working day contract period. Proper management of subcontractors and use of flaggers can allow through traffic to access the project without total closure for these operations and minimize construction time:
 - Subdrain improvements along the shoulder
 - Pavement surface patching
 - Pavement surface milling

- Bridge approach repair or replacements
- Intersection or shoulder earthwork reconstruction or enhancements
- 3. For items requiring partial or total road closure to through traffic, total construction time can be reduced with two-lane paving and total closure over one-lane paving:
 - Pavement widening trench and final shoulder preparation construction
- 4. Maturity technology and project design and planning innovation can reduce times for the following operations:
 - Pavement overlay construction
 - Shoulder and access construction
- 5. Project coordination and subcontractor time availability are the only ways to reduce the following time separately from the shoulder construction:
 - Safety device installation (e.g., guardrail, signs, painting)

FWD Testing

- FWD testing in both pavement lanes prior to overlay design indicated that the testing frequency of 1.0 miles in each lane was adequate for design of the overlay.
- Additional testing at smaller frequencies of 0.1 to 0.5 miles can be used to identify and isolate spot locations of moderate to severe pavement distress that may require replacement or strengthening of the existing surface.
- FWD loads of 6, 9, and 12 kips are recommended for each test site. The 9 kip load related deflection is well suited for pavement overlay design purposes.

RECOMMENDATIONS

Longitudinal Joint Formation with GPS-Controlled Saws

- Contractors should consider the use of GPS-controlled saws as a way to replicate the existing centerline joint in a bonded overlay situation.
- Consider utilizing the new design of the “knife” and its location on the slipform paver in additional tests (5–10 projects) to verify the results of this test and consider reinstitution in all paving in Iowa. The tests should consider the “knife” for joints in new pavements, overlay centerlines, and widening joints.

GPS Pavement Surface Mapping

- Continue to investigate methods of mapping the existing pavement surfaces using existing manual techniques and combinations of GPS, lasers, ultrasonics, and radar.
- Investigate the application of proprietary software that allows the collection of existing surface profiles to be done accurately at speeds of up to 35 mph and be converted into concrete estimating quantities and machine control files.
- Utilize mapping equipment and profile development techniques that can be adapted to highway agency existing equipment and design methods.
- Pavement surveys should be conducted on 25-foot intervals for best results and 50 foot intervals as an alternative distance between measured elevations on the existing pavement to meet concrete yield expectations.
- At a minimum, existing surface profile information should be gathered prior to final design on the edges and centerline of the pavement, and additional surveys should be conducted at the quarter points and in each wheel path if the existing surface is badly rutted or exhibits lateral shoving.

Milling

- Consider the economic benefits of milling versus non-milling during the design phase and decide accordingly on its use.
- The researchers recommend that specifying a 12-foot head for removal of any existing surface and the number of teeth per foot of width can improve the overall cross-slope of the finished surface and affect the yield of the overlay concrete.
- Consider a mill head that is wider than the designed pavement widening unit to yield an improved paver pad line, better drainage of the excavated area, and the opportunity for improved ride in the final product.

Slipform Paver Machine Control

- Consider alternative forms of equipment that are available for both the development of vertical control and guidance of the slipform paver to continue this type of paving for overlays or full-depth pavements.
- Look at ways to improve concrete yield with prior planning of highway agency goals, mapping of the existing surface, and development of tight vertical control point systems along the pavement prior to design and construction.
- Consider the goals of milling in the design process. If milling is a requirement, the milling may be done to the same grade line that the overlay construction will use and use the same control devices, i.e. total stations.
- Consider the opportunity to demonstrate stringless controls with a combination of GPS, total stations, and lasers.
- Consider increasing the minimum length of vertical curves and transitions between tangents and super-elevated curves from 150 feet in the existing pavements to 300 feet in the overlays to improve ride and geometrics over buried pipes and transitions to bridges and horizontal curves.

Geotextile Bond Breaker

- Consider geotextile bond breakers as an alternative bond breaker between PCC layers.
- Recommend geotextile placement within one day prior to paving to reduce the potential for wind and traffic damage.
- Consider the positive economic benefits of alternative bond breaker materials to the contractor and contracting authority and the relative performance in making material selections.

Concrete Opening Strength Requirement for Local Traffic Use

- Consider using maturity measurements to manage joint sawing operations and reduce overall overlay construction timelines.
- Consider using the maturity concept to understand strength gain and potential joint raveling versus concrete saw blade selection for each mix overlay design.
- Consider using maturity measurements to open local resident access points less than 24 hours after paving with flexural strengths of 350 psi.
- Consider using maturity measurements to begin shouldering operations less than 48 hours after paving with flexural strengths of 500 psi.
- Consider the development of maturity value relationships that are specific to the coarse aggregate and cement characteristics and the environment they are subjected to.

Traffic Control for One- and Two-Lane Overlay Construction

- Consider analyzing the cost for through traffic and potential reduction in overlay construction costs associated with single- and dual-lane paving during the project planning phase.
- Require a preconstruction meeting between the contractor, highway owner and the adjacent property owners prior to construction in order to eliminate problems during construction.

Overlay Construction Operation Timing

A combination of good project management and good use of existing technology can reduce construction times, as shown in the following recommendations:

- Consider utilizing contract working day specifications to effect planning activities, such as utility relocations, drainage improvements outside the pavement area, and pavement survey activities prior to the official start of the pavement construction.
- Recommend utilizing good multi-tasking contract management and flaggers to maintain partial through traffic under charged working days to make longitudinal subdrain installations, conduct pavement patching, mill surfaces, replace bridge approaches, and build intersection or shoulder enhancements that are short of total removal and replacement.
- Consider utilizing total road closure to through traffic, detours, and two-lane versus one-lane overlay construction to minimize traffic delay time whenever possible. Traffic control creativity may be required in suburban and urban settings.
- Recommend the use of traffic cones to keep through traffic away from the pavement widening trench and final shoulder preparation construction.
- Recommend utilization of maturity technology, haul road selection tied to paving plans, and innovative management of the access re-establishment and shouldering operations to reduce overall overlay construction time.

FWD Testing

- FWD testing should be carried out on a frequency of at least 1.0 mile increments in each of the lanes being considered for an overlay.
- FWD testing frequencies of 0.1 to 0.5 miles should be used to identify and isolate specific areas of distress in a paving project for consideration of strengthening or replacement pavement options.
- Testing loadings should be applied at the midpanel location in the right wheelpath of each lane in increments of 6, 9, and 12 thousand pound loads. The 9 thousand pound load deflection values are recommended for use in design programs.
- Before and after overlay FWD testing should be done to measure the improvement in deflection reduction and verify overlay design depth adequacy.

FUTURE RESEARCH

Longitudinal Joint Formation for Bonded Overlays

- Consider manual measurements from hub lines to existing centerlines prior to overlay and re-establishment of those points on the surface prior to the centerline sawing of the overlay.

Surface Mapping

- Consider the use of Lidar, slow moving GPS units, or robotic total station work to accurately map the road surface prior to overlay design.

Surface Milling

- Investigate the amount of concrete required to fill the milled surface from a single lane with coarse or widely spaced cutting head and a narrowly spaced cutting head.
- Compare the yield in overlay concrete of a surface milled from the paving grade line to one that is independent of the paving grade line. The same comparison can be made between a milling unit using only GPS control to one with total station control.

Machine Control

- Investigate the potential for a printout of the actual pavement overlay surface elevations at the centerline and pavement edges of the finished product to ensure quality compliance with the paving model.
- Investigate the potential of using a combination of lasers and GPS for machine control of the slipform paver.
- Consider the use of the ski or moving stringline to achieve overlays and still maintain smoothness specifications.

Opening Strength

- Investigate the impact of shouldering on the durability of the overlay edges when the concrete has reached a flexural strength of 500, 400, and 350 psi.

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APPENDIX A. GEOTEXTILE SPECIFICATIONS AND FASTENING PLAN

Tolerances recommended for use in US material specifications are listed in the following table. Test standards common to the US (e.g., ASTM) are listed when applicable. The original (e.g., ISO, EN, and DIN) tests should be considered the standard, however, until full equivalency can be verified. A list of accredited laboratory facilities capable of conducting these tests can be found on The Geosynthetic Institute Web site (www.geosynthetic-institute.org).

A. Proposed Interim Specifications for Geotextile Interlayer Material

Property	Requirements ¹	Test Procedure ²
Geotextile Type	Nonwoven, needle-punched geotextile, no thermal treatment (calendaring or IR)	EN 13249, Annex F (Manufacturer Certification of Production)
Color	Uniform/nominally same-color fibers	(Visual Inspection) ³
Mass per unit area	$\geq 450 \text{ g/m}^2$ (13.3 oz/yd ²) $\leq 550 \text{ g/m}^2$ (16.2 oz/yd ²)	ISO 9864 (ASTM D 5261)
Thickness under load (pressure) ⁴	[a] At 2 kPa (0.29 psi): $\geq 3.0 \text{ mm}$ (0.12 in.) [b] At 20 kPa (2.9 psi): $\geq 2.5 \text{ mm}$ (0.10 in.) [c] At 200 kPa (29 psi): $\geq 1.0 \text{ mm}$ (0.04 in.)	ISO 9863-1 (ASTM D 5199)
Wide-width tensile strength ⁵	$\geq 10 \text{ kN/m}$ (685 lb/ft)	ISO 10319 (ASTM D 4595)
Wide-width maximum elongation ⁶	$\leq 130\%$	ISO 10319 (ASTM D 4595)
Water permeability in normal direction under load (pressure)	At 20 kPa (2.9 psi): $\geq 1 \times 10^{-4} \text{ m/s}$ ($3.3 \times 10^{-4} \text{ ft/s}$)	DIN 60500-4 (mod. ASTM D 5493 or ASTM D 4491)
In-plane water permeability (transmissivity) ⁷ under load (pressure)	[a] At 20 kPa (2.9 psi): $\geq 5 \times 10^{-4} \text{ m/s}$ ($1.6 \times 10^{-3} \text{ ft/s}$) [b] At 200 kPa (29 psi): $\geq 2 \times 10^{-4} \text{ m/s}$ ($6.6 \times 10^{-4} \text{ ft/s}$)	ISO 12958 (mod. ASTM D 6574 or ASTM D 4716)
Weather resistance	Retained strength $\geq 60\%$	EN 12224 (ASTM D 4355 @ 500 hrs. exposure)
Alkali resistance	$\geq 96\%$ Polypropylene/Polyethylene	EN 13249, Annex B (Manufacturer Certification of Polymer)

1 Requirements must be met for 95 percent of samples, compared to minimum average roll value (MARV) requirements commonly specified for geotextiles in the United States, which require a 97.7 percent degree of confidence (see AASHTO M 288).

2 All test procedures shown in (parentheses) are tentatively suggested for U.S. practice, but their replacement of the corresponding ISO/DIN/EN specifications should be further reviewed by geosynthetic industry experts.

3 Multi-color geotextiles may possess undesirable qualities due to a lack of uniformity.

4 Old thickness requirement was $\geq 2.0 \text{ mm}$ (0.08 in.) at 20 kPa (2.9 psi) only (ZTV Beton–StB 01).

5 Note that other measures of tensile strength commonly reported in product literature are not comparable to the results of this test procedure.

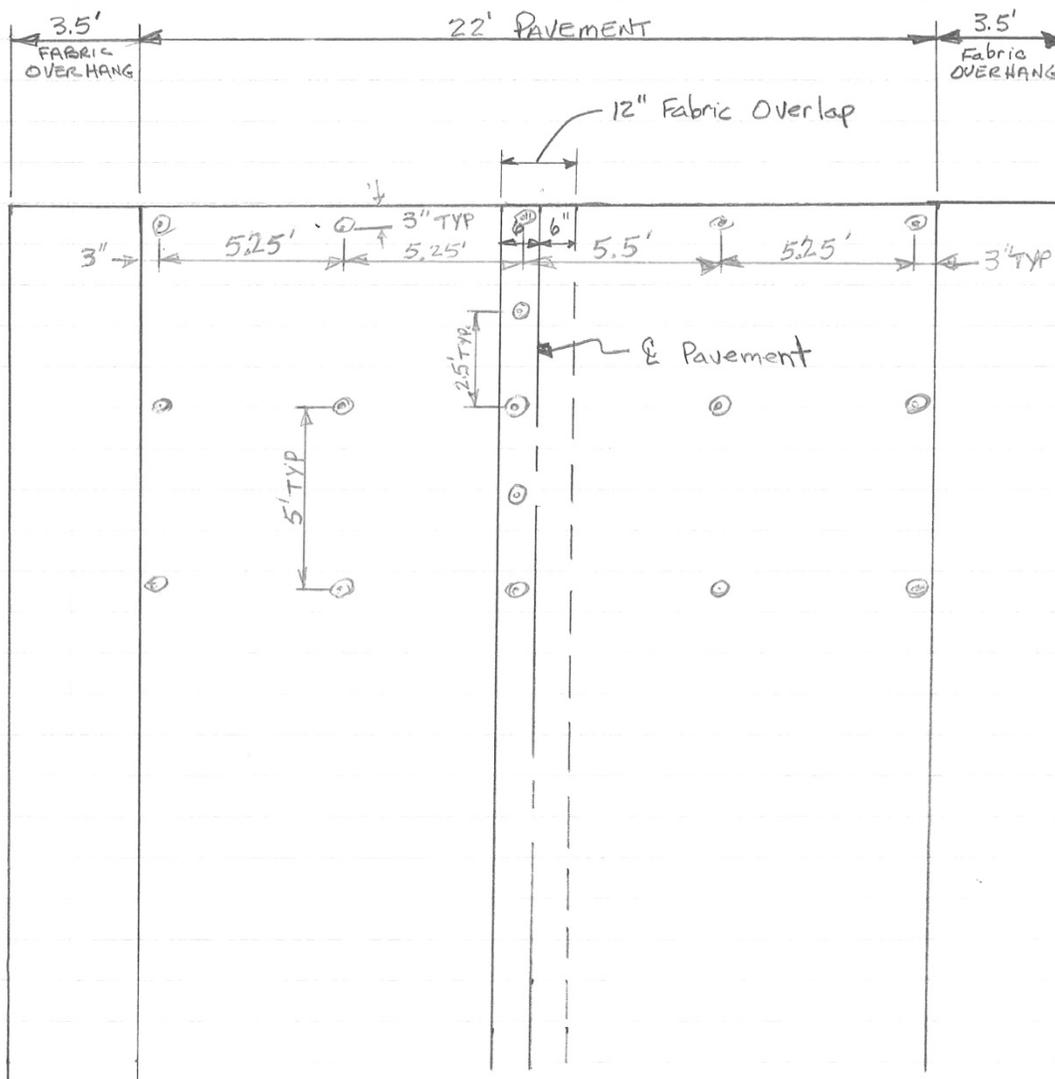
6 A maximum elongation of ≤ 60 percent is recommended as a better practice.

7 Old transmissivity requirement included only testing at 20 kPa (2.9 psi) (ZTV Beton–StB 01).

Fabric Interlayer Construction Practices

- Sweep the underlying surface to remove loose debris before applying the interlayer.
- Roll the geotextile out on the underlying layer. The geotextile should be tight and without excess wrinkles and folds. There is no specific process to roll out the layer, and numerous techniques have been used based on available equipment and labor.
- Ideally, the interlayer should be placed within 2-3 days of concrete paving to minimize damage or contamination due to weather and/or traffic.
- Driving on the interlayer should be kept to a minimum. Tight-radius turns or excessive accelerations or braking should be avoided.
- Do not place the geotextile on areas subject to excess traffic (e.g., crossovers). Installation of the geotextile should be delayed on these areas until immediately before concrete placement.
- The geotextile should be secured to underlying layer with nails punched through 2- to 2.75-in. galvanized washers/discs every 6 ft. or less.
- Additional fasteners can be used as needed to ensure that the geotextile does not shift or fold prior to concrete placement.
- Where it occurs, edges of the geotextile should overlap by no more than 12 inches.
- There should not be more than three layers of geotextile overlap at any location.
- Care should be taken to roll out the geotextile in a sequence that will facilitate good lapping practice, and that will prevent folding or tearing by construction traffic. For example, the end of a roll laid in the direction of paving should lie atop the beginning of the next roll, minimizing the potential for being disturbed by the paver.
- The free edge of the geotextile should extend beyond the edge of the new concrete and into a location that facilitates drainage.

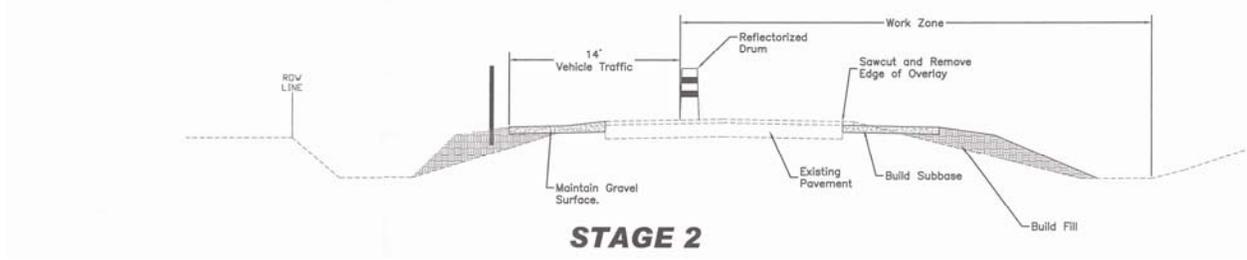
FABRIC INTERLAYER LAYOUT



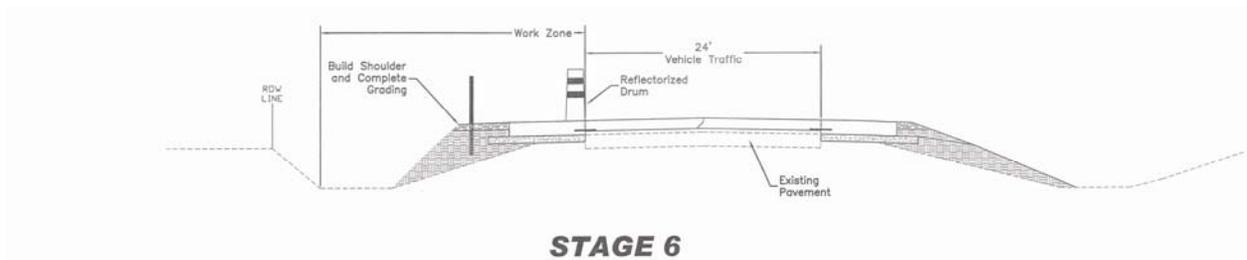
FABRIC = 15' wide x 300' long
 LONGITUDINAL LAP — No More than 12" with the top layer
 in direction of paving

Fabric Interlayer Layout

APPENDIX B. JOHNSON COUNTY CROSS-SECTION PLANS



Johnson County Construction Staging



Johnson County Construction Staging



Johnson County Construction Phases

Construction Phasing Notes:

Phase A:

Build Stage 1 and Stage 2 improvements throughout the length of the project (See Sheet J.01). Use flaggers and pilot car during construction operations. Reopen to two way traffic at the end of each day.

When ready to begin Stage 3, Close Oak Crest Hill Road from the beginning of project to south of 500th Street. Use standard road closure details. Build all improvements and open to traffic south of 500th Street. Maintain closure of Oak Crest Hill Road from 500th Street north.

Phase B:

Close Oak Crest Hill Road to through traffic. Operate as one-way southbound.

Build Stage 3 and Stage 4 improvements along east side of Oak Crest Hill Road from 500th Street to IA 923. Maintain access to the electric substation and to Izaak Walton Road (See Sheet J.03).

Phase C:

Open Oak Crest Hill Road to northbound traffic only through entire length of the project.

Prep subbase and build Stage 5 improvements along west side of Oak Crest Hill Road from 500th Street to IA 923 (See Sheet J.03).

Phase D:

Build permanent pavement markings. Open Oak Crest Hill Road to two-way traffic. Build Stage 6 improvements (See Sheet J.03).

Phase E:

Complete any remaining construction items (seeding, grading, etc.).

Johnson County Construction Phasing Notes

APPENDIX C. CONCRETE CORES



Sta 20+50



Sta 21+00



C-1

Sta 22+00



Sta 36+50



Sta 37+00



Sta 38+00



Sta 190+00



Sta 200+50



Sta 235+00



Sta 340+00



Sta 372+00



Sta 373+00

APPENDIX D. MATURITY TTF STATISTICS

Osceola County

Curve 2

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07571	191+25	3.25	216.78	8.5	31.75
N07574	197+25	6.00	295.90	9.00	33.25
N07563	207+00	8.75	326.42	10.25	35.75
N07572	837+00	8.50	324.08	10.00	32.50
N07573	847+00	5.50	281.51	9.25	32.25
N07560	857+00	7.00	299.48	10.25	32.75
N07557	867+00	6.75	284.77	10.25	32.75
N07563	877+00	2.75	172.66	9.75	33.50
N07571	890+00	6.25	293.62	9.50	32.00
N07569	900+00	7.75	303.27	11.00	33.00
N07574	910+00	6.50	288.12	9.75	32.50
N07568	920+00	8.75	347.69	9.00	31.00
N07577	935+00	3.75	208.78	10.25	35.75
N07572	945+00	4.75	245.05	10.50	33.50
N07565	955+00	6.00	269.10	10.25	34.00
N07561	965+00	5.25	257.94	10.25	34.75
N07570	975+00	5.00	249.50	10.50	34.75
N07573	985+00	5.50	255.12	10.25	35.50
N07563	995+00	6.75	294.59	10.00	35.25
N07560	1005+00	3.75	211.41	9.50	32.25
N07562	1015+00	5.25	265.90	9.50	31.50
N07564	1025+00	6.00	279.80	10.00	32.00
N07571	1035+00	N/A	N/A	10.25	33.75
N07557	1045+00	7.25	300.33	10.00	34.25

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	23	23	24	24
Average	5.96	272.69	9.91	33.34
Median	6.00	281.51	10.00	33.13
Maximum	8.75	347.69	11.00	35.75
Minimum	2.75	172.66	8.50	31.00
Std. Devi	1.66285	41.77659	0.570147	1.398393

Curve 4

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07573	217+00	9.25	199.29	15.00	26.75
N07569	227+00	7.00	289.78	11.00	37.50
N07570	247+00	5.50	270.31	9.25	31.75
N07577	257+00	5.75	289.69	9.00	30.50
N07572	267+00	8.00	329.30	9.50	32.50
N07564	277+00	6.00	276.94	10.25	33.50
N07565	287+00	9.75	353.66	9.50	32.25
N07561	297+00	10.50	360.42	9.75	32.75
N07562	307+00	8.00	307.92	10.25	33.50
N07560	317+00	10.75	368.62	9.50	32.50
N07574	327+00	14.25	400.34	9.50	32.75
N07571	337+00	2.25	146.78	10.50	33.75
N07557	347+00	7.75	308.19	10.50	36.50
N07573	357+00	N/A	N/A	10.75	37.00
N07563	767+00	10.75	331.71	12.50	39.25
N07577	777+00	7.50	287.45	11.50	36.50
N07570	787+00	7.00	287.05	11.25	34.50
N07564	797+00	8.25	308.91	11.25	34.25
N07562	807+00	7.25	302.57	10.00	33.75
N07561	817+00	8.25	335.42	9.25	31.25
N07565	827+00	8.50	335.48	9.50	30.50

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	20	20	21	21
Average	8.11	304.49	10.45	33.50
Median	8.00	308.05	10.25	33.50
Maximum	14.25	400.3425	15	39.25
Minimum	2.25	146.78	9.00	26.75
Std. Devi	2.475505	56.69241	1.377541	2.805129

Combined

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07571	191+25	3.25	216.78	8.5	31.75
N07574	197+25	6.00	295.90	9.00	33.25
N07563	207+00	8.75	326.42	10.25	35.75
N07573	217+00	9.25	199.29	15.00	26.75
N07569	227+00	7.00	289.78	11.00	37.50
N07570	247+00	5.50	270.31	9.25	31.75
N07577	257+00	5.75	289.69	9.00	30.50
N07572	267+00	8.00	329.30	9.50	32.50
N07564	277+00	6.00	276.94	10.25	33.50
N07565	287+00	9.75	353.66	9.50	32.25
N07561	297+00	10.50	360.42	9.75	32.75
N07562	307+00	8.00	307.92	10.25	33.50
N07560	317+00	10.75	368.62	9.50	32.50
N07574	327+00	14.25	400.34	9.50	32.75
N07571	337+00	2.25	146.78	10.50	33.75
N07557	347+00	7.75	308.19	10.50	36.50
N07573	357+00	N/A	N/A	10.75	37.00
N07563	767+00	10.75	331.71	12.50	39.25
N07577	777+00	7.50	287.45	11.50	36.50
N07570	787+00	7.00	287.05	11.25	34.50
N07564	797+00	8.25	308.91	11.25	34.25
N07562	807+00	7.25	302.57	10.00	33.75
N07561	817+00	8.25	335.42	9.25	31.25
N07565	827+00	8.50	335.48	9.50	30.50
N07572	837+00	8.50	324.08	10.00	32.50
N07573	847+00	5.50	281.51	9.25	32.25
N07560	857+00	7.00	299.48	10.25	32.75
N07557	867+00	6.75	284.77	10.25	32.75
N07563	877+00	2.75	172.66	9.75	33.50
N07571	890+00	6.25	293.62	9.50	32.00
N07569	900+00	7.75	303.27	11.00	33.00
N07574	910+00	6.50	288.12	9.75	32.50
N07568	920+00	8.75	347.69	9.00	31.00
N07577	935+00	3.75	208.78	10.25	35.75
N07572	945+00	4.75	245.05	10.50	33.50
N07565	955+00	6.00	269.10	10.25	34.00
N07561	965+00	5.25	257.94	10.25	34.75
N07570	975+00	5.00	249.50	10.50	34.75
N07573	985+00	5.50	255.12	10.25	35.50
N07563	995+00	6.75	294.59	10.00	35.25
N07560	1005+00	3.75	211.41	9.50	32.25
N07562	1015+00	5.25	265.90	9.50	31.50
N07564	1025+00	6.00	279.80	10.00	32.00
N07571	1035+00	N/A	N/A	10.25	33.75
N07557	1045+00	7.25	300.33	10.00	34.25

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	43	43	45	45
Average	6.96	287.48	10.16	33.42
Median	7.00	289.78	10.00	33.25
Maximum	14.25	400.34	15.00	39.25
Minimum	2.25	146.78	8.50	26.75
Std. Devi	2.32477	51.24251	1.052804	2.145953

Worth County

Curve 1

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07571	2335+00 NBL	8.25	269.03	10.75	17.75
N07567	2345+00NBL	3.50	37.04	9.25	16.00
N07564	2355+00 NBL	7.00	212.90	10.75	18.00
N07560	2365+00 NBL	7.00	217.01	10.50	17.50
N07573	2375+00 NBL	7.75	250.09	10.75	18.25
N07561	2395+00 NBL	5.25	152.06	10.00	17.25
N07557	2430+00 NBL	5.75	164.66	10.50	18.00
N07568	2440+00 NBL	7.25	203.61	11.00	17.50
N07570	2470+00 NBL	8.00	275.15	10.25	18.50
N07559	2480+00 NBL	7.00	225.31	10.50	17.75
N07572	2490+00 NBL	7.50	246.04	10.50	17.75
N07565	2500+00 NBL	9.75	315.10	11.00	18.50
N07573	2565+00 NBL	N/A	N/A	10.75	18.25
N07572	2575+00 NBL	6.25	186.29	10.00	16.75
N07570	2585+00 NBL	7.25	227.93	10.50	17.25
N07561	2595+00 NBL	7.25	259.72	10.25	17.75
N07557	2605+00 NBL	7.50	258.96	10.25	18.00
N07565	2615+00 NBL	8.00	274.68	10.25	17.75
N07573	2625+00 NBL	9.00	301.28	10.50	17.75
N07560	2635+00 NBL	4.75	75.81	10.50	17.00
N07564	2645+00 NBL	5.75	163.59	9.75	16.00

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	20	20	21	21
Average	6.99	215.81	10.40	17.58
Median	7.25	226.62	10.50	17.75
Maximum	9.75	315.10	11.00	18.50
Minimum	3.50	37.04	9.25	16.00
Std. Devi	1.458809	70.74611	0.414399	0.690712

Curve 2

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07573	2685+00 NBL	4.50	54.52	10.00	15.75
N07572	2695+00 NBL	5.50	119.39	10.25	16.25
N07560	2705+00 NBL	6.25	172.67	10.00	15.75
N07565	2715+00 NBL	6.75	203.55	10.25	16.00
N07559	2725+00 NBL	6.75	193.09	10.50	16.75
N07571	2735+00 NBL	7.25	202.12	11.00	17.25
N07569	2745+00 NBL	9.50	285.70	11.50	18.00
N07561	2765+00 NBL	4.75	77.54	9.75	15.00
N07557	2775+00 NBL	4.75	40.66	11.25	17.50

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	9	9	9	9
Average	6.22	149.92	10.50	16.47
Median	6.25	172.67	10.25	16.25
Maximum	9.50	285.70	11.50	18.00
Minimum	4.50	40.66	9.75	15.00
Std. Devi	1.58826	81.87513	0.612372	0.971825

Curve 3

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07570	2755+00 NBL	5.75	92.67	10.75	16.00
N07559	2785+00 NBL	4.00	262.41	6.25	14.50
N07561	2795+00 NBL	5.25	298.94	6.75	15.00
N07560	2805+00 NBL	7.00	359.41	6.75	14.50
N07570	2815+00 NBL	9.25	406.31	7.00	15.25
N07569	2825+00 NBL	14.25	487.95	7.00	15.25
N07567	2835+00 NBL	14.25	477.71	7.25	16.25
N07557	2845+00 NBL	N/A	N/A	7.00	15.00
N07559	2855+00 NBL	N/A	N/A	6.50	14.25
N07565	2865+00 NBL	8.50	399.82	6.75	14.25
N07561	2875+00 NBL	12.50	473.66	6.50	14.50
N07561	2885+00 NBL	10.75	445.97	6.50	14.50
N07561	2890+00 SBL	3.75	229.91	7.25	16.75
N07569	2895+00 NBL	10.50	437.70	6.75	14.75
N07570	2900+00 SBL	4.00	241.36	7.25	16.50
N07570	2905+00 NBL	15.00	498.88	7.00	15.00

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	14	14	16	16
Average	8.91	365.19	7.08	15.14
Median	8.88	403.07	6.88	15.00
Maximum	15.00	498.88	10.75	16.75
Minimum	3.75	92.67	6.25	14.25
Std. Devi	4.067468	122.4527	1.023551	0.811217

Curve 4

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07570	2780+00 SBL	5.50	223.15	8.25	14.25
N07569	2790+00 SBL	5.50	197.05	8.75	14.50
N07558	2800+00 SBL	4.75	175.83	8.25	13.50
N07562	2810+00 SBL	4.50	139.03	9.25	15.75
N07558	2820+00 SBL	3.75	130.88	8.00	13.50
N07561	2830+00 SBL	3.50	81.07	8.50	14.25
N07565	2840+00 SBL	3.75	99.31	8.50	14.25
N07570	2850+00 SBL	4.25	122.80	8.75	14.75
N07569	2860+00 SBL	4.25	114.60	8.75	14.25
N07557	2870+00 SBL	5.00	155.50	9.25	15.25
N07567	2880+00 SBL	4.75	124.13	9.50	15.50

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	11	11	11	11
Average	4.50	142.12	8.70	14.52
Median	4.50	130.88	8.75	14.25
Maximum	5.50	223.15	9.50	15.75
Minimum	3.50	81.07	8.00	13.50
Std. Devi	0.680074	42.48352	0.47194	0.737009

Curve 5

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07557	2340+00 SBL	4.25	305.89	5.50	12.25
N07567	2350+00 SBL	4.00	311.77	5.00	12.25
N07559	2360+00 SBL	4.00	310.81	5.00	12.25
N07569	2370+00 SBL	5.00	334.38	5.50	13.75
N07570	2380+00 SBL	5.50	336.22	5.75	19.00
N07570	2430+00 SBL	6.25	356.09	6.00	15.00
N07569	2449+00 SBL	7.50	382.44	6.25	15.25
N07559	2460+00 SBL	6.00	347.40	6.00	14.75
N07567	2470+00 SBL	6.25	358.01	6.00	14.50
N07562	2490+00 SBL	6.50	368.30	5.75	14.25
N07564	2500+00 SBL	N/A	N/A	5.75	14.25

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	10	10	11	11
Average	5.525	341.131	5.681818	14.31818
Median	5.75	341.81	5.75	14.25
Maximum	7.5	382.44	6.25	19
Minimum	4	305.89	5	12.25
Std. Devi	1.187025	26.00345	0.40452	1.914063

Curve 6

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07565	2450+00 NBL	N/A	N/A	18.00	28.00
N07573	2460+00 NBL	N/A	N/A	19.00	28.50
N07557	2510+00 SBL	9.00	154.39	16.25	N/A

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	1	1	3	2
Average	9.00	154.39	17.75	28.25
Median	9.00	154.39	18.00	28.25
Maximum	9.00	154.39	19.00	28.50
Minimum	9.00	154.39	16.25	28.00
Std. Devi	#DIV/0!	#DIV/0!	1.391941	0.353553

Combined

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07571	2335+00 NBL	8.25	269.03	10.75	17.75
N07567	2345+00NBL	3.50	37.04	9.25	16.00
N07564	2355+00 NBL	7.00	212.90	10.75	18.00
N07560	2365+00 NBL	7.00	217.01	10.50	17.50
N07573	2375+00 NBL	7.75	250.09	10.75	18.25
N07561	2395+00 NBL	5.25	152.06	10.00	17.25
N07557	2430+00 NBL	5.75	164.66	10.50	18.00
N07568	2440+00 NBL	7.25	203.61	11.00	17.50
N07570	2470+00 NBL	8.00	275.15	10.25	18.50
N07559	2480+00 NBL	7.00	225.31	10.50	17.75
N07572	2490+00 NBL	7.50	246.04	10.50	17.75
N07565	2500+00 NBL	9.75	315.10	11.00	18.50
N07573	2565+00 NBL	N/A	N/A	10.75	18.25
N07572	2575+00 NBL	6.25	186.29	10.00	16.75
N07570	2585+00 NBL	7.25	227.93	10.50	17.25
N07561	2595+00 NBL	7.25	259.72	10.25	17.75
N07557	2605+00 NBL	7.50	258.96	10.25	18.00
N07565	2615+00 NBL	8.00	274.68	10.25	17.75
N07573	2625+00 NBL	9.00	301.28	10.50	17.75
N07560	2635+00 NBL	4.75	75.81	10.50	17.00
N07564	2645+00 NBL	5.75	163.59	9.75	16.00
N07573	2685+00 NBL	4.50	54.52	10.00	15.75
N07572	2695+00 NBL	5.50	119.39	10.25	16.25
N07560	2705+00 NBL	6.25	172.67	10.00	15.75
N07565	2715+00 NBL	6.75	203.55	10.25	16.00
N07559	2725+00 NBL	6.75	193.09	10.50	16.75
N07571	2735+00 NBL	7.25	202.12	11.00	17.25
N07569	2745+00 NBL	9.50	285.70	11.50	18.00
N07561	2765+00 NBL	4.75	77.54	9.75	15.00
N07557	2775+00 NBL	4.75	40.66	11.25	17.50
N07570	2755+00 NBL	5.75	92.67	10.75	16.00
N07559	2785+00 NBL	4.00	262.41	6.25	14.50
N07561	2795+00 NBL	5.25	298.94	6.75	15.00
N07560	2805+00 NBL	7.00	359.41	6.75	14.50
N07570	2815+00 NBL	9.25	406.31	7.00	15.25
N07569	2825+00 NBL	14.25	487.95	7.00	15.25
N07567	2835+00 NBL	14.25	477.71	7.25	16.25
N07557	2845+00 NBL	N/A	N/A	7.00	15.00
N07559	2855+00 NBL	N/A	N/A	6.50	14.25
N07565	2865+00 NBL	8.50	399.82	6.75	14.25
N07561	2875+00 NBL	12.50	473.66	6.50	14.50
N07561	2885+00 NBL	10.75	445.97	6.50	14.50
N07561	2890+00 SBL	3.75	229.91	7.25	16.75
N07569	2895+00 NBL	10.50	437.70	6.75	14.75

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07570	2900+00 SBL	4.00	241.36	7.25	16.50
N07570	2905+00 NBL	15.00	498.88	7.00	15.00
N07570	2780+00 SBL	5.50	223.15	8.25	14.25
N07569	2790+00 SBL	5.50	197.05	8.75	14.50
N07558	2800+00 SBL	4.75	175.83	8.25	13.50
N07562	2810+00 SBL	4.50	139.03	9.25	15.75
N07558	2820+00 SBL	3.75	130.88	8.00	13.50
N07561	2830+00 SBL	3.50	81.07	8.50	14.25
N07565	2840+00 SBL	3.75	99.31	8.50	14.25
N07570	2850+00 SBL	4.25	122.80	8.75	14.75
N07569	2860+00 SBL	4.25	114.60	8.75	14.25
N07557	2870+00 SBL	5.00	155.50	9.25	15.25
N07567	2880+00 SBL	4.75	124.13	9.50	15.50
N07557	2340+00 SBL	4.25	305.89	5.50	12.25
N07567	2350+00 SBL	4.00	311.77	5.00	12.25
N07559	2360+00 SBL	4.00	310.81	5.00	12.25
N07569	2370+00 SBL	5.00	334.38	5.50	13.75
N07570	2380+00 SBL	5.50	336.22	5.75	19.00
N07570	2430+00 SBL	6.25	356.09	6.00	15.00
N07569	2449+00 SBL	7.50	382.44	6.25	15.25
N07559	2460+00 SBL	6.00	347.40	6.00	14.75
N07567	2470+00 SBL	6.25	358.01	6.00	14.50
N07562	2490+00 SBL	6.50	368.30	5.75	14.25
N07564	2500+00 SBL	N/A	N/A	5.75	14.25
N07565	2450+00 NBL	N/A	N/A	18.00	28.00
N07573	2460+00 NBL	N/A	N/A	19.00	28.50
N07557	2510+00 SBL	9.00	154.39	16.25	N/A

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	65	65	71	70
Average	6.68	244.73	8.98	16.19
Median	6.25	229.91	9.25	15.75
Maximum	15.00	498.88	19.00	28.50
Minimum	3.50	37.04	5.00	12.25
Std. Devi	2.614379	117.6251	2.67906	2.66015

Poweshiek County

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07576	7+50	4.50	151.11	14.50	40.25
N07575	12+50	3.75	118.00	15.50	43.25
N07579	17+50	2.50	59.89	15.00	42.25
N07581	22+50	4.75	166.22	15.50	43.00
N07580	27+50	5.25	162.71	16.50	43.75
N07567	32+50	4.75	158.18	16.25	42.50
N07575	CP 152	7.25	175.16	19.75	50.25
N07584	CP 156	7.25	180.93	18.75	48.75
N07578	CP 160	7.00	174.34	20.25	50.75
N07567	CP 164	5.75	145.42	20.50	49.75
N07586	CP 168	7.50	196.45	19.25	47.25
N07579	CP 172	5.25	135.02	17.50	47.25
N07582	CP 176	3.50	83.43	15.75	43.75
N07576	CP 180	3.75	101.92	17.00	47.00
N07583	CP 184	N/A	N/A	17.75	47.25
N07583	CP 188	3.25	76.76	17.75	47.25
N07586	CP 193	4.50	122.96	17.00	47.25
N07575	CP 201	4.75	151.94	16.25	45.50
N07584	CP 205	5.00	163.42	16.25	46.75
N07580	CP 209	4.50	132.71	17.00	45.25
N07578	CP 213	4.75	123.19	17.25	47.75
N07579	CP 217	5.50	161.16	16.75	46.50
N07582	CP 221	5.75	168.55	16.50	46.75
N07576	CP 225	4.25	126.26	16.50	46.00
N07583	CP 229	4.25	125.20	17.25	47.75
N07567	CP 233	6.25	172.87	18.75	50.75
N07580	CP 237	5.50	151.61	18.50	52.75
N07559	CP 241	4.00	110.96	17.25	50.00
N07584	CP 245	6.75	190.37	19.25	52.75
N07586	CP 249	7.25	187.52	21.25	55.25
N07558	CP 253	7.50	192.47	19.25	49.25
N07585	CP 257	9.75	227.23	21.00	53.50
N07575	CP261	7.50	177.73	21.75	54.25
N07581	CP 265	6.50	150.43	21.75	53.00
N07559	CP269	3.50	65.96	18.75	53.75
N07581	CP 273	5.25	128.21	19.00	53.25
N07558	CP 277	3.75	106.07	16.50	46.75
N07585	CP 281	5.00	142.88	18.50	52.50
N07586	CP 285	7.00	195.43	19.00	52.00
N07575	CP 289	7.25	192.02	20.00	52.00
N07584	CP 297	5.25	152.98	16.50	45.00
N07567	CP 301	5.25	157.90	17.50	45.75
N07583	CP 305	6.50	182.79	18.25	N/A
N07578	PT 114	6.75	173.38	18.25	47.25

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07583	PT 118	5.00	120.22	19.75	49.75
N07586	PT 122	4.75	120.32	18.75	47.75
N07585	PT 126	6.00	172.91	17.50	45.25
N07582	PT 132	5.00	131.63	17.50	47.50
N07584	PT 136	5.00	134.26	17.75	48.25
N07576	PT 140	4.50	119.33	19.25	51.00
N07575	PT 144	5.00	144.33	17.75	47.75
N07567	PT 148	6.00	176.22	17.75	48.50

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	51	51	52	51
Average	5.43	147.82	17.99	48.22
Median	5.25	151.61	17.75	47.75
Maximum	9.75	227.23	21.75	55.25
Minimum	2.50	59.89	14.50	40.25
Std. Devi	1.400963	35.46013	1.698048	3.471184

Johnson County

Curve 2

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07584	1671+00 NB	6.25	-233.68	26.25	38.50
N07585	1681+00 NB	6.75	-193.05	25.25	37.25
N07580	1690+00 NB	12.25	58.37	25.50	38.75
N07582	1695+00 NB	7.75	-137.05	26.50	N/A
N07575	1696+25 SB	6.00	-253.04	27.50	40.75
N07567	1703+50 NB	5.00	-332.39	26.50	39.00

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	6	6	6	5
Average	7.33	-181.81	26.25	38.85
Median	6.50	-213.37	26.38	38.75
Maximum	12.25	58.37	27.50	40.75
Minimum	5.00	-332.39	25.25	37.25
Std. Devi	2.572288	134.3538	0.806226	1.257478

Curve 3

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07580	1464+00 SB	6.50	240.48	12.25	33.00
N07585	1469+00 NB	7.00	238.40	13.00	35.25
N07584	1474+00 SB	7.00	248.78	12.00	32.25
N07583	1480+00 SB	7.25	255.02	12.50	33.75
N07582	1489+00 NB	1.25	-58.45	13.25	32.75
N07582	1501+00 NB	7.50	249.77	13.00	N/A
N07580	1502+00 SB	7.25	197.00	19.00	N/A
N07582	1515+75 SB	6.75	203.63	16.50	N/A
N07580	1519+50 NB	8.00	275.20	12.75	34.25
N07583	1527+50 NB	13.50	351.80	13.50	35.00
N07585	1535+00 SB	20.00	348.32	20.25	54.25
N07582	1541+50 NB	5.50	191.17	12.75	34.00
N07583	1545+00 SB	19.25	349.29	19.50	52.00
N07585	1550+75 NB	7.75	276.41	12.00	33.50
N07578	1555+00 SB	19.00	344.58	19.75	51.25
N07576	1565+00 SB	21.00	359.56	19.75	51.75
N07580	1569+50 NB	8.25	277.37	12.50	33.75
N07582	1579+00 NB	7.25	263.59	11.75	31.50
N07582	1595+00 NB	9.25	282.30	13.50	35.50

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07580	1605+00 NB	8.50	275.34	13.25	35.00
N07583	1614+50 NB	13.50	343.56	14.00	36.50
N07585	1626+75 NB	7.00	244.43	13.00	36.00
N07585	1637+00 NB	12.00	284.13	17.50	46.00
N07582	1647+00 NB	12.00	292.40	17.25	45.75
N07580	1658+00 NB	12.00	269.69	19.75	49.50
N07581	1666+60 SB	N/A	N/A	12.00	33.75
N07583	1672+00 SB	4.50	199.94	11.25	32.25
N07581	1686+00 SB	4.50	186.03	11.50	32.00

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	27	27	28	25
Average	9.75	258.88	14.61	38.42
Median	7.75	269.69	13.13	35.00
Maximum	21.00	359.56	20.25	54.25
Minimum	1.25	-58.45	11.25	31.50
Std. Devi	5.08911	82.08973	3.093773	7.68139

Combined

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07580	1464+00 SB	6.50	240.48	12.25	33.00
N07585	1469+00 NB	7.00	238.40	13.00	35.25
N07584	1474+00 SB	7.00	248.78	12.00	32.25
N07583	1480+00 SB	7.25	255.02	12.50	33.75
N07582	1489+00 NB	1.25	-58.45	13.25	32.75
N07582	1501+00 NB	7.50	249.77	13.00	N/A
N07580	1502+00 SB	7.25	197.00	19.00	N/A
N07582	1515+75 SB	6.75	203.63	16.50	N/A
N07580	1519+50 NB	8.00	275.20	12.75	34.25
N07583	1527+50 NB	13.50	351.80	13.50	35.00
N07585	1535+00 SB	20.00	348.32	20.25	54.25
N07582	1541+50 NB	5.50	191.17	12.75	34.00
N07583	1545+00 SB	19.25	349.29	19.50	52.00
N07585	1550+75 NB	7.75	276.41	12.00	33.50
N07578	1555+00 SB	19.00	344.58	19.75	51.25
N07576	1565+00 SB	21.00	359.56	19.75	51.75
N07580	1569+50 NB	8.25	277.37	12.50	33.75
N07582	1579+00 NB	7.25	263.59	11.75	31.50
N07582	1595+00 NB	9.25	282.30	13.50	35.50

Device ID	Station	Saw Time	Saw Time lb/in ²	350 lb/in ² Time	500 lb/in ² Time
N07580	1605+00 NB	8.50	275.34	13.25	35.00
N07583	1614+50 NB	13.50	343.56	14.00	36.50
N07585	1626+75 NB	7.00	244.43	13.00	36.00
N07585	1637+00 NB	12.00	284.13	17.50	46.00
N07582	1647+00 NB	12.00	292.40	17.25	45.75
N07580	1658+00 NB	12.00	269.69	19.75	49.50
N07581	1666+60 SB	N/A	N/A	12.00	33.75
N07583	1672+00 SB	4.50	199.94	11.25	32.25
N07581	1686+00 SB	4.50	186.03	11.50	32.00
N07584	1671+00 NB	6.25	-233.68	26.25	38.50
N07585	1681+00 NB	6.75	-193.05	25.25	37.25
N07580	1690+00 NB	12.25	58.37	25.50	38.75
N07582	1695+00 NB	7.75	-137.05	26.50	N/A
N07575	1696+25 SB	6.00	-253.04	27.50	40.75
N07567	1703+50 NB	5.00	-332.39	26.50	39.00

	Saw Time (hr)	Saw Time (lb/in ²)	350 lb/in ² Time (hr)	500 lb/in ² Time (hr)
Count	33	33	34	30
Average	9.31	178.75	16.66	38.49
Median	7.50	249.77	13.50	35.38
Maximum	21.00	359.56	27.50	54.25
Minimum	1.25	-332.39	11.25	31.50
Std. Devi	4.792992	195.1624	5.312893	7.005381

Brooklyn Cable NB	PCC	Excel.	None	622												
-8537	2	5877	137	125	124	113	104	83	64	48	68	79	15:07:00	CTR	None	622
-8537	3	9022	212	194	193	180	164	131	101	75	68	79	15:07:06	CTR	None	615
-8537	4	12300	293	268	248	227	182	142	106	65	68	79	15:07:13	CTR	None	606
-9537	2	5851	155	144	133	115	103	79	60	45	68	77	15:09:15	RWP	None	545
-9537	3	9043	244	228	206	183	162	124	94	70	68	77	15:09:20	RWP	None	534
-9537	4	12271	335	312	285	252	221	170	128	96	68	77	15:09:27	RWP	None	529
-9562	2	5855	126	115	118	103	96	77	61	46	68	77	15:10:12	CTR	None	673
-9562	3	9020	194	178	185	164	149	120	95	73	68	77	15:10:18	CTR	None	670
-9562	4	12313	266	244	231	226	205	165	130	100	68	77	15:10:25	CTR	None	668
-9722	2	5838	144	133	130	118	109	87	68	52	68	79	15:11:19	CTR	None	587
-9722	3	8957	224	207	204	189	171	137	107	82	68	79	15:11:25	CTR	None	578
-9722	4	12234	305	280	279	259	234	188	147	113	68	79	15:11:32	CTR	None	579
Comment at -9722	ft	Time: 15:11:57	extensive longitudinal cracking													
-10758	2	5888	122	113	115	100	92	73	56	44	68	78	15:13:30	CTR	None	699
-10758	3	9076	192	177	177	159	144	116	90	68	68	78	15:13:36	CTR	None	684
-10758	4	12388	261	242	240	218	198	159	124	94	68	78	15:13:43	CTR	None	685
-11601	2	5857	129	121	119	110	104	85	68	50	68	77	15:13:27	CTR	None	658
-11601	3	9025	201	188	185	175	163	134	106	79	68	77	15:13:33	CTR	None	650
-11601	4	12294	273	257	254	242	221	184	146	109	68	77	15:13:40	CTR	None	649
Comment at -12511	ft	Time: 15:17:46	Deflection is not decreasing													
-12511	2	5761	493	276	386	333	288	210	149	104	69	78	15:17:49	RWP	None	169
Comment at -12511	ft	Time: 15:17:54	Deflection is not decreasing													
-12511	3	8793	747	434	585	508	456	317	226	158	69	78	15:17:56	RWP	None	170
Comment at -12511	ft	Time: 15:18:03	Deflection is not decreasing													
-12511	4	11908	992	589	777	676	578	422	301	210	69	78	15:18:07	RWP	None	173
Comment at -12538	ft	Time: 15:18:49	Deflection is not decreasing													
-12538	2	5891	193	165	205	169	151	120	92	72	69	76	15:18:52	CTR	None	441
Comment at -12538	ft	Time: 15:18:57	Deflection is not decreasing													
-12538	3	9101	299	256	318	265	239	190	148	114	69	76	15:18:59	CTR	None	440
Comment at -12538	ft	Time: 15:19:06	Deflection is not decreasing													
-12538	4	12335	404	345	432	363	328	262	203	156	69	76	15:19:09	CTR	None	441
-13452	2	5877	136	125	112	103	82	62	47	36	68	78	15:20:44	CTR	None	622
-13452	3	9094	213	194	193	179	162	129	99	74	68	78	15:20:50	CTR	None	617
-13452	4	12353	292	266	265	246	223	178	137	102	68	78	15:20:57	CTR	None	612
-14518	2	5873	129	118	115	103	95	74	57	43	68	79	15:22:36	CTR	None	660
-14518	3	9076	201	185	181	165	148	117	90	68	68	79	15:22:42	CTR	None	653
-14518	4	12351	274	252	247	227	203	161	124	94	68	79	15:22:49	CTR	None	651
-15512	2	5871	118	108	106	96	86	67	49	36	68	78	15:24:38	CTR	None	718
-15512	3	9061	186	170	167	154	138	107	80	58	68	78	15:24:43	CTR	None	704
-15512	4	12329	259	235	233	213	191	149	112	81	68	78	15:24:50	CTR	None	687
-15862	2	5874	105	95	94	84	76	60	45	33	68	79	15:26:16	CTR	None	806
-15862	3	9087	165	149	147	133	120	93	70	52	68	79	15:26:21	CTR	None	794
-15862	4	12399	229	205	204	186	165	129	97	72	68	79	15:26:28	CTR	None	784
-16621	2	5822	155	131	123	103	90	65	47	33	68	79	15:28:40	RWP	None	544
-16621	3	9010	247	208	195	167	144	104	75	53	68	79	15:28:46	RWP	None	527
-16621	4	12292	346	288	272	234	201	147	105	74	68	79	15:28:53	RWP	None	513
-16646	2	5866	117	104	108	93	84	64	48	35	70	78	15:31:35	CTR	None	727
-16646	3	9081	184	164	170	149	134	102	77	56	70	78	15:31:40	CTR	None	715
-16646	4	12371	254	227	229	209	186	144	108	79	70	78	15:31:47	CTR	None	704
-17600	2	5867	130	118	121	106	97	76	59	44	69	80	15:33:22	CTR	None	651
-17600	3	9065	203	185	187	169	152	121	92	70	69	80	15:33:28	CTR	None	646
-17600	4	12375	277	252	254	232	208	166	127	96	69	80	15:33:35	CTR	None	645
-18530	2	5846	150	138	137	124	115	92	72	55	68	79	15:33:11	CTR	None	563
-18530	3	9033	234	214	213	197	180	144	112	86	68	79	15:33:17	CTR	None	558
-18530	4	12304	318	291	289	268	244	197	155	118	68	79	15:33:24	CTR	None	559
-19555	2	5850	128	116	115	102	93	72	55	41	69	83	15:37:03	CTR	None	638
-19555	3	8955	201	181	183	163	147	113	86	64	69	83	15:37:09	CTR	None	645
-19555	4	12272	275	248	246	226	202	156	119	89	69	83	15:37:16	CTR	None	644
-20488	2	5820	166	137	132	117	108	85	66	51	70	82	15:39:10	RWP	None	507
-20488	3	8974	263	216	211	189	170	135	105	81	70	82	15:39:09	RWP	None	493
-20488	4	12271	363	297	290	262	234	186	145	111	70	82	15:39:16	RWP	None	488
-20514	2	5841	118	108	108	99	92	75	60	47	71	81	15:39:59	CTR	None	714
-20514	3	8972	185	168	170	158	144	118	94	74	71	81	15:40:04	CTR	None	702

Brooklyn Cable NB	PCC	Excel.	None	417												
D -39473	2	5839	202	188	176	159	145	116	89	66	71	83	16:20:06	RWP	None	417
D -39473	3	9027	323	300	279	254	230	183	140	103	71	83	16:20:11	RWP	None	404
D -39473	4	12293	447	416	385	353	317	252	193	141	71	83	16:20:18	RWP	None	397
D -39498	2	5804	149	138	137	127	117	95	75	57	71	82	16:21:21	CTR	None	563
D -39498	3	8987	232	215	213	199	183	149	118	89	71	82	16:21:26	CTR	None	559
D -39498	4	12378	317	294	294	275	251	206	163	123	71	82	16:21:33	CTR	None	563
D -40545	2	5802	121	110	109	98	91	72	56	44	71	81	16:23:52	CTR	None	695
D -40545	3	9042	188	173	169	156	142	113	89	69	71	81	16:23:58	CTR	None	693
D -40545	4	12342	257	235	232	215	193	154	122	93	71	81	16:24:05	CTR	None	694
D -42256	2	5820	139	132	123	112	106	87	71	56	72	83	16:26:40	RWP	None	604
D -42256	3	9056	219	208	195	182	168	139	112	88	72	83	16:26:46	RWP	None	597
D -42256	4	12373	301	285	269	251	230	192	154	121	72	83	16:26:53	RWP	None	594
D -42281	2	5861	125	115	112	104	96	78	63	48	72	82	16:27:39	CTR	None	679
D -42281	3	9023	195	181	177	166	151	124	98	75	72	82	16:27:44	CTR	None	668
D -42281	4	12332	268	247	245	228	208	171	136	105	72	82	16:27:52	CTR	None	666
D -43148	2	5837	132	123	120	110	102	82	65	49	74	83	16:29:26	CTR	None	638
D -43148	3	8981	207	193	189	176	161	129	102	77	74	83	16:29:36	CTR	None	628
D -43148	4	12308	289	269	265	248	225	184	145	110	74	83	16:29:43	CTR	None	615
D -44228	2	5832	112	104	103	95	87	71	58	46	73	81	16:31:28	CTR	None	752
D -44228	3	9014	174	162	160	148	137	113	91	72	73	81	16:31:33	CTR	None	747
D -44228	4	12387	239	221	221	207	189	157	126	99	73	81	16:31:40	CTR	None	748
D -45275	2	5826	146	137	125	113	104	81	65	48	73	82	16:33:16	RWP	None	577
D -45275	3	8998	232	218	199	183	164	131	103	78	73	82	16:33:22	RWP	None	560
D -45275	4	12290	320	301	274	251	227	182	143	109	73	82	16:33:29	RWP	None	554
D -45302	2	5817	146	133	133	121	112	90	71	53	74	82	16:34:25	CTR	None	576
D -45302	3	9006	230	212	212	196	179	144	114	86	74	82	16:34:31	CTR	None	565
D -45302	4	12318	317	291	293	272	247	202	159	121	74	82	16:34:38	CTR	None	561
D -46380	2	5824	138	129	128	117	111	92	75	60	73	83	16:36:18	CTR	None	610
D -46380	3	9009	216	202	203	188	174	145	118	94	73	83	16:36:24	CTR	None	603
D -46380	4	12301	295	274	275	257	238	199	163	129	73	83	16:36:31	CTR	None	602
D -47312	2	5821	124	116	111	102	94	75	59	44	72	83	16:37:59	CTR	None	677
D -47312	3	9010	194	181	175	164	148	119	93	70	72	83	16:38:04	CTR	None	670
D -47312	4	12315	266	248	242	225	204	166	130	98	72	83	16:38:11	CTR	None	668
D -48230	2	5777	190	154	154	134	114	71	50	37	73	79	16:39:56	RWP	None	440
D -48230	3	8946	302	248	248	217	183	115	81	60	73	79	16:40:02	RWP	None	428
D -48230	4	12240	419	345	345	301	255	162	114	83	73	79	16:40:09	RWP	None	422
C Comment at -48230 ft	Time: 16:40:55	:full	depth	patch	1	6	7	not on patch								
D -48255	2	5885	121	115	106	95	84	63	46	32	74	82	16:41:35	CTR	None	703
D -48255	3	9117	189	181	166	150	134	101	73	51	74	82	16:41:40	CTR	None	695
D -48255	4	12397	261	249	230	208	186	142	103	71	74	82	16:41:47	CTR	None	687
D -49362	2	5831	139	129	128	118	111	88	69	51	73	84	16:43:34	CTR	None	605
D -49362	3	9047	220	204	204	190	175	142	111	81	73	84	16:43:39	CTR	None	594
D -49362	4	12379	300	279	279	261	239	197	153	112	73	84	16:43:46	CTR	None	595
D -50336	2	5853	112	108	102	92	86	67	53	40	72	83	16:45:32	CTR	None	752
D -50336	3	9034	173	166	157	146	133	106	82	62	72	83	16:45:37	CTR	None	755
D -50336	4	12370	237	224	215	199	181	147	113	86	72	83	16:45:44	CTR	None	754
D -50808	2	5828	110	102	99	90	82	65	52	39	73	83	16:47:18	CTR	None	768
D -50808	3	9033	173	161	156	145	131	106	82	62	73	83	16:47:24	CTR	None	756
D -50808	4	12323	236	220	216	200	181	147	115	87	73	83	16:47:30	CTR	None	754

Brooklyn Northbound, November 2009

Brooklyn NB 17Nov09

IKUAB FWD FILE : Brooklyn NB 17Nov09.fwd

HProject No. : Cable Overlay
 HLocation : hwy 17 nb
 HClient : Cable
 HStart Station :
 HDirection :
 HEnd Station :
 HWeather : sunny clear
 HOperator : hg

ILdate Created : 11/11/2009
 IVersion : 2.3.11
 ILoad Mode : 1 (SHRP 8+8 buffers, 0 plates)
 IPlate Radius : 5.91 (in)
 IExtra Field Set : Example Road
 IDrop Sequence : 2123
 ING of drops : 1111
 IRecord Drop? : NYYY
 IDrop Height : 1 2 3 4
 IImpact Load : 6003 9005 12007 16009 1bf
 ISensor Number : 0
 ISensor Distance : 0.00 12.00 12.00 18.00 24.00 36.00 48.00 60.00 8.00 0.00 (in)
 ISensor Position : CENTER FRONT BEHIND BEHIND

IReference offset : 0 ft
 ITestpoint spacing : 500 ft

J	IDistance	Imp	Load	D0	D1	D2	D3	D4	D5	D6	D7	Air	Pave	Time	Location	Type	Condition	Distress	Surface	
	ft	Num	Ibf	mils	°F	°F	end													
D	491	2	6266	2.23	2.09	1.98	1.88	1.79	1.60	1.40	1.17	59	60	12:29:29	RWP	PCC	Excel.	None	1598	
D	491	3	9464	3.37	3.17	3.03	2.91	2.74	2.44	2.12	1.81	59	60	12:29:35	RWP	PCC	Excel.	None	1596	
D	491	4	12603	4.49	4.21	4.08	3.88	3.69	3.27	2.86	2.44	59	60	12:29:41	RWP	PCC	Excel.	None	1595	
D	1013	3	6291	3.36	3.30	3.07	2.96	2.85	2.57	2.25	1.97	59	62	12:31:29	RWP	PCC	Excel.	None	1064	
D	1013	3	9455	5.08	4.99	4.71	4.53	4.35	3.91	3.46	3.01	59	62	12:31:34	RWP	PCC	Excel.	None	1059	
D	1013	4	12611	6.82	6.63	6.35	6.09	5.83	5.23	4.68	4.07	59	62	12:31:41	RWP	PCC	Excel.	None	1051	
D	1016	2	6263	4.56	4.37	4.18	4.00	3.81	3.46	3.05	2.64	59	61	12:32:32	RWP	PCC	Excel.	None	781	
D	1016	3	9431	6.72	6.56	6.37	6.19	6.00	5.62	5.18	4.72	59	61	12:32:37	RWP	PCC	Excel.	None	798	
D	1016	4	12567	8.82	8.62	8.41	8.21	8.01	7.56	7.12	6.68	59	61	12:32:44	RWP	PCC	Excel.	None	810	
C	Comment at 1016. ft Time: 12:32:54 :load transfer																			
D	1522	2	6245	2.37	2.21	2.18	2.09	2.10	1.94	1.64	1.43	60	62	12:34:21	RWP	PCC	Excel.	None	1499	
D	1522	3	9460	3.55	3.33	3.37	3.32	3.18	2.91	2.54	2.15	60	62	12:34:27	RWP	PCC	Excel.	None	1515	
D	1522	4	12625	4.72	4.40	4.52	4.34	4.24	3.88	3.38	2.88	60	62	12:34:34	RWP	PCC	Excel.	None	1522	
D	1995	2	6259	4.03	3.84	3.58	3.36	3.22	2.83	2.36	2.02	60	60	12:36:01	RWP	PCC	Excel.	None	883	
D	1995	3	9428	6.09	5.83	5.46	5.24	4.92	4.29	3.65	3.09	60	60	12:36:07	RWP	PCC	Excel.	None	880	
D	1995	4	12595	8.13	7.76	7.35	7.00	6.62	5.76	4.93	4.13	60	60	12:36:14	RWP	PCC	Excel.	None	881	
C	Comment at 1995. ft Time: 12:36:24 :Test on panel with core hole (middle hole s end)																			
D	2651	2	6221	1.95	1.88	1.79	1.67	1.61	1.54	1.36	1.21	61	61	12:37:47	RWP	PCC	Excel.	None	1815	
D	2651	3	9382	2.91	2.82	2.69	2.62	2.51	2.30	2.06	1.82	61	61	12:37:53	RWP	PCC	Excel.	None	1831	
D	2651	4	12542	3.86	3.73	3.57	3.48	3.35	3.09	2.78	2.47	61	61	12:38:00	RWP	PCC	Excel.	None	1848	
D	2999	2	6273	2.30	2.21	2.15	2.05	2.00	1.88	1.68	1.53	60	60	12:39:09	RWP	PCC	Excel.	None	1549	
D	2999	3	9429	3.44	3.30	3.20	3.16	3.04	2.81	2.54	2.29	60	60	12:39:14	RWP	PCC	Excel.	None	1559	
D	2999	4	12604	4.55	4.36	4.30	4.18	4.08	3.77	3.45	3.06	60	60	12:39:21	RWP	PCC	Excel.	None	1575	
D	3513	3	6175	2.28	2.18	2.04	1.92	1.82	1.61	1.36	1.20	61	60	12:40:40	RWP	PCC	Excel.	None	1539	
D	3513	3	9315	3.46	3.33	3.11	2.99	2.79	2.44	2.13	1.81	61	60	12:40:45	RWP	PCC	Excel.	None	1529	
D	3513	4	12517	4.67	4.46	4.23	4.03	3.80	3.30	2.90	2.46	61	60	12:40:51	RWP	PCC	Excel.	None	1525	
D	4019	2	6226	1.97	1.90	1.78	1.69	1.64	1.50	1.32	1.18	61	60	12:42:12	RWP	PCC	Excel.	None	1790	
D	4019	3	9367	2.98	2.87	2.68	2.61	2.50	2.23	2.01	1.78	61	60	12:42:22	RWP	PCC	Excel.	None	1815	
D	4019	4	12605	3.95	3.81	3.61	3.49	3.34	3.01	2.72	2.37	61	60	12:42:29	RWP	PCC	Excel.	None	1815	
D	4521	2	6260	2.19	2.08	2.00	1.88	1.86	1.68	1.50	1.32	61	61	12:43:55	RWP	PCC	Excel.	None	1629	
D	4521	3	9415	3.30	3.15	3.03	2.93	2.80	2.55	2.28	2.02	61	61	12:44:01	RWP	PCC	Excel.	None	1622	
D	4521	4	12612	4.40	4.19	4.09	3.91	3.76	3.45	3.09	2.75	61	61	12:44:08	RWP	PCC	Excel.	None	1629	
D	5019	2	6226	2.44	2.32	2.22	2.07	2.03	1.81	1.56	1.38	62	62	12:45:32	RWP	PCC	Excel.	None	1449	
D	5019	3	9378	3.68	3.51	3.37	3.26	3.13	2.83	2.43	2.08	62	62	12:45:38	RWP	PCC	Excel.	None	1448	
D	5019	4	12607	4.93	4.70	4.56	4.33	4.15	3.70	3.26	2.84	62	62	12:45:44	RWP	PCC	Excel.	None	1454	
D	5023	2	6219	3.35	3.23	3.07	2.93	2.76	2.43	2.08	1.73	61	61	12:46:44	RWP	PCC	Excel.	None	1054	

Brooklyn NB 17Nov09	1292	None	Excel.	PCC	None	1292
D 15039	4 12550	5.52	4.83	4.56	4.25	3.11
D 15043	2 6134	4.01	3.00	2.62	2.40	1.38
D 15045	3 9285	6.01	3.99	4.55	4.10	2.38
D 15045	4 12478	7.96	5.32	6.12	5.43	3.25
C Comment at 13:045	ft Time: 13:22:25 :load Transfer					
D 15504	2 6163	1.98	1.88	1.67	1.62	1.26
D 15504	3 9358	3.03	2.87	2.74	2.64	1.91
D 15504	4 12591	4.08	3.87	3.73	3.54	2.63
D 16076	2 6228	2.20	1.96	2.04	1.85	1.28
D 16076	3 9403	3.34	3.00	3.09	2.73	2.00
D 16076	4 12643	4.48	4.02	4.17	3.91	2.72
C Comment at 16076	ft Time: 13:25:50 :N street 460th					
D 16497	2 6192	1.61	1.54	1.40	1.39	1.12
D 16497	3 9334	2.46	2.34	2.25	2.11	1.72
D 16497	4 12567	3.32	3.15	3.08	2.88	2.35
D 17004	2 6165	1.84	1.75	1.68	1.53	1.22
D 17004	3 9334	2.80	2.66	2.55	2.46	1.86
D 17004	4 12560	3.74	3.55	3.46	3.18	2.52
D 17507	2 6201	2.14	2.01	1.87	1.82	1.49
D 17507	3 9390	3.26	3.08	3.03	2.89	2.26
D 17507	4 12578	4.32	4.08	4.03	3.89	3.03
D 18016	2 6199	2.10	2.01	1.90	1.76	1.32
D 18016	3 9371	3.18	3.06	2.90	2.75	2.04
D 18016	4 12539	4.25	4.09	3.90	3.70	2.78
D 18495	2 6212	2.80	2.67	2.57	2.36	2.04
D 18495	3 9416	4.24	4.04	3.87	3.76	2.85
D 18495	4 12641	5.66	5.40	5.22	5.02	3.84
D 19048	2 6234	2.05	1.98	1.81	1.72	1.27
D 19048	3 9387	3.14	3.02	2.77	2.70	1.96
D 19048	4 12596	4.22	4.06	3.76	3.62	2.67
D 19511	2 6182	2.46	2.38	2.21	2.05	1.59
D 19511	3 9373	3.71	3.61	3.34	3.21	2.40
D 19511	4 12574	4.94	4.81	4.47	4.22	3.22
D 20003	2 6200	2.75	2.37	2.24	2.12	1.63
D 20003	3 9372	3.75	3.59	3.42	3.29	2.46
D 20003	4 12598	5.01	4.80	4.60	4.41	3.35
D 20023	2 6189	3.17	2.62	2.85	2.41	1.58
D 20023	3 9360	4.83	3.96	4.06	3.77	2.45
D 20023	4 12540	6.47	5.29	5.46	5.04	3.43
C Comment at 20023	ft Time: 13:40:28 :Load Transfer					
D 20505	2 6210	1.75	1.63	1.57	1.47	1.17
D 20505	3 9400	2.63	2.46	2.41	2.35	1.78
D 20505	4 12602	3.52	3.29	3.25	3.11	2.44
D 21004	2 6181	2.21	2.10	2.00	1.89	1.44
D 21004	3 9311	3.35	3.19	3.05	2.99	2.23
D 21004	4 12549	4.45	4.23	4.10	3.98	3.01
C Comment at 21004	ft Time: 13:44:16 :Center 450th ave					
D 21481	2 6143	3.19	3.01	2.90	2.69	1.79
D 21481	3 9320	4.88	4.60	4.44	4.24	3.21
D 21481	4 12487	6.54	6.14	5.98	5.68	4.53
D 22045	2 6163	3.65	3.46	3.36	3.16	2.57
D 22045	3 9347	5.58	5.29	5.13	4.87	3.32
D 22045	4 12546	7.46	7.07	6.90	6.57	4.49
D 22517	2 6184	2.92	2.55	2.62	2.34	1.59
D 22517	3 9324	4.43	3.90	4.03	3.77	2.46
D 22517	4 12540	5.95	5.24	5.04	4.67	3.30
D 23000	2 6202	2.28	2.18	2.07	1.91	1.59
D 23000	3 9443	3.46	3.29	3.17	2.94	2.40
D 23000	4 12616	4.60	4.38	4.28	4.13	3.27
D 23498	2 6176	1.97	1.91	1.69	1.61	1.23
D 23498	3 9353	3.00	2.94	2.79	2.58	1.90
D 23498	4 12514	4.01	3.90	3.64	3.48	2.60
D 23998	2 6198	1.85	1.72	1.69	1.58	1.21
D 23998	3 9408	2.81	2.62	2.56	2.48	1.83
D 23998	4 12626	3.77	3.51	3.47	3.34	2.50
D 24522	2 6177	2.04	1.92	1.85	1.73	1.27

24522	D	9361	3-13	2-93	2-85	2-75	2-57	2-27	1-97	1-67	64	Brooklyn NB 17Nov09	PCC	Excel.	None	1702
24522	D	9361	3-13	2-93	2-85	2-75	2-57	2-27	1-97	1-67	64	66 13:55:48 RNP	PCC	Excel.	None	1702
24522	D	12558	4-22	3-93	3-88	3-68	3-49	3-08	2-70	2-28	64	66 13:55:55 RNP	PCC	Excel.	None	1698
25016	D	6187	2-20	2-19	1-99	1-87	1-83	1-61	1-41	1-22	64	63 13:57:06 RNP	PCC	Excel.	None	1602
25016	D	9363	3-36	3-36	3-04	2-95	2-83	2-50	2-19	1-90	64	63 13:57:11 RNP	PCC	Excel.	None	1586
25016	D	12613	4-50	4-49	4-13	3-95	3-78	3-39	2-98	2-60	64	63 13:57:18 RNP	PCC	Excel.	None	1592
25023	D	6173	3-24	2-58	2-41	2-25	2-25	1-82	1-47	1-18	63	63 13:58:03 RNP	PCC	Excel.	None	1084
25023	D	9334	4-95	3-92	4-12	3-80	3-44	2-79	2-25	1-85	63	63 13:58:09 RNP	PCC	Excel.	None	1072
25023	D	12322	6-64	5-34	5-18	5-11	4-65	3-77	3-07	2-51	63	63 13:58:16 RNP	PCC	Excel.	None	1072
Comment	at	25023	ft	Time: 13:58:27	Load Transfer											
25549	D	6197	3-29	2-95	2-82	2-55	2-44	2-05	1-70	1-36	64	62 14:00:02	PCC	Excel.	None	1073
25549	D	9387	5-00	4-47	4-31	4-07	3-74	3-13	2-58	2-07	62	62 14:00:08	PCC	Excel.	None	1068
25549	D	12585	6-68	5-98	5-77	5-36	5-23	4-22	3-49	2-79	64	62 14:00:15	PCC	Excel.	None	1072
26120	D	6185	2-58	2-48	2-39	2-27	2-23	1-99	1-75	1-53	64	63 14:01:30 RNP	PCC	Excel.	None	1366
26120	D	9366	3-90	3-74	3-65	3-58	3-41	3-05	2-72	2-36	64	63 14:01:37 RNP	PCC	Excel.	None	1364
26120	D	12571	4-23	4-98	4-92	4-70	4-55	4-14	3-67	3-19	64	63 14:01:45 RNP	PCC	Excel.	None	1367
26562	D	6181	1-98	1-89	1-82	1-73	1-70	1-55	1-40	1-24	63	62 14:03:11 RNP	PCC	Excel.	None	1777
26562	D	9359	2-96	2-85	2-76	2-75	2-59	2-37	2-13	1-88	63	62 14:03:16 RNP	PCC	Excel.	None	1800
26562	D	12583	3-93	3-76	3-68	3-57	3-48	3-17	2-87	2-53	63	62 14:03:23 RNP	PCC	Excel.	None	1821
27033	D	6187	2-60	2-41	2-26	2-07	1-95	1-55	1-34	1-12	63	60 14:04:42 RNP	PCC	Excel.	None	1354
27033	D	9375	4-00	3-65	3-45	3-26	2-97	2-53	2-10	1-72	63	60 14:04:47 RNP	PCC	Excel.	None	1353
27033	D	12601	5-38	4-90	4-66	4-34	4-04	3-44	2-86	2-35	63	60 14:04:54 RNP	PCC	Excel.	None	1331
27507	D	6185	2-02	1-92	1-86	1-74	1-74	1-57	1-37	1-19	63	60 14:06:10 RNP	PCC	Excel.	None	1740
27507	D	9349	3-07	2-90	2-83	2-78	2-63	2-40	2-10	1-84	63	60 14:06:16 RNP	PCC	Excel.	None	1732
27507	D	12603	4-12	3-89	3-83	3-69	3-57	3-24	2-88	2-50	63	60 14:06:24 RNP	PCC	Excel.	None	1738
27989	D	6175	1-94	1-83	1-74	1-65	1-60	1-42	1-23	1-07	63	62 14:07:39 RNP	PCC	Excel.	None	1805
27989	D	9341	2-97	2-81	2-69	2-60	2-49	2-21	1-92	1-65	63	62 14:07:44 RNP	PCC	Excel.	None	1789
27989	D	12541	3-99	3-77	3-64	3-51	3-35	3-00	2-63	2-25	63	62 14:07:51 RNP	PCC	Excel.	None	1789
28557	D	6156	1-91	1-81	1-72	1-59	1-58	1-41	1-21	1-07	63	62 14:09:15 RNP	PCC	Excel.	None	1835
28557	D	9345	2-89	2-72	2-64	2-53	2-42	2-16	1-87	1-65	63	62 14:09:20 RNP	PCC	Excel.	None	1826
28557	D	12571	3-89	3-63	3-56	3-43	3-26	2-93	2-59	2-24	63	62 14:09:25 RNP	PCC	Excel.	None	1839
29006	D	6173	2-35	2-25	2-12	1-97	1-94	1-70	1-48	1-28	64	62 14:10:48 RNP	PCC	Excel.	None	1496
29006	D	9364	3-58	3-48	3-24	3-12	2-95	2-51	2-29	1-98	64	64 14:10:53 RNP	PCC	Excel.	None	1489
29006	D	12592	4-79	4-63	4-36	4-13	3-97	3-52	3-11	2-67	64	64 14:11:00 RNP	PCC	Excel.	None	1495
29535	D	6172	3-21	2-98	2-66	2-38	2-21	1-80	1-43	1-12	64	64 14:12:14 RNP	PCC	Excel.	None	1093
29535	D	9363	4-92	4-13	4-11	3-75	3-40	2-77	2-22	1-73	64	64 14:12:19 RNP	PCC	Excel.	None	1082
29535	D	12561	6-65	5-58	5-58	5-04	4-61	3-78	3-03	2-34	64	64 14:12:26 RNP	PCC	Excel.	None	1075
30044	D	6205	2-09	1-98	1-88	1-76	1-71	1-51	1-32	1-16	64	63 14:13:44 RNP	PCC	Excel.	None	1687
30044	D	9330	3-16	3-04	2-85	2-80	2-63	2-34	2-08	1-79	64	63 14:13:50 RNP	PCC	Excel.	None	1678
30044	D	12532	4-23	4-05	3-87	3-71	3-55	3-20	2-83	2-43	64	63 14:13:57 RNP	PCC	Excel.	None	1686
30065	D	6111	2-67	2-61	2-18	1-98	1-92	1-62	1-38	1-18	64	62 14:15:37 RNP	PCC	Excel.	None	1302
30065	D	9275	4-01	3-82	3-32	3-11	2-92	2-49	2-13	1-81	64	62 14:15:45 RNP	PCC	Excel.	None	1314
30065	D	12473	5-33	4-98	4-44	4-19	3-90	3-36	2-92	2-45	64	62 14:15:50 RNP	PCC	Excel.	None	1330
Comment	at	30064	ft	Time: 14:15:59	Load Transfer											
30494	D	6147	2-13	1-99	1-97	1-89	1-80	1-56	1-32	1-13	64	63 14:17:10 RNP	PCC	Excel.	None	1641
30494	D	9314	3-22	3-03	3-01	2-96	2-76	2-38	2-05	1-73	64	63 14:17:17 RNP	PCC	Excel.	None	1644
30494	D	12459	4-30	4-05	4-06	3-93	3-72	3-22	2-76	2-33	64	63 14:17:24 RNP	PCC	Excel.	None	1652
31018	D	6104	2-21	2-09	1-99	1-89	1-82	1-64	1-40	1-26	65	64 14:18:52 RNP	PCC	Excel.	None	1570
31018	D	9282	3-38	3-20	3-08	2-94	2-81	2-51	2-19	1-91	65	64 14:18:58 RNP	PCC	Excel.	None	1562
31018	D	12449	4-53	4-27	4-18	3-99	3-83	3-41	3-02	2-60	65	64 14:19:05 RNP	PCC	Excel.	None	1562
31512	D	6096	2-17	2-04	1-94	1-82	1-80	1-58	1-37	1-20	65	63 14:20:42 RNP	PCC	Excel.	None	1598
31512	D	9316	3-30	3-13	3-02	2-91	2-77	2-45	2-17	1-86	65	63 14:20:47 RNP	PCC	Excel.	None	1603
31512	D	12513	4-42	4-18	4-10	3-88	3-74	3-32	2-94	2-52	65	63 14:20:54 RNP	PCC	Excel.	None	1609
32017	D	6119	2-15	2-04	1-93	1-87	1-80	1-62	1-44	1-24	65	62 14:22:14 RNP	PCC	Excel.	None	1622
32017	D	9280	3-27	3-09	2-98	2-95	2-77	2-48	2-21	1-91	65	62 14:22:19 RNP	PCC	Excel.	None	1616
32017	D	12450	4-39	4-14	4-06	3-95	3-73	3-38	3-01	2-62	65	62 14:22:26 RNP	PCC	Excel.	None	1614
32543	D	6158	2-05	1-94	1-85	1-77	1-72	1-52	1-34	1-18	65	63 14:23:41 RNP	PCC	Excel.	None	1707
32543	D	9372	3-13	2-99	2-91	2-78	2-66	2-41	2-12	1-83	65	63 14:23:46 RNP	PCC	Excel.	None	1700
32543	D	12562	4-19	4-00	3-93	3-73	3-61	3-24	2-88	2-50	65	63 14:23:53 RNP	PCC	Excel.	None	1706
33045	D	6161	2-25	2-18	2-02	1-96	1-91	1-74	1-54	1-34	64	64 14:25:19 RNP	PCC	Excel.	None	1558
33045	D	9339	3-40	3-28	3-19	3-06	2-92	2-65	2-36	2-08	64	64 14:25:24 RNP	PCC	Excel.	None	1560
33045	D	12528	4-55	4-37	4-23	4-11	3-91	3-57	3-19	2-81	64	64 14:25:31 RNP	PCC	Excel.	None	1566
33517	D	6151	2-41	2-26	2-27	2-17	2-01	1-76	1-81	1-66	65	63 14:26:42 RNP	PCC	Excel.	None	1452
33517	D	9348	3-62	3-51	3-44	3-35	3-27	3-02	2-78	2-51	65	63 14:26:49 RNP	PCC	Excel.	None	1467
33517	D	12539	4-81	4-65	4-63	4-50	4-36	4-06	3-73	3-36	65	63 14:26:57 RNP	PCC	Excel.	None	1481
34013	D	6178	2-23	2-10	2-01	1-88	1-84	1-68	1-48	1-33	64	63 14:28:15 RNP	PCC	Excel.	None	1578
34013	D	9363	3-37	3-20	3-04	2-88	2-78	2-53	2-27	2-01	64	63 14:28:20 RNP	PCC	Excel.	None	1578

Brooklyn NB 17Nov09																			
D	41992	4	12553	5.41	5.02	5.02	4.70	4.46	3.92	3.38	2.88	62	62	15:05:43	RMP	PCC	Excel.	None	1321
D	42508	2	6175	2.22	2.25	1.99	1.86	1.80	1.60	1.43	1.23	62	61	15:07:03	RMP	PCC	Excel.	None	1381
D	42508	3	9371	3.36	3.38	3.05	2.94	2.78	2.47	2.20	1.90	62	61	15:07:08	RMP	PCC	Excel.	None	1584
D	42508	4	12578	4.48	4.48	4.09	3.86	3.74	3.36	2.96	2.58	62	61	15:07:15	RMP	PCC	Excel.	None	1596
D	43013	2	6162	2.05	2.19	2.43	2.26	2.25	2.01	1.77	1.59	62	61	15:08:37	RMP	PCC	Excel.	None	1324
D	43013	3	9377	4.04	3.97	3.72	3.61	3.44	3.10	2.78	2.46	62	61	15:08:42	RMP	PCC	Excel.	None	1314
D	43013	4	12575	5.38	5.28	4.97	4.79	4.62	4.19	3.75	3.34	62	61	15:08:49	RMP	PCC	Excel.	None	1328
D	43517	2	6183	2.77	2.77	2.35	2.41	2.35	2.14	1.88	1.63	62	58	15:10:05	RMP	PCC	Excel.	None	1271
D	43517	3	9355	4.19	4.17	3.85	3.72	3.56	3.22	2.86	2.50	62	58	15:10:11	RMP	PCC	Excel.	None	1268
D	43517	4	12541	5.54	5.49	5.13	4.96	4.75	4.29	3.83	3.34	62	58	15:10:18	RMP	PCC	Excel.	None	1286
D	44014	2	6215	2.01	1.89	1.84	1.74	1.74	1.60	1.42	1.30	62	58	15:11:30	RMP	PCC	Excel.	None	1762
D	44014	3	9379	3.03	2.88	2.82	2.75	2.66	2.44	2.23	1.99	62	58	15:11:35	RMP	PCC	Excel.	None	1760
D	44014	4	12531	4.04	3.81	3.76	3.67	3.57	3.31	3.00	2.68	62	58	15:11:41	RMP	PCC	Excel.	None	1764
D	44546	2	6208	2.43	2.35	2.24	2.08	2.06	1.87	1.68	1.48	62	59	15:13:02	RMP	PCC	Excel.	None	1454
D	44546	3	9401	3.66	3.53	3.38	3.29	3.14	2.84	2.57	2.26	62	59	15:13:08	RMP	PCC	Excel.	None	1459
D	44546	4	12601	4.88	4.70	4.56	4.39	4.25	3.85	3.48	3.05	62	59	15:13:16	RMP	PCC	Excel.	None	1478
D	45004	2	6233	2.40	2.34	2.24	2.14	2.14	1.98	1.80	1.65	62	60	15:14:34	RMP	PCC	Excel.	None	1478
D	45004	3	9402	3.61	3.48	3.41	3.35	3.24	3.01	2.78	2.53	62	60	15:14:40	RMP	PCC	Excel.	None	1483
D	45004	4	12616	4.80	4.62	4.58	4.43	4.33	4.03	3.73	3.39	62	60	15:14:47	RMP	PCC	Excel.	None	1494
D	45008	2	6210	3.07	3.00	2.63	2.47	2.39	2.09	1.81	1.59	62	60	15:15:36	RMP	PCC	Excel.	None	1151
D	45008	3	9351	4.53	4.48	3.99	3.82	3.60	3.17	2.79	2.40	62	60	15:15:42	RMP	PCC	Excel.	None	1179
D	45008	4	12559	6.05	5.96	5.37	5.08	4.84	4.28	3.80	3.27	62	60	15:15:49	RMP	PCC	Excel.	None	1185
ft Time: 15:15:59 : Load transfer																			
C	45503	2	6194	2.38	2.26	2.24	2.17	2.14	1.96	1.76	1.52	62	60	15:17:02	RMP	PCC	Excel.	None	1480
D	45503	3	9382	3.61	3.41	3.42	3.35	3.25	3.03	2.73	2.33	62	60	15:17:08	RMP	PCC	Excel.	None	1478
D	45503	4	12585	4.80	4.55	4.58	4.52	4.38	4.08	3.70	3.17	62	60	15:17:15	RMP	PCC	Excel.	None	1490
D	46035	2	6183	3.25	3.22	3.04	2.92	2.90	2.57	2.22	1.91	61	59	15:18:23	RMP	PCC	Excel.	None	1082
D	46035	3	9353	4.92	4.80	4.62	4.54	4.44	4.03	3.66	3.02	61	59	15:18:30	RMP	PCC	Excel.	None	1080
D	46035	4	12530	6.65	6.61	6.27	6.15	6.01	5.32	4.82	3.99	61	59	15:18:37	RMP	PCC	Excel.	None	1074
D	46545	2	6230	2.22	2.11	2.13	2.07	2.05	1.98	1.86	1.78	61	60	15:19:50	RMP	PCC	Excel.	None	1399
D	46545	3	9426	3.34	3.17	3.23	3.19	3.13	2.96	2.82	2.68	61	60	15:19:56	RMP	PCC	Excel.	None	1604
D	46545	4	12627	4.42	4.17	4.31	4.24	4.17	3.96	3.76	3.56	61	60	15:20:03	RMP	PCC	Excel.	None	1625
ft Time: 15:21:17 : Deflection is not decreasing																			
C	47006	2	6186	2.51	2.31	2.50	2.46	2.53	2.57	2.62	2.71	61	58	15:21:20	RMP	PCC	Excel.	None	1403
C	47006	3	9341	3.82	3.48	3.85	3.91	3.94	4.01	4.09	4.22	61	58	15:21:29	RMP	PCC	Excel.	None	1389
C	47006	4	12573	5.11	4.66	5.20	5.24	5.33	5.43	5.56	5.75	61	58	15:21:38	RMP	PCC	Excel.	None	1398
D	47526	2	6180	3.37	3.12	2.98	2.78	2.63	2.28	1.98	1.62	61	58	15:22:52	RMP	PCC	Excel.	None	1043
D	47526	3	9386	5.12	4.74	4.54	4.27	4.00	3.40	2.95	2.47	61	58	15:23:38	RMP	PCC	Excel.	None	1049
D	47526	4	12577	6.82	6.33	6.09	5.75	5.38	4.55	3.97	3.32	61	58	15:23:05	RMP	PCC	Excel.	None	1049
D	47996	2	6239	1.67	1.59	1.52	1.46	1.46	1.35	1.21	1.09	61	58	15:24:28	RMP	PCC	Excel.	None	2129
D	47996	3	9455	2.50	2.36	2.31	2.26	2.20	2.03	1.83	1.66	61	58	15:24:33	RMP	PCC	Excel.	None	2151
D	47996	4	12673	3.32	3.13	3.10	3.03	2.93	2.72	2.49	2.22	61	58	15:24:39	RMP	PCC	Excel.	None	2171
D	48512	2	6173	2.13	2.08	1.98	1.88	1.74	1.58	1.42	1.28	61	58	15:25:57	RMP	PCC	Excel.	None	1647
D	48512	3	9333	3.25	3.11	3.01	2.97	2.85	2.63	2.40	2.15	61	58	15:26:03	RMP	PCC	Excel.	None	1635
D	48512	4	12555	4.30	4.12	4.03	3.95	3.80	3.53	3.21	2.88	61	58	15:26:10	RMP	PCC	Excel.	None	1661
D	49013	2	6211	2.08	1.97	1.92	1.82	1.77	1.61	1.39	1.17	61	59	15:27:25	RMP	PCC	Excel.	None	1697
D	49013	3	9415	3.20	3.01	2.97	2.88	2.75	2.52	2.21	1.84	61	59	15:27:30	RMP	PCC	Excel.	None	1671
D	49013	4	12641	4.31	4.04	4.02	3.91	3.75	3.42	3.03	2.52	61	59	15:27:37	RMP	PCC	Excel.	None	1667
D	49500	2	6222	1.51	1.41	1.38	1.32	1.33	1.23	1.09	0.99	61	58	15:28:51	RMP	PCC	Excel.	None	2348
D	49500	3	9388	2.29	2.16	2.11	2.09	1.99	1.87	1.69	1.54	61	58	15:28:55	RMP	PCC	Excel.	None	2327
D	49500	4	12638	3.06	2.85	2.86	2.80	2.70	2.52	2.30	2.10	61	58	15:29:02	RMP	PCC	Excel.	None	2352
D	49996	2	6266	1.89	1.81	1.73	1.63	1.57	1.43	1.28	1.14	61	59	15:30:15	RMP	PCC	Excel.	None	1887
D	49996	3	9431	2.87	2.75	2.62	2.53	2.43	2.21	1.98	1.75	61	59	15:30:20	RMP	PCC	Excel.	None	1869
D	49996	4	12663	3.85	3.67	3.53	3.42	3.28	2.98	2.71	2.37	61	59	15:30:25	RMP	PCC	Excel.	None	1872
ft Time: 15:31:12 : Deflection is not decreasing																			
C	50001	2	6196	5.22	4.34	3.36	2.77	2.46	1.99	1.56	1.23	61	59	15:31:13	RMP	PCC	Excel.	None	675
C	50001	3	9350	7.45	6.33	4.88	4.19	3.68	2.97	2.36	1.87	61	59	15:31:20	RMP	PCC	Excel.	None	713
C	50001	4	12543	9.29	8.28	6.28	5.41	4.79	3.90	3.13	2.47	61	59	15:31:29	RMP	PCC	Excel.	None	768
ft Time: 15:32:51 : Deflection is not decreasing																			
C	50544	2	6199	4.64	5.21	3.70	3.29	2.97	2.29	1.73	1.34	61	59	15:32:53	RMP	PCC	Excel.	None	759
ft Time: 15:32:59 : Deflection is not decreasing																			
C	50544	3	9358	7.02	7.87	5.62	5.04	4.52	3.51	2.71	2.06	61	59	15:33:00	RMP	PCC	Excel.	None	758

C Comment at 50544 ft Time: 15:33:07 :deflection is not decreasing
 D 50544 4 12552 9.09 10.11 7.36 6.65 5.96 4.66 3.61 2.75 61 59 15:33:08 RMP PCC Exce]. None 785

D	-49748	3	9078	195	181	178	168	155	126	100	71	65	76	14:43:23	CTR	Brooklyn PCC PCC	SB Cable Excell. Excell.	None None	671 667
D	-49748	4	12408	269	248	245	230	212	174	138	98	65	76	14:43:30	CTR				

Brooklyn Southbound, November 2009

Brooklyn SB 17 Nov09

IKUAB FWD FILE : Brooklyn SB 17 Nov09.fwd

HProject No. : Cable Overlay
 HLocation : Hwy 17 sb
 HClient : Cable
 HStart Station :
 HEnd Station :
 HWeather : sunny clear
 HOperator : hg

IDate Created : 11/11/2009
 IVersion : 2.3.11
 ILoad Mode : 1 (SHRP 8-8 buffers, 0 plates)
 IPlate Radius (in) : 5.91
 IExtra Field Set : Example Road
 IDrop Sequence : 2123
 INo of drops : 1111
 IRecord Drop? : NYY
 IDrop Height : 1 2 3 4
 IImpact Load : 6003 9005 12007 16009 lbf
 ISensor Number : 0 0 1 2 3
 ISensor Distance : 0.00 12.00 18.00 24.00 36.00 48.00 60.00 84.00 (in)
 ISensor Position : CENTER FRONT BEHIND BEHIND BEHIND BEHIND BEHIND BEHIND BEHIND

Reference offset : 0 ft
 Itrestpoint spacing : 0 ft

J	IDistance	Imp	Load	DL	D1	D2	D3	D4	D5	D6	D7	Air	Pave	Time	Location	Pavement Type	Pavement Condition	Pavement Distress	Surface Modtl
J	ft	Num	lbf	mm	mm	mm	mm	mm	mm	mm	mm	F	F	mm					
D	97	2	6273	3.10	3.07	2.73	2.45	2.32	1.87	1.44	1.12	45	44	08:54:16	RWP	PCC	Excel	None	1152
D	97	3	9612	4.85	4.76	4.30	3.98	3.64	2.93	2.31	1.74	45	44	08:54:21	RWP	PCC	Excel	None	1128
D	97	4	12766	6.46	6.33	5.76	5.31	4.88	3.95	3.10	2.33	45	44	08:54:29	RWP	PCC	Excel	None	1124
D	518	2	6290	1.66	1.65	1.47	1.39	1.35	1.19	1.04	0.89	47	45	09:03:10	RWP	PCC	Excel	None	2156
D	518	3	9659	2.58	2.55	2.35	2.25	2.12	1.88	1.64	1.39	47	45	09:03:16	RWP	PCC	Excel	None	2127
D	518	4	12830	3.45	3.41	3.14	2.96	2.82	2.52	2.21	1.89	47	45	09:03:23	RWP	PCC	Excel	None	2117
D	1019	2	6312	1.63	1.57	1.50	1.42	1.38	1.23	1.10	0.99	47	45	09:05:05	RWP	PCC	Excel	None	2204
D	1019	3	9596	2.53	2.38	2.34	2.28	2.17	1.95	1.75	1.55	47	45	09:05:11	RWP	PCC	Excel	None	2158
D	1019	4	12765	3.37	3.21	3.16	3.04	2.92	2.66	2.37	2.09	47	45	09:05:19	RWP	PCC	Excel	None	2157
D	1520	2	6363	1.66	1.56	1.50	1.42	1.40	1.25	1.10	0.99	48	42	09:07:07	RWP	PCC	Excel	None	2180
D	1520	3	9678	2.55	2.43	2.35	2.29	2.17	1.95	1.72	1.55	48	42	09:07:13	RWP	PCC	Excel	None	2154
D	2024	4	12854	3.41	3.26	3.16	3.01	2.89	2.61	2.33	2.06	48	42	09:07:21	RWP	PCC	Excel	None	2144
D	2024	3	9638	2.16	2.07	1.98	1.88	1.84	1.62	1.42	1.25	48	44	09:09:05	RWP	PCC	Excel	None	1675
D	2024	4	12789	3.31	3.17	3.06	2.98	2.82	2.52	2.21	1.93	48	44	09:09:12	RWP	PCC	Excel	None	1657
D	2506	2	6368	2.74	2.53	2.61	2.41	2.36	2.05	1.80	1.61	48	45	09:10:48	RWP	PCC	Excel	None	1660
D	2506	3	9597	4.23	3.91	4.05	3.85	3.63	3.19	2.80	2.51	48	45	09:10:54	RWP	PCC	Excel	None	1319
D	3023	2	6374	2.45	2.35	2.25	2.11	2.09	1.85	1.61	1.44	48	45	09:12:35	RWP	PCC	Excel	None	1291
D	3023	3	9613	3.70	3.63	3.51	3.30	3.23	2.87	2.56	2.24	48	45	09:12:50	RWP	PCC	Excel	None	1477
D	3023	4	12789	5.07	4.84	4.73	4.47	4.32	3.87	3.41	3.02	48	45	09:12:58	RWP	PCC	Excel	None	1443
D	3511	2	6425	2.67	2.42	2.44	2.36	2.28	2.00	1.75	1.55	48	45	09:14:40	RWP	PCC	Excel	None	1435
D	3511	3	9580	4.01	3.66	3.82	3.67	3.43	3.04	2.66	2.32	48	45	09:14:53	RWP	PCC	Excel	None	1368
D	4019	2	6420	2.56	2.48	2.35	2.21	2.18	1.92	1.70	1.55	47	46	09:15:41	RWP	PCC	Excel	None	1363
D	4019	3	9642	3.88	3.76	3.58	3.46	3.32	2.98	2.63	2.32	47	46	09:15:54	RWP	PCC	Excel	None	1413
D	4019	4	12806	5.17	4.95	4.80	4.59	4.40	3.95	3.50	3.09	47	46	09:15:54	RWP	PCC	Excel	None	1408
D	4494	2	6410	3.35	3.17	2.97	2.77	2.64	2.32	1.99	1.72	47	46	09:18:45	RWP	PCC	Excel	None	1088
D	4494	3	9620	5.05	4.80	4.50	4.29	4.02	3.50	3.03	2.59	47	46	09:18:51	RWP	PCC	Excel	None	1084
D	5026	2	6402	3.24	2.86	2.90	2.67	2.51	2.14	1.79	1.47	47	46	09:20:33	RWP	PCC	Excel	None	1091
D	5026	3	9546	4.93	4.37	4.50	4.20	3.86	3.29	2.75	2.27	47	47	09:20:39	RWP	PCC	Excel	None	1101
D	5026	4	12704	6.58	5.79	6.02	5.55	5.19	4.41	3.69	3.05	47	47	09:20:47	RWP	PCC	Excel	None	1057
C	Comment at	5472	ft	Time: 09:23:00	Deflection	is not decreasing													
C	5472	2	6374	4.21	2.41	3.31	2.95	2.79	2.28	1.87	1.52	48	46	09:23:08	RWP	PCC	Excel	None	860
C	Comment at	5472	ft	Time: 09:23:14	Deflection	is not decreasing													

Brooklyn SB 17 Nov09												
5472	3	9511	6.24	3.60	4.97	4.57	4.13	3.41	2.81	2.28	2.28	866
Comment at 5472 ft Time: 09:23:24 :Deflection is not decreasing												
5472	4	12653	8.28	4.77	6.60	5.99	5.47	4.54	3.75	3.05	3.05	869
Comment at 5472 ft Time: 09:23:41 :Load transfer at 5472												
5518	2	6412	2.41	2.32	2.22	2.14	2.08	1.86	1.65	1.45	1.45	1511
5518	3	9551	3.65	3.51	3.36	3.28	3.13	2.85	2.54	2.22	2.22	1488
5518	4	12689	4.85	4.65	4.52	4.39	4.19	3.83	3.40	2.96	2.96	1485
5985	2	6418	2.76	2.82	2.41	2.35	2.16	1.86	1.59	1.40	1.40	1321
5985	3	9570	4.14	4.29	3.66	3.46	3.27	2.84	2.46	2.11	2.11	1314
5985	4	12713	5.48	5.64	4.86	4.57	4.33	3.70	3.30	2.80	2.80	1319
6513	2	6414	2.20	2.14	2.03	1.92	1.87	1.70	1.55	1.39	1.39	1659
6513	3	9605	3.28	3.19	3.05	2.96	2.84	2.31	2.09	2.09	2.09	1667
6513	4	12765	4.32	4.17	4.05	3.86	3.43	3.11	2.77	2.48	2.48	1680
6984	2	6441	2.85	2.78	2.72	2.59	2.59	2.38	2.18	2.03	2.03	1282
6984	3	9592	4.30	4.16	4.10	4.01	3.89	3.61	3.31	3.04	3.04	1269
7510	2	6389	3.01	3.01	2.73	2.54	2.52	2.21	1.96	1.74	1.74	1273
7510	3	9503	4.47	4.51	4.04	3.87	3.73	3.32	2.93	2.61	2.61	1209
7510	4	12649	5.90	5.92	5.38	5.11	4.90	4.42	3.91	3.49	3.49	1220
7998	2	6370	2.71	2.62	2.42	2.29	2.25	1.96	1.71	1.53	1.53	1336
7998	3	9506	4.08	3.96	3.71	3.56	3.36	3.00	2.62	2.31	2.31	1325
7998	4	12687	5.47	5.27	5.00	4.73	4.51	3.99	3.52	3.09	3.09	1319
8409	2	6287	13.78	10.79	1.94	1.78	1.68	1.42	1.22	1.07	1.07	259
8409	3	9396	19.33	15.20	2.89	2.73	2.50	2.13	1.84	1.59	1.59	276
8409	4	12425	24.15	18.98	3.79	3.57	3.29	2.82	2.45	2.11	2.11	293
Comment at 8409 ft Time: 09:36:14 :8409 last test before overpass section												
Comment at 8509 ft Time: 09:36:58 :8509 start of overpass section												
Comment at 8525 ft Time: 09:39:04 :8525 end of overpass section												
10002	2	6371	8.23	6.52	5.17	4.62	4.11	3.17	2.43	1.88	1.88	440
10002	3	9513	11.58	9.27	8.23	7.38	6.54	5.02	3.85	2.96	2.96	467
10002	4	12597	14.53	11.71	11.08	9.94	8.80	6.76	5.17	3.97	3.97	490
Comment at 10079 ft Time: 09:42:00 :Deflection is not decreasing												
10079	2	6362	9.76	2.74	7.70	6.77	6.03	4.38	3.49	2.69	2.69	371
Comment at 10079 ft Time: 09:42:09 :Deflection is not decreasing												
10079	3	9454	13.11	5.83	10.36	9.20	8.23	6.24	4.84	3.75	3.75	410
Comment at 10079 ft Time: 09:42:18 :Deflection is not decreasing												
10079	4	12581	16.14	8.75	12.88	11.49	10.18	7.82	6.12	4.70	4.70	443
Comment at 10079 ft Time: 09:42:33 :10079 load transfer test												
10522	2	6475	2.05	1.96	1.94	1.86	1.83	1.69	1.55	1.45	1.45	1800
10522	3	9633	3.04	2.90	2.90	2.87	2.77	2.58	2.37	2.19	2.19	1802
10522	4	12816	4.07	3.85	3.89	3.78	3.70	3.43	3.21	2.97	2.97	1790
11017	2	6461	2.25	2.14	2.11	2.01	1.94	1.78	1.56	1.38	1.38	1632
11017	3	9653	3.39	3.22	3.18	3.11	2.95	2.60	2.40	2.08	2.08	1620
11017	4	12821	4.49	4.26	4.24	4.09	3.95	3.60	3.21	2.80	2.80	1623
11506	2	6444	2.18	2.13	2.00	1.89	1.86	1.71	1.51	1.33	1.33	1684
11506	3	9597	3.25	3.19	3.00	2.92	2.81	2.57	2.30	2.01	2.01	1680
11506	4	12767	4.29	4.23	3.99	3.85	3.75	3.43	3.09	2.69	2.69	1691
11991	2	6439	2.22	2.15	2.09	2.01	1.97	1.77	1.63	1.45	1.45	1652
11991	3	9591	3.33	3.23	3.16	3.05	2.96	2.73	2.48	2.23	2.23	1637
12519	2	6393	4.41	4.23	4.21	4.06	3.95	3.64	3.33	2.99	2.99	1643
12519	3	9569	6.77	6.06	6.18	5.74	5.32	4.95	4.54	4.24	4.24	816
12519	4	12700	9.06	8.07	8.29	7.73	7.11	6.05	5.27	4.62	4.62	797
13016	2	6432	2.33	2.28	2.13	2.01	1.97	1.77	1.56	1.40	1.40	1370
13016	3	9594	3.49	3.43	3.22	3.11	2.99	2.72	2.42	2.14	2.14	1364
13016	4	12746	4.59	4.52	4.28	4.10	3.99	3.62	3.24	2.88	2.88	1377
13394	2	6417	2.46	2.41	2.31	2.20	2.17	1.95	1.75	1.56	1.56	1484
13394	3	9580	3.71	3.62	3.46	3.38	3.25	2.98	2.38	2.38	2.38	1470
13394	4	12748	4.88	4.73	4.62	4.44	4.30	3.95	3.56	3.18	3.18	1486
Comment at 13542 ft Time: 09:57:05 :Turned to sb section												
13985	2	6392	2.63	2.47	2.56	2.54	2.53	2.29	1.96	1.71	1.71	1381
13985	3	9532	3.94	3.69	3.87	3.90	3.81	3.42	2.98	2.55	2.55	1377
Comment at 13985 ft Time: 10:00:46 :Deflection is not decreasing												
13985	4	12679	5.16	4.82	5.15	5.10	5.03	4.52	3.96	3.27	3.27	1398
Comment at 13985 ft Time: 10:00:59 :Joint spacings is greater than 15ft in this area												
14504	2	6410	2.29	2.20	2.14	2.07	2.05	1.89	1.72	1.58	1.58	1590

D	14504	3	9601	3.44	3.30	3.24	3.18	3.10	2.88	2.62	2.37	51	50	10:02:49	RNP	PCC	Excel.	None	1588	
D	14504	4	12748	4.56	4.35	4.33	4.22	4.14	3.85	3.51	3.17	51	50	10:02:56	RNP	PCC	Excel.	None	1590	
D	14975	2	6386	2.58	2.48	2.35	2.29	2.26	2.15	1.88	1.69	51	51	10:04:38	RNP	PCC	Excel.	None	1406	
D	14975	3	9548	3.84	3.72	3.65	3.51	3.42	3.09	2.85	2.54	51	51	10:04:44	RNP	PCC	Excel.	None	1415	
D	14975	4	12720	5.07	4.89	4.83	4.64	4.54	4.20	3.82	3.40	51	51	10:04:51	RNP	PCC	Excel.	None	1426	
D	14980	2	6391	3.96	3.81	3.26	3.03	2.90	2.53	2.38	1.90	52	51	10:07:21	RNP	PCC	Excel.	None	918	
D	14980	3	9541	5.95	4.53	4.94	4.67	4.38	3.81	3.33	2.87	52	51	10:07:27	RNP	PCC	Excel.	None	912	
D	14980	4	12685	7.87	5.94	6.57	6.16	5.79	5.06	4.42	3.81	52	51	10:07:34	RNP	PCC	Excel.	None	916	
C				ft Time: 10:07:44 : Load transfer																
D	15530	2	6341	2.25	2.15	2.09	2.00	1.95	1.82	1.59	1.44	52	52	10:09:23	RNP	PCC	Excel.	None	1604	
D	15530	3	9511	3.36	3.22	3.14	3.06	2.96	2.71	2.44	2.17	52	52	10:09:29	RNP	PCC	Excel.	None	1612	
D	15530	4	12700	4.44	4.26	4.20	4.08	3.94	3.58	3.25	2.90	52	52	10:09:36	RNP	PCC	Excel.	None	1625	
D	16016	2	6349	2.75	2.61	2.45	2.25	2.16	1.89	1.63	1.44	52	51	10:11:10	RNP	PCC	Excel.	None	1312	
D	16016	3	9516	4.16	3.97	3.73	3.52	3.31	2.90	2.53	2.16	52	51	10:11:16	RNP	PCC	Excel.	None	1300	
D	16016	4	12664	5.52	5.24	4.99	4.67	4.41	3.88	3.40	2.90	52	51	10:11:23	RNP	PCC	Excel.	None	1304	
D	16501	2	6355	2.19	2.10	2.03	1.96	1.94	1.78	1.61	1.46	51	52	10:13:05	RNP	PCC	Excel.	None	1651	
D	16501	3	9228	3.28	3.15	3.09	3.01	2.92	2.71	2.47	2.22	51	52	10:13:11	RNP	PCC	Excel.	None	1654	
D	16501	4	12696	4.52	4.17	4.11	3.97	3.87	3.58	3.28	2.95	51	52	10:13:18	RNP	PCC	Excel.	None	1673	
D	17021	2	6364	2.38	2.28	2.19	2.10	2.05	1.86	1.64	1.47	52	53	10:14:45	RNP	PCC	Excel.	None	1323	
D	17021	3	9523	3.55	3.40	3.33	3.20	3.10	2.83	2.54	2.23	52	53	10:14:51	RNP	PCC	Excel.	None	1525	
D	17021	4	12725	4.70	4.47	4.42	4.26	4.11	3.75	3.36	2.96	52	53	10:14:58	RNP	PCC	Excel.	None	1541	
D	17486	2	6351	2.03	1.92	1.89	1.77	1.75	1.55	1.35	1.21	52	52	10:16:25	RNP	PCC	Excel.	None	1781	
D	17486	3	9459	3.06	2.90	2.85	2.74	2.63	2.36	2.09	1.83	52	52	10:16:30	RNP	PCC	Excel.	None	1757	
D	17486	4	12664	4.06	3.83	3.79	3.65	3.49	3.13	2.81	2.45	52	52	10:16:37	RNP	PCC	Excel.	None	1774	
D	18024	2	6390	1.92	1.84	1.77	1.69	1.66	1.51	1.34	1.21	52	53	10:18:12	RNP	PCC	Excel.	None	1853	
D	18024	3	9544	2.89	2.75	2.68	2.63	2.51	2.28	2.04	1.80	52	53	10:18:17	RNP	PCC	Excel.	None	1893	
D	18024	4	12715	3.83	3.64	3.59	3.46	3.35	3.05	2.75	2.41	52	53	10:18:24	RNP	PCC	Excel.	None	1880	
D	18539	2	6270	2.12	2.01	1.97	1.86	1.81	1.65	1.45	1.25	52	53	10:20:04	RNP	PCC	Excel.	None	1877	
D	18539	3	9319	3.22	3.04	2.97	2.88	2.77	2.51	2.22	1.88	52	53	10:20:09	RNP	PCC	Excel.	None	1878	
D	18539	4	12699	4.30	4.03	4.01	3.85	3.71	3.37	2.97	2.51	52	53	10:20:15	RNP	PCC	Excel.	None	1681	
D	19007	2	6401	1.94	1.81	1.75	1.69	1.64	1.46	1.30	1.14	53	53	10:21:42	RNP	PCC	Excel.	None	1879	
D	19007	3	9552	2.91	2.73	2.66	2.60	2.46	2.23	1.97	1.74	53	53	10:21:48	RNP	PCC	Excel.	None	1864	
D	19007	4	12697	3.88	3.61	3.57	3.44	3.32	2.99	2.68	2.33	53	53	10:21:54	RNP	PCC	Excel.	None	1861	
D	19514	2	6412	1.92	1.81	1.79	1.71	1.61	1.41	1.24	1.13	53	53	10:23:49	RNP	PCC	Excel.	None	1901	
D	19514	3	9623	2.88	2.70	2.67	2.58	2.47	2.21	1.94	1.68	53	53	10:23:54	RNP	PCC	Excel.	None	1899	
D	19514	4	12784	3.88	3.64	3.60	3.45	3.33	2.99	2.64	2.29	53	53	10:24:01	RNP	PCC	Excel.	None	1873	
D	20013	2	6393	1.95	1.86	1.77	1.68	1.64	1.50	1.31	1.18	53	53	10:25:28	RNP	PCC	Excel.	None	1863	
D	20013	3	9558	2.95	2.82	2.72	2.64	2.49	2.26	2.02	1.76	53	53	10:25:34	RNP	PCC	Excel.	None	1842	
D	20013	4	12738	3.91	3.72	3.63	3.48	3.34	3.02	2.71	2.34	53	53	10:25:40	RNP	PCC	Excel.	None	1853	
C				ft Time: 10:26:49 : deflection is not decreasing																
D	20017	2	6371	4.36	4.06	3.91	3.71	3.57	3.24	2.84	2.44	54	52	10:26:55	RNP	PCC	Excel.	None	831	
C				ft Time: 10:27:00 : deflection is not decreasing																
D	20017	3	9518	6.40	5.02	4.48	4.00	3.61	2.98	2.45	2.03	54	52	10:27:01	RNP	PCC	Excel.	None	845	
C				ft Time: 10:27:08 : deflection is not decreasing																
D	20017	4	12677	8.35	4.49	5.91	5.22	4.75	3.93	3.25	2.67	54	52	10:27:11	RNP	PCC	Excel.	None	863	
C				ft Time: 10:27:21 : load transfer																
D	20504	2	6374	2.15	2.12	1.88	1.78	1.70	1.49	1.27	1.07	54	54	10:28:39	RNP	PCC	Excel.	None	1688	
D	20504	3	9533	3.26	3.23	2.90	2.77	2.60	2.25	1.94	1.64	54	54	10:28:44	RNP	PCC	Excel.	None	1663	
D	20504	4	12702	4.37	4.31	3.90	3.69	3.49	3.04	2.61	2.21	54	54	10:28:51	RNP	PCC	Excel.	None	1651	
D	21009	2	6407	1.94	1.85	1.78	1.70	1.67	1.52	1.35	1.20	54	53	10:30:20	RNP	PCC	Excel.	None	1882	
D	21009	3	9571	2.90	2.77	2.69	2.60	2.50	2.28	2.03	1.81	54	53	10:30:25	RNP	PCC	Excel.	None	1877	
D	21009	4	12765	3.83	3.53	3.36	3.45	3.51	3.03	2.73	2.39	54	53	10:30:35	RNP	PCC	Excel.	None	1897	
D	21520	2	6356	1.99	1.92	1.80	1.72	1.73	1.55	1.39	1.24	53	52	10:32:58	RNP	PCC	Excel.	None	1813	
D	21520	3	9546	2.98	2.86	2.75	2.64	2.56	2.30	2.04	1.80	53	52	10:33:03	RNP	PCC	Excel.	None	1823	
D	21520	4	12711	3.93	3.77	3.68	3.50	3.39	3.07	2.76	2.40	53	52	10:33:10	RNP	PCC	Excel.	None	1838	
D	22009	2	6327	1.90	1.82	1.72	1.65	1.56	1.38	1.19	1.01	52	52	10:34:41	RNP	PCC	Excel.	None	1890	
D	22009	3	9489	2.94	2.78	2.68	2.58	2.44	2.11	1.83	1.55	52	52	10:34:46	RNP	PCC	Excel.	None	1834	
D	22009	4	12650	3.89	3.67	3.57	3.43	3.23	2.86	2.47	2.10	52	52	10:34:53	RNP	PCC	Excel.	None	1847	
D	22509	2	6353	1.85	1.77	1.70	1.63	1.57	1.42	1.26	1.11	52	51	10:36:37	RNP	PCC	Excel.	None	1947	
D	22509	3	9487	2.79	2.64	2.60	2.47	2.36	2.13	1.89	1.64	52	51	10:36:42	RNP	PCC	Excel.	None	1935	
D	22509	4	12661	3.74	3.52	3.44	3.30	3.16	2.85	2.54	2.20	52	51	10:36:49	RNP	PCC	Excel.	None	1924	
D	22982	2	6358	1.73	1.69	1.55	1.51	1.43	1.29	1.12	0.95	52	51	10:38:14	RNP	PCC	Excel.	None	2066	
D	22982	3	9515	2.65	2.54	2.45	2.31	2.21	1.95	1.69	1.45	52	51	10:38:20	RNP	PCC	Excel.	None	2039	
D	22982	4	12699	3.56	3.39	3.23	3.11	2.94	2.61	2.31	1.97	52	51	10:38:26	RNP	PCC	Excel.	None	2028	
D	23533	2	6368	1.95	1.88	1.82	1.74	1.71	1.55	1.39	1.26	51	52	10:40:14	RNP	PCC	Excel.	None	1835	
D	23533	3	9535	2.95	2.84	2.75	2.71	2.57	2.35	2.12	1.89	51	52	10:40:19	RNP	PCC	Excel.	None	1840	

Brooklyn SB 17 Nov09

D	23533	4	12737	3:88	3:71	3:66	3:54	3:42	3:11	2:83	2:52	51	52 10:40:25 RWP	PCC	Excel.	None	1867				
D	24146	2	6373	3:12	2:88	2:69	2:53	2:38	2:04	1:75	1:47	52	10:42:45 RWP	PCC	Excel.	None	1163				
D	24146	3	9523	4:69	4:33	4:11	3:86	3:61	3:10	2:64	2:22	53	10:42:50 RWP	PCC	Excel.	None	1154				
D	24146	6	21	6:21	5:71	5:50	5:12	4:80	4:12	3:56	2:98	52	10:42:57 RWP	PCC	Excel.	None	1161				
D	24517	2	6342	2:58	2:61	2:32	2:18	2:12	1:88	1:65	1:46	52	10:44:23 RWP	PCC	Excel.	None	1398				
D	24517	3	9450	3:91	3:94	3:52	3:38	3:23	2:89	2:58	2:24	52	10:44:28 RWP	PCC	Excel.	None	1376				
D	24517	4	12671	5:15	5:18	4:70	4:48	4:29	3:87	3:45	3:01	52	10:44:35 RWP	PCC	Excel.	None	1399				
D	25001	2	6315	2:21	2:21	2:04	1:92	1:89	1:72	1:50	1:29	53	10:46:07 RWP	PCC	Excel.	None	1633				
D	25001	3	9314	3:55	3:25	3:07	3:03	2:89	2:62	2:31	1:96	55	10:46:12 RWP	PCC	Excel.	None	1615				
D	25001	4	12690	4:46	4:30	4:15	4:00	3:86	3:53	3:11	2:61	53	10:46:19 RWP	PCC	Excel.	None	1619				
C	Comment	at 25004	ft	Time: 10:47:16	:Deflection is not decreasing																
D	25004	2	6326	3:88	2:21	2:86	2:59	2:42	1:99	1:64	1:35	53	10:47:18 RWP	PCC	Excel.	None	528				
C	Comment	at 25004	ft	Time: 10:47:24	:Deflection is not decreasing																
D	25004	3	9457	5:86	3:33	4:35	4:02	3:69	3:04	2:53	2:06	53	10:47:24 RWP	PCC	Excel.	None	518				
C	Comment	at 25004	ft	Time: 10:47:31	:Deflection is not decreasing																
D	25004	4	12628	7:81	4:43	5:87	5:38	4:92	4:10	3:40	2:77	53	10:47:32 RWP	PCC	Excel.	None	520				
C	Comment	at 25004	ft	Time: 10:47:42	:Load Transfer																
D	25540	2	6374	1:65	1:54	1:53	1:49	1:46	1:40	1:26	1:11	54	10:48:53 RWP	PCC	Excel.	None	2193				
D	25540	3	9511	2:50	2:32	2:34	2:31	2:24	2:13	1:93	1:65	54	10:48:58 RWP	PCC	Excel.	None	2160				
D	25540	4	12732	3:32	3:08	3:16	3:07	3:02	2:85	2:59	2:23	54	10:48:05 RWP	PCC	Excel.	None	2182				
D	25005	2	6349	1:99	1:84	1:79	1:69	1:62	1:45	1:26	1:07	54	10:50:22 RWP	PCC	Excel.	None	1615				
D	25005	3	9479	3:00	2:81	2:73	2:63	2:50	2:19	1:91	1:61	54	10:50:28 RWP	PCC	Excel.	None	1795				
D	25005	4	12674	4:02	3:74	3:67	3:50	3:35	2:97	2:59	2:19	54	10:50:34 RWP	PCC	Excel.	None	1793				
D	25508	2	6334	2:18	2:06	2:00	1:89	1:87	1:65	1:48	1:32	54	10:51:58 RWP	PCC	Excel.	None	1650				
D	25508	3	9456	3:30	3:14	3:05	2:97	2:83	2:55	2:26	1:99	54	10:52:03 RWP	PCC	Excel.	None	1629				
D	25508	4	12640	4:42	4:22	4:12	3:98	3:83	3:48	3:11	2:70	54	10:52:10 RWP	PCC	Excel.	None	1628				
D	25990	2	6309	2:66	2:57	2:44	2:33	2:29	2:05	1:81	1:60	54	10:53:45 RWP	PCC	Excel.	None	1348				
D	25990	3	9448	4:00	3:85	3:70	3:60	3:45	3:10	2:76	2:40	54	10:53:52 RWP	PCC	Excel.	None	1343				
D	25990	4	12638	5:28	5:06	4:92	4:76	4:57	4:11	3:67	3:20	54	10:53:55 RWP	PCC	Excel.	None	1359				
D	27504	2	6302	2:15	2:07	1:97	1:86	1:83	1:66	1:41	1:32	54	10:55:44 RWP	PCC	Excel.	None	1668				
D	27504	3	9432	3:21	3:10	2:98	2:93	2:78	2:50	2:24	1:96	54	10:55:49 RWP	PCC	Excel.	None	1668				
D	27504	4	12613	4:26	4:09	3:97	3:83	3:69	3:35	2:98	2:56	54	10:55:56 RWP	PCC	Excel.	None	1683				
D	27995	2	6310	4:60	3:43	3:28	3:05	2:95	2:52	2:15	1:93	54	10:57:41 RWP	PCC	Excel.	None	598				
D	27995	3	9505	5:43	5:18	5:02	4:82	4:51	3:91	3:35	2:92	54	10:57:47 RWP	PCC	Excel.	None	595				
D	27995	4	12646	7:22	6:86	6:70	6:38	6:03	5:24	4:46	3:89	54	10:57:54 RWP	PCC	Excel.	None	596				
D	28530	2	6389	3:57	3:32	3:19	2:93	2:71	2:22	1:78	1:42	54	10:59:27 RWP	PCC	Excel.	None	1016				
D	28530	3	9495	5:43	5:03	4:89	4:56	4:16	3:37	2:72	2:15	54	10:59:32 RWP	PCC	Excel.	None	594				
D	28530	4	12670	7:32	6:78	6:63	6:14	5:62	4:64	3:68	2:91	54	10:59:39 RWP	PCC	Excel.	None	584				
D	28973	2	6331	1:97	1:88	1:83	1:76	1:71	1:54	1:37	1:17	54	11:01:25 RWP	PCC	Excel.	None	1829				
D	28973	3	9490	2:99	2:84	2:79	2:71	2:61	2:41	2:07	1:77	54	11:01:31 RWP	PCC	Excel.	None	1808				
D	28973	4	12661	4:00	3:78	3:76	3:62	3:50	3:21	2:79	2:41	54	11:01:37 RWP	PCC	Excel.	None	1799				
C	Comment	at 28973	ft	Time: 11:01:47	:center of 450th street																
D	29501	2	6369	1:55	1:50	1:46	1:39	1:34	1:20	1:08	0:94	55	11:03:19 RWP	PCC	Excel.	None	2331				
D	29501	3	9508	2:38	2:26	2:22	2:16	2:03	1:84	1:64	1:44	55	11:03:24 RWP	PCC	Excel.	None	2269				
D	29501	4	12713	3:21	3:00	2:97	2:87	2:76	2:49	2:22	1:95	55	11:03:31 RWP	PCC	Excel.	None	2251				
D	29968	2	6321	2:27	2:31	2:08	2:00	1:97	1:75	1:61	1:44	55	11:04:56 RWP	PCC	Excel.	None	1584				
D	29968	3	9474	3:43	3:47	3:16	3:07	2:94	2:67	2:43	2:18	55	11:05:01 RWP	PCC	Excel.	None	1571				
D	29968	4	12649	4:54	4:56	4:19	4:04	3:51	3:58	3:22	2:91	55	11:05:08 RWP	PCC	Excel.	None	1584				
D	30507	2	6358	2:29	2:21	2:11	2:00	1:95	1:76	1:58	1:41	55	11:06:40 RWP	PCC	Excel.	None	1582				
D	30507	3	9485	3:41	3:30	3:16	3:08	2:94	2:68	2:39	2:10	55	11:06:45 RWP	PCC	Excel.	None	1579				
D	30507	4	12686	4:50	4:33	4:21	4:05	3:81	3:56	3:19	2:79	55	11:06:52 RWP	PCC	Excel.	None	1602				
C	Comment	at 30512	ft	Time: 11:07:39	:Deflection is not decreasing																
D	30512	2	6278	4:08	2:17	3:20	2:88	2:71	2:32	1:98	1:66	56	11:07:41 RWP	PCC	Excel.	None	867				
C	Comment	at 30512	ft	Time: 11:07:47	:Deflection is not decreasing																
D	30512	3	9391	6:12	3:23	4:78	4:45	4:12	3:49	2:97	2:50	56	11:07:54 RWP	PCC	Excel.	None	873				
C	Comment	at 30512	ft	Time: 11:08:01	:Deflection is not decreasing																
D	30512	4	12525	8:10	4:23	6:38	5:87	5:48	4:66	3:97	3:33	56	11:08:05 RWP	PCC	Excel.	None	879				
C	Comment	at 30512	ft	Time: 11:08:15	:Load transfer																
D	30997	2	6311	3:34	3:21	2:84	2:58	2:38	1:98	1:63	1:38	55	11:09:42 RWP	PCC	Excel.	None	1074				
D	30997	3	9439	5:08	4:84	4:32	4:01	3:64	3:11	2:53	2:08	55	11:09:47 RWP	PCC	Excel.	None	1057				
D	30997	4	12604	6:81	6:48	5:84	5:37	4:89	4:11	3:43	2:81	55	11:09:54 RWP	PCC	Excel.	None	1052				
D	31529	2	6388	1:84	1:74	1:69	1:65	1:60	1:46	1:34	1:17	56	11:11:31 RWP	PCC	Excel.	None	1574				
D	31529	3	9544	2:76	2:62	2:55	2:53	2:42	2:19	1:97	1:76	56	11:11:35 RWP	PCC	Excel.	None	1564				
D	31529	4	12736	3:64	3:45	3:41	3:31	3:19	2:93	2:65	2:33	56	11:11:43 RWP	PCC	Excel.	None	1588				
D	32002	2	6293	2:19	2:01	1:94	1:94	1:88	1:73	1:54	1:39	56	11:13:24 RWP	PCC	Excel.	None	1635				
D	32002	3	9452	3:19	3:18	3:02	2:98	2:86	2:61	2:36	2:08	56	11:13:30 RWP	PCC	Excel.	None	1632				

Page 4

D	50509	2	6276	3.46	3.29	3.16	2.94	2.79	2.34	1.88	1.52	58	Brooklyn SB 17 Nov09	PCC	Exce].	None	1031		
D	50509	3	9431	5.24	4.96	4.82	4.61	4.26	3.57	2.91	2.30	59	12:22:04 RWP	PCC	Exce].	None	1023		
D	50509	4	12574	7.01	6.63	6.49	6.14	5.73	4.82	3.95	3.09	59	12:22:10 RWP	PCC	Exce].	None	1020		
C	Comment at 50639 ft Time: 12:23:23 :End																		

IA9	EB	Table	100	86	73	73	85	11:27:41	None	628						
D	-10010	2	5849	134	127	123	117	111	100	86	73	85	11:27:41 <td>RVP <td>None</td> <td>628</td> </td>	RVP <td>None</td> <td>628</td>	None	628
D	-10010	3	9038	214	202	195	187	178	158	137	117	85	11:27:46 <td>RVP <td>None</td> <td>611</td> </td>	RVP <td>None</td> <td>611</td>	None	611
D	-10010	4	12277	292	274	265	256	244	218	189	161	85	11:27:53 <td>RVP <td>None</td> <td>607</td> </td>	RVP <td>None</td> <td>607</td>	None	607
D	-11010	3	5846	164	147	138	130	126	110	94	77	82	11:29:42 <td>RVP <td>None</td> <td>516</td> </td>	RVP <td>None</td> <td>516</td>	None	516
D	-11010	3	9010	258	232	220	209	200	175	150	124	82	11:29:49 <td>RVP <td>None</td> <td>505</td> </td>	RVP <td>None</td> <td>505</td>	None	505
D	-11010	4	12207	352	317	301	287	274	240	206	172	82	11:29:56 <td>RVP <td>None</td> <td>501</td> </td>	RVP <td>None</td> <td>501</td>	None	501
D	-12039	2	5846	201	156	138	125	117	99	82	68	85	11:31:19 <td>RVP <td>None</td> <td>420</td> </td>	RVP <td>None</td> <td>420</td>	None	420
D	-12039	3	8976	309	245	218	199	186	157	130	107	85	11:31:25 <td>RVP <td>None</td> <td>419</td> </td>	RVP <td>None</td> <td>419</td>	None	419
D	-12039	4	12134	417	334	299	274	256	216	180	148	85	11:31:33 <td>RVP <td>None</td> <td>420</td> </td>	RVP <td>None</td> <td>420</td>	None	420
D	-13053	2	5858	251	197	169	144	129	110	94	75	84	11:33:23 <td>RVP <td>None</td> <td>337</td> </td>	RVP <td>None</td> <td>337</td>	None	337
D	-13053	3	9016	368	298	260	227	205	175	149	120	84	11:33:29 <td>RVP <td>None</td> <td>354</td> </td>	RVP <td>None</td> <td>354</td>	None	354
D	-13053	4	12241	480	366	350	308	281	241	206	166	84	11:33:36 <td>RVP <td>None</td> <td>368</td> </td>	RVP <td>None</td> <td>368</td>	None	368
D	-14062	2	5813	237	209	191	175	165	139	112	87	83	11:35:08 <td>RVP <td>None</td> <td>354</td> </td>	RVP <td>None</td> <td>354</td>	None	354
D	-14062	3	8932	368	326	300	277	260	220	176	136	83	11:35:13 <td>RVP <td>None</td> <td>350</td> </td>	RVP <td>None</td> <td>350</td>	None	350
D	-14062	4	12146	497	440	406	378	354	301	240	185	83	11:35:20 <td>RVP <td>None</td> <td>353</td> </td>	RVP <td>None</td> <td>353</td>	None	353
D	-15002	2	5814	245	212	190	165	149	123	103	82	84	11:36:52 <td>RVP <td>None</td> <td>342</td> </td>	RVP <td>None</td> <td>342</td>	None	342
D	-15002	3	8956	373	324	293	259	233	194	162	129	84	11:36:58 <td>RVP <td>None</td> <td>347</td> </td>	RVP <td>None</td> <td>347</td>	None	347
D	-15002	4	12130	494	430	391	349	315	264	221	176	84	11:37:06 <td>RVP <td>None</td> <td>354</td> </td>	RVP <td>None</td> <td>354</td>	None	354
D	-16025	2	5910	181	154	145	133	127	111	93	78	82	11:38:47 <td>RVP <td>None</td> <td>471</td> </td>	RVP <td>None</td> <td>471</td>	None	471
D	-16025	3	9052	285	243	229	211	201	175	148	124	82	11:38:54 <td>RVP <td>None</td> <td>488</td> </td>	RVP <td>None</td> <td>488</td>	None	488
D	-16025	4	12263	389	329	310	290	274	238	203	169	82	11:39:02 <td>RVP <td>None</td> <td>455</td> </td>	RVP <td>None</td> <td>455</td>	None	455
D	-16954	2	5847	206	160	143	125	115	98	80	65	84	11:41:05 <td>RVP <td>None</td> <td>410</td> </td>	RVP <td>None</td> <td>410</td>	None	410
D	-16954	3	8958	313	249	224	202	184	157	129	104	84	11:41:10 <td>RVP <td>None</td> <td>414</td> </td>	RVP <td>None</td> <td>414</td>	None	414
D	-16954	4	12196	423	340	307	280	257	218	180	145	84	11:41:17 <td>RVP <td>None</td> <td>417</td> </td>	RVP <td>None</td> <td>417</td>	None	417
C	Comment at -16954 ft.	Time:	11:41:51	just before HWY59 pavement change												
D	-18046	2	5844	160	145	129	117	110	96	81	64	84	11:43:37 <td>RVP <td>None</td> <td>527</td> </td>	RVP <td>None</td> <td>527</td>	None	527
D	-18046	3	8977	249	222	203	186	174	152	128	102	84	11:43:42 <td>RVP <td>None</td> <td>521</td> </td>	RVP <td>None</td> <td>521</td>	None	521
D	-18046	4	12222	338	301	276	256	238	209	175	141	84	11:43:49 <td>RVP <td>None</td> <td>522</td> </td>	RVP <td>None</td> <td>522</td>	None	522
D	-19046	2	5879	144	133	128	122	119	103	74	61	82	11:45:23 <td>RVP <td>None</td> <td>590</td> </td>	RVP <td>None</td> <td>590</td>	None	590
D	-19046	3	9068	228	212	204	196	190	165	119	97	82	11:45:30 <td>RVP <td>None</td> <td>575</td> </td>	RVP <td>None</td> <td>575</td>	None	575
D	-19046	4	12304	314	291	282	272	262	228	165	135	82	11:45:38 <td>RVP <td>None</td> <td>566</td> </td>	RVP <td>None</td> <td>566</td>	None	566
D	-20021	2	5856	182	157	146	139	131	112	92	75	84	11:47:17 <td>RVP <td>None</td> <td>465</td> </td>	RVP <td>None</td> <td>465</td>	None	465
D	-20021	3	9020	280	245	230	220	207	177	147	119	84	11:47:22 <td>RVP <td>None</td> <td>466</td> </td>	RVP <td>None</td> <td>466</td>	None	466
D	-20021	4	12241	376	334	314	301	285	245	202	166	84	11:47:29 <td>RVP <td>None</td> <td>470</td> </td>	RVP <td>None</td> <td>470</td>	None	470
D	-21041	2	5924	135	125	121	116	113	100	86	72	85	11:49:01 <td>RVP <td>None</td> <td>635</td> </td>	RVP <td>None</td> <td>635</td>	None	635
D	-21041	3	9097	212	197	191	186	178	158	136	114	85	11:49:07 <td>RVP <td>None</td> <td>619</td> </td>	RVP <td>None</td> <td>619</td>	None	619
D	-21041	4	12320	289	269	261	254	244	217	187	158	85	11:49:15 <td>RVP <td>None</td> <td>617</td> </td>	RVP <td>None</td> <td>617</td>	None	617
D	-22034	2	5974	75	68	66	63	62	56	50	44	83	11:50:41 <td>RVP <td>None</td> <td>1144</td> </td>	RVP <td>None</td> <td>1144</td>	None	1144
D	-22034	3	9195	118	107	104	101	97	88	79	68	83	11:50:46 <td>RVP <td>None</td> <td>1123</td> </td>	RVP <td>None</td> <td>1123</td>	None	1123
D	-22034	4	12523	162	148	142	140	132	121	108	93	83	11:50:53 <td>RVP <td>None</td> <td>1118</td> </td>	RVP <td>None</td> <td>1118</td>	None	1118
D	-23059	2	5874	128	121	115	110	105	93	81	68	85	11:52:30 <td>RVP <td>None</td> <td>662</td> </td>	RVP <td>None</td> <td>662</td>	None	662
D	-23059	3	9028	205	189	181	173	164	146	128	107	85	11:52:36 <td>RVP <td>None</td> <td>637</td> </td>	RVP <td>None</td> <td>637</td>	None	637
D	-23059	4	12238	291	259	249	237	224	200	176	149	85	11:52:44 <td>RVP <td>None</td> <td>629</td> </td>	RVP <td>None</td> <td>629</td>	None	629
D	-24009	2	5920	140	115	108	98	89	77	64	51	83	11:54:50 <td>RVP <td>None</td> <td>611</td> </td>	RVP <td>None</td> <td>611</td>	None	611
D	-24009	3	9097	223	183	172	158	144	123	102	81	83	11:54:55 <td>RVP <td>None</td> <td>589</td> </td>	RVP <td>None</td> <td>589</td>	None	589
D	-24009	4	12356	311	254	239	222	201	173	145	115	83	11:55:02 <td>RVP <td>None</td> <td>575</td> </td>	RVP <td>None</td> <td>575</td>	None	575
D	-25001	2	5872	138	111	100	92	86	75	64	53	82	11:56:37 <td>RVP <td>None</td> <td>613</td> </td>	RVP <td>None</td> <td>613</td>	None	613
D	-25001	3	9106	218	176	158	146	137	121	102	85	82	11:56:42 <td>RVP <td>None</td> <td>602</td> </td>	RVP <td>None</td> <td>602</td>	None	602
D	-25001	4	12288	317	281	271	261	251	217	158	127	81	11:57:28 <td>RVP <td>None</td> <td>617</td> </td>	RVP <td>None</td> <td>617</td>	None	617
D	-25001	3	9088	216	176	158	147	137	120	103	85	81	11:57:34 <td>RVP <td>None</td> <td>609</td> </td>	RVP <td>None</td> <td>609</td>	None	609
D	-25001	4	12304	296	241	217	203	190	168	142	118	81	11:57:40 <td>RVP <td>None</td> <td>600</td> </td>	RVP <td>None</td> <td>600</td>	None	600
D	-26045	2	5873	116	101	90	80	74	62	51	40	79	11:58:49 <td>RVP <td>None</td> <td>728</td> </td>	RVP <td>None</td> <td>728</td>	None	728
D	-26045	3	9036	179	156	141	128	118	100	81	63	79	11:58:55 <td>RVP <td>None</td> <td>728</td> </td>	RVP <td>None</td> <td>728</td>	None	728
D	-26045	4	12272	242	212	192	175	161	137	112	87	79	12:00:03 <td>RVP <td>None</td> <td>732</td> </td>	RVP <td>None</td> <td>732</td>	None	732
D	-27058	2	5897	146	128	120	110	101	79	61	46	84	12:01:32 <td>RVP <td>None</td> <td>584</td> </td>	RVP <td>None</td> <td>584</td>	None	584
D	-27058	3	9039	225	195	182	168	153	120	95	71	84	12:01:38 <td>RVP <td>None</td> <td>580</td> </td>	RVP <td>None</td> <td>580</td>	None	580
D	-27058	4	12279	305	263	244	224	203	160	127	96	84	12:01:46 <td>RVP <td>None</td> <td>582</td> </td>	RVP <td>None</td> <td>582</td>	None	582
D	-27984	2	5906	114	97	91	85	82	71	60	50	80	12:03:35 <td>RVP <td>None</td> <td>748</td> </td>	RVP <td>None</td> <td>748</td>	None	748
D	-27984	3	9021	178	154	145	136	129	113	96	80	80	12:03:40 <td>RVP <td>None</td> <td>733</td> </td>	RVP <td>None</td> <td>733</td>	None	733
D	-27984	4	12308	243	210	199	190	179	157	134	111	80	12:03:47 <td>RVP <td>None</td> <td>731</td> </td>	RVP <td>None</td> <td>731</td>	None	731
D	-29038	2	5891	196	135	115	104	96	80	66	52	80	12:05:19 <td>RVP <td>None</td> <td>435</td> </td>	RVP <td>None</td> <td>435</td>	None	435
D	-29038	3	9083	296	209	182	165	151	126	104	83	80	12:05:25 <td>RVP <td>None</td> <td>443</td> </td>	RVP <td>None</td> <td>443</td>	None	443
D	-29038	4	12351	337	284	251	229	209	176	144	116	80	12:05:32 <td>RVP <td>None</td> <td>450</td> </td>	RVP <td>None</td> <td>450</td>	None	450

D -11035	2	5885	230	194	171	148	133	106	76	51	73	91	12:57:37	RMP	None	369
D -11035	3	9014	558	302	268	234	211	168	122	82	73	91	12:57:44	RMP	None	363
D -11035	4	12218	488	408	366	322	289	231	168	112	73	91	12:57:52	RMP	None	362
D -12015	2	5894	114	98	91	87	81	70	58	46	72	91	12:59:14	RMP	None	745
D -12015	3	9101	182	156	146	139	131	112	92	73	72	91	12:59:21	RMP	None	723
D -12015	4	12331	250	212	201	194	181	155	128	101	72	91	12:59:29	RMP	None	714
D -13028	2	5834	155	134	130	123	118	103	87	70	73	92	13:01:08	RMP	None	544
D -13028	3	8980	247	215	206	195	187	163	138	112	73	92	13:01:14	RMP	None	544
D -13028	4	12195	340	295	282	270	258	223	192	135	73	92	13:01:21	RMP	None	519
D -14029	2	5840	150	133	123	114	109	94	79	62	74	92	13:02:45	RMP	None	562
D -14029	3	9017	238	208	194	181	172	150	126	100	74	92	13:02:51	RMP	None	547
D -15033	2	5840	185	144	129	111	102	83	68	53	74	94	13:04:27	RMP	None	543
D -15033	3	9003	284	224	200	176	161	132	109	86	74	94	13:04:33	RMP	None	456
D -15033	4	12229	385	304	274	243	221	183	152	120	74	94	13:04:40	RMP	None	459
D -16037	2	5846	210	158	150	127	114	92	72	54	74	94	13:06:09	RMP	None	402
D -16037	3	8926	330	256	237	205	187	150	118	89	74	94	13:06:15	RMP	None	390
D -16037	4	12103	455	357	331	290	263	212	167	128	74	94	13:06:22	RMP	None	385
D -17069	2	5893	135	112	105	101	95	83	70	58	74	93	13:07:58	RMP	None	630
D -17069	3	9046	214	178	168	161	153	134	113	92	74	93	13:08:05	RMP	None	612
D -17069	4	12321	295	245	234	225	214	187	159	130	74	96	13:08:12	RMP	None	604
D -18043	2	5878	113	89	78	68	62	50	42	34	74	96	13:09:31	RMP	None	750
D -18043	3	9021	176	140	123	110	101	80	67	55	74	96	13:09:37	RMP	None	742
D -18043	4	12290	239	192	170	153	140	111	93	76	74	96	13:09:45	RMP	None	742
D -19048	2	5827	105	82	72	65	62	51	41	34	73	90	13:11:10	RMP	None	798
D -19048	3	9028	165	128	115	107	99	82	66	54	73	90	13:11:15	RMP	None	791
D -19048	4	12258	223	174	158	147	136	114	92	74	73	90	13:11:22	RMP	None	793
D -19997	2	5868	175	146	121	105	97	83	70	57	74	93	13:12:52	RMP	None	484
D -19997	3	9052	266	223	190	166	154	132	111	91	74	93	13:12:58	RMP	None	492
D -19997	4	12341	356	299	257	229	211	182	155	127	74	93	13:13:05	RMP	None	501
D -21040	2	5859	130	104	95	77	69	57	48	39	74	92	13:14:53	RMP	None	652
D -21040	3	9076	202	163	147	123	110	91	77	63	74	92	13:14:59	RMP	None	650
D -21040	4	12383	275	222	200	172	153	128	108	88	74	92	13:15:07	RMP	None	650
D -22070	2	5882	112	94	94	89	86	77	67	56	73	89	13:16:23	RMP	None	761
D -22070	3	8999	176	151	148	143	136	122	107	90	73	89	13:16:29	RMP	None	737
D -22070	4	12250	240	207	203	197	187	170	148	125	73	89	13:16:36	RMP	None	737
D -23007	2	5776	139	115	104	97	92	78	67	56	72	87	13:18:03	RMP	None	601
D -23007	3	8963	220	182	166	156	147	126	107	89	72	87	13:18:09	RMP	None	588
D -23007	4	12246	304	248	227	217	203	175	149	124	72	87	13:18:15	RMP	None	582
D -24034	2	5773	146	136	133	120	116	103	89	74	72	86	13:19:43	RMP	None	572
D -24034	3	8919	231	213	206	192	184	164	142	119	72	86	13:19:49	RMP	None	557
D -24034	4	12129	318	286	279	265	254	227	197	165	72	86	13:19:56	RMP	None	551
D -25040	2	5776	154	132	128	121	113	97	80	64	71	84	13:21:35	RMP	None	543
D -25040	3	8908	245	212	204	193	182	153	129	104	71	84	13:21:40	RMP	None	524
D -25040	4	12146	339	293	282	268	250	216	181	147	71	84	13:21:47	RMP	None	517
D -26058	2	5924	162	141	133	123	120	105	87	71	70	85	13:23:09	RMP	None	528
D -26058	3	9055	258	225	214	203	192	169	140	114	70	85	13:23:15	RMP	None	506
D -26058	4	12362	355	311	296	281	266	233	197	161	70	85	13:23:22	RMP	None	503
D -27023	2	5844	142	128	126	123	119	108	95	82	69	89	13:24:52	RMP	None	596
D -27023	3	9005	223	200	198	193	188	169	149	129	69	89	13:24:58	RMP	None	583
D -27023	4	12299	307	273	270	266	257	233	206	177	69	89	13:25:05	RMP	None	579
D -28023	2	5844	184	166	163	146	136	116	100	85	69	83	13:26:43	RMP	None	459
D -28023	3	9028	288	263	253	230	215	184	159	135	69	83	13:26:49	RMP	None	453
D -28023	4	12266	394	359	345	318	295	253	220	188	69	83	13:26:56	RMP	None	450
D -29052	2	5900	196	159	146	120	108	90	76	61	68	85	13:28:23	RMP	None	435
D -29052	3	9079	301	246	227	190	170	144	121	98	68	85	13:28:29	RMP	None	435
D -29052	4	12341	409	335	308	263	235	199	168	136	68	85	13:28:37	RMP	None	436
D -30039	2	5857	242	188	177	146	133	111	87	67	67	80	13:30:05	RMP	None	350
D -30039	3	8960	364	293	276	231	211	176	139	109	67	80	13:30:17	RMP	None	356
D -30039	4	12218	486	398	374	321	291	245	195	149	67	80	13:30:17	RMP	None	363
D -31015	2	5882	177	153	141	134	127	108	93	78	67	78	13:31:46	RMP	None	480
D -31015	3	9101	272	239	220	209	198	170	147	124	67	78	13:31:52	RMP	None	483
D -31015	4	12409	369	324	299	286	271	234	203	171	67	78	13:32:00	RMP	None	486

IA9	WB	Cable	AC	73	87	105	124	131	152	166	196	5879	2	D	-32016
AC	13:33:23	RMP	67	73	87	105	124	131	152	166	196	5879	2	D	-32016
AC	13:33:30	RMP	67	115	139	195	207	237	256	299	6037	3	D	-32016	
AC	13:33:38	RMP	67	159	228	267	286	314	341	402	12282	4	D	-32016	
AC	13:35:34	RMP	67	159	89	130	137	130	144	137	5835	2	D	-33011	
AC	13:35:40	RMP	67	140	140	167	207	215	242	242	8972	3	D	-33011	
AC	13:35:48	RMP	67	159	195	267	231	285	308	293	12215	3	D	-33011	
AC	13:37:35	RMP	67	86	117	138	134	138	155	146	5770	2	D	-34050	
AC	13:37:40	RMP	67	139	164	220	212	217	230	247	8878	2	D	-34050	
AC	13:37:48	RMP	67	190	225	303	290	312	335	373	12129	4	D	-34050	
AC	13:59:30	RMP	66	75	92	110	128	135	152	162	5737	2	D	-35022	
AC	13:59:35	RMP	66	120	146	203	174	253	237	321	8884	3	D	-35022	
AC	13:59:42	RMP	66	166	202	280	240	299	347	324	12145	4	D	-35022	
AC	13:41:13	RMP	65	80	80	108	112	126	142	173	5836	2	D	-36013	
AC	13:41:20	RMP	65	104	125	169	147	196	219	196	8959	2	D	-36013	
AC	13:41:28	RMP	65	144	171	201	216	246	300	267	12283	4	D	-36013	
AC	13:43:08	RMP	65	82	82	112	117	121	151	126	5769	2	D	-37017	
AC	13:43:14	RMP	65	105	129	175	175	187	202	230	8902	2	D	-37017	
AC	13:43:21	RMP	65	146	178	242	242	277	310	277	12192	4	D	-37017	
AC	13:44:52	RMP	65	51	79	92	92	97	104	104	5860	2	D	-38029	
AC	13:44:57	RMP	65	81	104	148	127	157	173	166	9024	3	D	-38029	
AC	13:45:04	RMP	65	114	146	205	219	229	259	239	12270	4	D	-38029	
AC	13:46:35	RMP	64	79	99	135	118	149	154	149	5722	2	D	-39000	
AC	13:46:41	RMP	64	128	159	216	216	236	248	285	8891	2	D	-39000	
AC	13:46:48	RMP	64	177	219	297	216	314	323	337	12115	4	D	-39000	
AC	13:48:23	RMP	64	79	93	129	112	137	150	169	5842	2	D	-40004	
AC	13:48:29	RMP	64	124	147	203	177	217	237	262	8960	3	D	-40004	
AC	13:48:36	RMP	64	172	205	280	244	300	323	355	12159	4	D	-40004	
AC	13:50:03	RMP	64	61	76	112	125	140	157	140	5882	2	D	-41011	
AC	13:50:15	RMP	64	97	122	199	178	199	221	245	9018	3	D	-41011	
AC	13:50:44	RMP	64	135	169	245	245	274	303	303	12281	4	D	-41011	
AC	13:51:44	RMP	64	72	89	124	124	152	144	162	5924	2	D	-42001	
AC	13:51:49	RMP	64	113	143	196	170	209	227	252	9056	3	D	-42001	
AC	13:51:56	RMP	64	162	198	270	237	289	311	344	12348	4	D	-42001	
AC	13:53:05	RMP	63	73	88	125	125	135	152	172	5841	2	D	-42536	
AC	13:53:10	RMP	63	116	140	198	166	240	268	240	9023	3	D	-42536	
AC	13:53:17	RMP	63	162	195	273	298	333	367	428	12282	4	D	-42536	

LANE CENTER OF SECTION	ALL TESTING DONE IN CENTER OF LANE	TRANSVERSE	LOAD TRANSFER	IN CENTER OF SECTION	TRANSVERSE	LOAD TRANSFER	LANE CENTER OF SECTION							
4584	2	6475	2.39	2.18	2.20	2.01	2.05	1.82	1.62	1.42	52	Exc'l.	None	1539
4584	3	9686	3.62	3.30	3.30	3.24	3.07	2.82	2.47	2.16	52	Exc'l.	None	1520
4584	4	12872	4.84	4.39	4.45	4.30	4.11	3.80	3.36	2.93	52	Exc'l.	None	1512
5098	2	6484	2.00	1.91	1.81	1.72	1.68	1.53	1.38	1.26	52	Exc'l.	None	1848
5098	3	9694	3.04	2.89	2.76	2.67	2.55	2.34	2.13	1.93	52	Exc'l.	None	1810
5098	4	12888	4.09	3.86	3.69	3.54	3.40	3.12	2.85	2.58	52	Exc'l.	None	1791
5100	2	6468	2.10	1.99	1.89	1.74	1.71	1.56	1.38	1.26	53	Exc'l.	None	1753
5100	3	9673	3.17	3.00	2.82	2.57	2.57	2.29	2.07	1.86	53	Exc'l.	None	1733
5100	4	12911	4.36	4.05	3.88	3.72	3.51	3.18	2.88	2.58	53	Exc'l.	None	1684
Comment at 5100 ft			Time: 10:43:55	LOAD TRANSFER										
5591	2	6486	2.71	2.55	2.62	2.45	2.51	2.18	1.85	1.62	53	Exc'l.	None	1359
5591	3	9685	4.15	3.85	3.95	3.92	3.78	3.32	2.93	2.42	53	Exc'l.	None	1327
5591	4	12893	5.33	5.11	5.29	5.17	5.04	4.46	3.79	3.22	53	Exc'l.	None	1325
6024	2	6476	2.68	2.23	2.18	1.94	1.88	1.61	1.40	1.23	53	Exc'l.	None	1349
6024	3	9740	3.67	3.41	3.30	3.12	2.89	2.49	2.18	1.89	53	Exc'l.	None	1510
6024	4	12945	4.86	4.49	4.37	4.08	3.78	3.27	2.84	2.46	53	Exc'l.	None	1515
6578	2	6491	2.13	2.14	2.00	1.82	1.81	1.60	1.39	1.21	53	Exc'l.	None	1730
6578	3	9752	3.38	3.30	3.08	2.98	2.80	2.47	2.17	1.87	53	Exc'l.	None	1640
6578	4	12929	4.64	4.44	4.20	4.01	3.81	3.38	2.97	2.55	53	Exc'l.	None	1583
7048	2	6514	1.78	1.72	1.72	1.61	1.62	1.49	1.35	1.24	53	Exc'l.	None	2068
7048	3	9779	2.69	2.55	2.54	2.52	2.42	2.25	2.05	1.85	53	Exc'l.	None	2079
7048	4	12983	3.55	3.32	3.35	3.28	3.21	2.99	2.72	2.44	53	Exc'l.	None	2324
7557	2	6507	1.59	1.54	1.44	1.33	1.33	1.17	1.07	0.96	53	Exc'l.	None	2312
7557	3	9767	2.40	2.28	2.17	2.11	1.98	1.78	1.60	1.45	53	Exc'l.	None	2318
8061	2	6495	2.23	2.07	2.03	1.87	1.81	1.61	1.45	1.32	53	Exc'l.	None	1633
8061	3	9738	3.41	3.17	3.09	3.00	2.80	2.48	2.23	2.03	53	Exc'l.	None	1623
8061	4	12899	4.55	4.21	4.14	3.97	3.75	3.33	3.02	2.73	53	Exc'l.	None	1614
8581	2	6522	1.85	1.77	1.72	1.61	1.60	1.47	1.37	1.25	53	Exc'l.	None	2004
8581	3	9767	2.77	2.66	2.54	2.51	2.38	2.21	2.04	1.87	53	Exc'l.	None	2003
8581	4	12982	3.64	3.48	3.37	3.27	3.16	2.95	2.71	2.47	53	Exc'l.	None	2025
9060	2	6475	2.22	2.09	2.03	1.89	1.86	1.66	1.45	1.28	53	Exc'l.	None	1659
9060	3	9709	3.38	3.19	3.07	3.02	2.85	2.58	2.25	1.97	53	Exc'l.	None	1633
9060	4	12886	4.51	4.22	4.13	3.94	3.79	3.45	3.03	2.64	53	Exc'l.	None	1624
9545	2	6480	2.22	2.12	2.01	1.86	1.79	1.60	1.45	1.32	53	Exc'l.	None	1659
9545	3	9692	3.41	3.19	3.04	2.90	2.73	2.45	2.22	1.99	53	Exc'l.	None	1618
9545	4	12872	4.53	4.23	4.04	3.82	3.61	3.25	2.94	2.61	53	Exc'l.	None	1615
10019	2	6500	2.91	2.70	2.67	2.49	2.37	2.10	1.85	1.63	53	Exc'l.	None	1272
10019	3	9722	4.41	4.06	4.05	3.87	3.61	3.20	2.83	2.50	53	Exc'l.	None	1254
10019	4	12872	5.89	5.40	5.41	5.15	4.83	4.30	3.80	3.35	53	Exc'l.	None	1243
10021	2	6504	2.84	2.65	2.60	2.42	2.34	2.03	1.81	1.58	53	Exc'l.	None	1301
10021	3	9739	4.30	3.99	3.94	3.79	3.55	3.13	2.76	2.42	53	Exc'l.	None	1288
10021	4	12876	5.70	5.27	5.24	4.97	4.72	4.20	3.70	3.24	53	Exc'l.	None	1284
Comment at 10021 ft			Time: 11:00:46	LOAD TRANSFER										
10548	2	6512	2.37	2.23	2.26	2.14	2.13	1.98	1.77	1.62	53	Exc'l.	None	1565
10548	3	9747	3.54	3.32	3.27	3.28	3.18	2.94	2.67	2.38	53	Exc'l.	None	1565
10548	4	12926	4.70	4.39	4.44	4.32	4.22	3.92	3.52	3.18	53	Exc'l.	None	1562
11052	2	6509	2.64	2.53	2.41	2.24	2.13	1.86	1.65	1.41	53	Exc'l.	None	1403
11052	3	9720	4.05	3.84	3.67	3.49	3.26	2.89	2.49	2.19	53	Exc'l.	None	1365
11052	4	12864	5.45	5.12	4.93	4.64	4.39	3.89	3.41	2.95	53	Exc'l.	None	1342
11551	2	6467	2.94	2.71	2.66	2.40	2.33	1.99	1.71	1.50	53	Exc'l.	None	1251
11551	3	9670	4.43	4.08	3.98	3.78	3.49	3.02	2.60	2.26	53	Exc'l.	None	1242
11551	4	12837	5.88	5.39	5.28	4.97	4.65	4.05	3.50	3.02	53	Exc'l.	None	1241
12085	2	6478	2.56	2.43	2.39	2.21	2.16	1.95	1.70	1.42	53	Exc'l.	None	1440
12085	3	9696	3.92	3.68	3.61	3.47	3.29	2.98	2.57	2.14	53	Exc'l.	None	1407
12085	4	12853	5.27	4.90	4.85	4.64	4.42	4.01	3.48	2.85	53	Exc'l.	None	1385
12538	2	6466	2.49	2.41	2.19	2.04	1.96	1.73	1.55	1.36	53	Exc'l.	None	1474
12538	3	9683	3.82	3.68	3.35	3.15	3.00	2.65	2.38	2.07	53	Exc'l.	None	1439
12538	4	12832	5.16	4.93	4.50	4.25	4.02	3.59	3.18	2.80	53	Exc'l.	None	1414
13032	2	6476	2.64	2.36	2.48	2.35	2.32	2.07	1.78	1.57	53	Exc'l.	None	1396
13032	3	9710	3.97	3.53	3.76	3.63	3.48	3.11	2.71	2.36	53	Exc'l.	None	1390
13032	4	12875	5.25	4.64	5.00	4.78	4.61	4.18	3.61	3.13	53	Exc'l.	None	1394
Comment at 13032 ft			Time: 11:09:51	ALL TESTING DONE IN CENTER OF LANE										

D	13543	2	6479	3.06	2.87	2.86	2.68	2.55	2.25	1.93	1.69	53	Sibley Hwy/3 EB NOV13	AC	Excel	None	1202
D	13543	3	9716	4.70	4.38	4.34	4.13	3.93	3.45	2.98	2.55	53	11:11:10 RWP	AC	Excel	None	1175
D	13543	4	12874	6.35	5.89	5.86	5.60	5.27	4.66	4.05	3.45	53	11:11:15 RWP	AC	Excel	None	1152
D	14117	2	6454	2.52	2.39	2.45	2.38	2.34	2.20	1.91	1.62	53	11:12:46 RWP	AC	Excel	None	1458
D	14117	3	9670	3.83	3.59	3.66	3.59	3.52	3.31	2.84	2.45	53	11:12:51 RWP	AC	Excel	None	1436
D	14117	4	12829	5.06	4.69	4.81	4.69	4.61	4.33	3.73	3.15	53	11:12:58 RWP	AC	Excel	None	1442
D	14541	2	6493	1.99	1.88	1.80	1.68	1.91	1.42	1.24	1.08	53	11:14:28 RWP	AC	Excel	None	1855
D	14541	3	9710	3.01	2.86	2.75	2.61	2.45	2.16	1.86	1.63	53	11:14:35 RWP	AC	Excel	None	1832
D	14541	4	12904	3.99	3.76	3.65	3.48	3.27	2.86	2.51	2.19	53	11:14:41 RWP	AC	Excel	None	1838
D	14542	2	6489	2.17	1.99	1.88	1.74	1.66	1.45	1.28	1.10	53	11:15:22 RWP	AC	Excel	None	1607
D	14542	3	9711	3.27	2.98	2.83	2.68	2.50	2.18	1.91	1.67	53	11:15:27 RWP	AC	Excel	None	1782
ft Time: 11:15:58 :LOAD TRANSFER																	
D	14541	2	6502	2.10	1.97	1.88	1.72	1.66	1.45	1.27	1.10	53	11:16:25 RWP	AC	Excel	None	1759
D	14541	3	9717	3.24	2.98	2.83	2.69	2.52	2.18	1.92	1.66	53	11:16:30 RWP	AC	Excel	None	1705
D	14541	4	12863	4.35	3.94	3.76	3.55	3.36	2.93	2.56	2.21	53	11:16:36 RWP	AC	Excel	None	1681
ft Time: 11:16:46 :LOAD TRANSFER RERUN																	
D	15090	2	6469	2.35	2.23	2.26	2.17	2.15	2.07	1.88	1.70	53	11:18:08 RWP	AC	Excel	None	1563
D	15090	3	9690	3.52	3.34	3.39	3.34	3.30	3.13	2.83	2.56	53	11:18:14 RWP	AC	Excel	None	1566
D	15090	4	12878	5.28	4.81	4.96	4.81	4.78	4.45	3.84	3.27	53	11:19:04 RWP	AC	Excel	None	1612
D	15090	5	9672	3.50	3.32	3.39	3.32	3.28	3.09	2.83	2.55	53	11:19:10 RWP	AC	Excel	None	1570
D	15090	6	12878	4.69	4.35	4.47	4.40	4.34	4.14	3.78	3.39	53	11:19:17 RWP	AC	Excel	None	1560
D	15664	2	6485	2.34	2.20	2.24	2.12	2.08	1.89	1.61	1.34	53	11:20:34 RWP	AC	Excel	None	1578
D	15664	3	9701	3.65	3.34	3.44	3.32	3.17	2.85	2.43	2.04	53	11:20:39 RWP	AC	Excel	None	1511
D	15664	4	12881	4.94	4.47	4.62	4.43	4.27	3.85	3.29	2.75	53	11:20:45 RWP	AC	Excel	None	1483
D	16018	2	6479	2.57	2.55	2.38	2.22	2.17	1.92	1.68	1.49	53	11:22:11 RWP	AC	Excel	None	1434
D	16018	3	9700	3.92	3.84	3.62	3.51	3.27	2.94	2.58	2.26	53	11:22:16 RWP	AC	Excel	None	1406
D	16018	4	12847	5.16	4.87	4.87	4.68	4.44	3.96	3.50	3.04	53	11:22:27 RWP	AC	Excel	None	1376
D	16608	2	6494	2.07	2.03	1.91	1.81	1.78	1.64	1.50	1.35	53	11:23:55 RWP	AC	Excel	None	1782
D	16608	3	9721	3.10	2.85	2.85	2.80	2.67	2.46	2.26	2.03	53	11:24:00 RWP	AC	Excel	None	1785
D	16608	4	12913	4.12	3.96	3.78	3.66	3.54	3.29	3.00	2.72	53	11:24:07 RWP	AC	Excel	None	1784
D	17560	2	6472	2.48	2.35	2.25	2.12	2.07	1.78	1.53	1.34	53	11:26:11 RWP	AC	Excel	None	1486
D	17560	3	9675	3.83	3.59	3.48	3.33	3.15	2.75	2.38	2.04	53	11:26:16 RWP	AC	Excel	None	1437
D	17560	4	12829	5.18	4.80	4.70	4.49	4.26	3.73	3.22	2.73	53	11:26:23 RWP	AC	Excel	None	1408
D	18021	2	6481	2.54	2.38	2.34	2.24	2.03	1.79	1.48	1.26	53	11:27:46 RWP	AC	Excel	None	1452
D	18021	3	9659	3.87	3.63	3.63	3.50	3.40	3.09	2.66	2.48	53	11:27:52 RWP	AC	Excel	None	1421
D	18021	4	12861	5.22	4.88	4.91	4.72	4.57	4.19	3.57	3.05	53	11:27:58 RWP	AC	Excel	None	1400
D	18505	2	6501	2.89	2.73	2.54	2.42	2.22	1.94	1.68	1.46	53	11:29:43 RWP	AC	Excel	None	1279
D	18505	3	9691	4.33	4.09	3.84	3.60	3.41	2.92	2.55	2.22	53	11:29:48 RWP	AC	Excel	None	1272
D	18505	4	12892	5.78	5.42	5.16	4.80	4.48	3.91	3.42	2.98	53	11:29:55 RWP	AC	Excel	None	1268
D	19030	2	6422	3.25	3.07	2.95	2.71	2.63	2.28	2.01	1.72	53	11:31:25 RWP	AC	Excel	None	1124
D	19030	3	9582	4.87	4.58	4.37	4.15	3.93	3.45	3.02	2.57	53	11:31:30 RWP	AC	Excel	None	1119
D	19030	4	12742	6.53	6.13	5.91	5.55	5.26	4.69	4.09	3.49	53	11:31:37 RWP	AC	Excel	None	1109
D	19539	2	6446	2.70	2.53	2.51	2.38	2.36	2.11	1.84	1.59	53	11:33:08 RWP	AC	Excel	None	1355
D	19539	3	9627	4.16	3.85	3.83	3.74	3.57	3.25	2.82	2.42	53	11:33:13 RWP	AC	Excel	None	1317
D	19539	4	12799	5.57	5.18	5.18	5.00	4.80	4.41	3.80	3.26	53	11:33:20 RWP	AC	Excel	None	1306
D	20040	2	6482	1.94	1.79	1.80	1.73	1.59	1.47	1.25	1.08	53	11:34:38 RWP	AC	Excel	None	1900
D	20040	3	9696	2.89	2.68	2.69	2.62	2.56	2.41	2.18	1.93	53	11:34:43 RWP	AC	Excel	None	1904
D	20040	4	12888	3.83	3.53	3.57	3.47	3.38	3.21	2.89	2.58	53	11:34:50 RWP	AC	Excel	None	1915
D	20041	2	6489	1.92	1.78	1.74	1.67	1.66	1.53	1.42	1.31	53	11:35:38 RWP	AC	Excel	None	1926
D	20041	3	9707	2.85	2.67	2.64	2.57	2.50	2.31	2.17	1.96	53	11:35:44 RWP	AC	Excel	None	1939
D	20041	4	12904	3.76	3.49	3.51	3.41	3.29	3.08	2.89	2.61	53	11:35:50 RWP	AC	Excel	None	1950
ft Time: 11:36:00 :LOAD TRANSFER																	
D	20529	2	6495	1.76	1.67	1.64	1.57	1.36	1.47	1.37	1.25	53	11:37:25 RWP	AC	Excel	None	2100
D	20529	3	9715	2.61	2.48	2.43	2.40	2.33	2.18	2.03	1.88	53	11:37:30 RWP	AC	Excel	None	2116
D	20529	4	12915	3.45	3.26	3.23	3.16	3.09	2.91	2.71	2.53	53	11:37:37 RWP	AC	Excel	None	2131
D	21042	2	6454	2.20	2.05	2.13	2.02	2.00	1.81	1.60	1.38	53	11:38:57 RWP	AC	Excel	None	1671
D	21042	3	9633	3.41	3.09	3.25	3.15	3.04	2.77	2.40	2.11	53	11:39:02 RWP	AC	Excel	None	1609
D	21042	4	12811	4.55	4.12	4.33	4.19	4.02	3.69	3.20	2.84	53	11:39:09 RWP	AC	Excel	None	1599
D	21522	2	6444	1.30	1.26	1.18	1.10	1.08	0.97	0.89	0.80	53	11:40:52 RWP	AC	Excel	None	2821
D	21522	3	9673	1.99	1.89	1.77	1.69	1.63	1.47	1.35	1.23	53	11:40:57 RWP	AC	Excel	None	2760
D	21522	4	12881	2.66	2.47	2.47	2.37	2.24	1.98	1.80	1.62	53	11:40:44 RWP	AC	Excel	None	2758
D	22007	2	6484	1.24	1.17	1.14	1.11	1.09	1.04	0.98	0.92	53	11:42:02 RWP	AC	Excel	None	2979
D	22007	3	9675	1.87	1.73	1.72	1.69	1.64	1.56	1.46	1.35	53	11:42:07 RWP	AC	Excel	None	2946

Sibley Hwy9 EB NOV13			AC		Exce].		None										
22007	4	12887	2-48	2-25	2-28	2-21	2-17	2-09	1-95	1-79	53	52	11:42:14	RMP	AC	Exce].	2960
22536	2	6480	1-62	1-43	1-49	1-38	1-35	1-19	1-08	0-98	53	52	11:43:44	RMP	AC	Exce].	2273
22536	3	9692	2-44	2-17	2-25	2-15	2-03	1-83	1-63	1-46	53	52	11:43:49	RMP	AC	Exce].	2263
22536	4	12880	3-20	2-80	2-99	2-83	2-68	2-42	2-17	1-95	53	52	11:43:55	RMP	AC	Exce].	2289
23033	2	6500	1-72	1-70	1-54	1-47	1-41	1-30	1-20	1-10	53	52	11:45:21	RMP	AC	Exce].	2151
23033	3	9697	2-35	2-50	2-29	2-22	2-10	1-60	1-77	1-62	53	52	11:45:26	RMP	AC	Exce].	2162
23033	4	12926	3-35	3-28	3-05	2-93	2-82	2-60	2-37	2-16	53	52	11:45:32	RMP	AC	Exce].	2191
23575	2	6457	1-51	1-42	1-45	1-37	1-33	1-12	0-96	0-83	53	52	11:46:47	RMP	AC	Exce].	2404
23575	3	9696	2-39	2-13	2-17	2-09	2-00	1-73	1-47	1-24	53	52	11:46:52	RMP	AC	Exce].	2304
23575	4	12888	3-15	2-83	2-89	2-79	2-66	2-31	1-97	1-64	53	52	11:46:59	RMP	AC	Exce].	2325
24074	2	6469	2-28	2-14	2-06	1-91	1-81	1-56	1-36	1-14	53	52	11:48:13	RMP	AC	Exce].	1611
24074	3	9637	3-48	3-25	3-14	2-98	2-76	2-40	2-04	1-73	53	52	11:48:18	RMP	AC	Exce].	1575
24074	4	12814	4-08	4-34	4-22	3-97	3-72	3-24	2-76	2-33	53	52	11:48:24	RMP	AC	Exce].	1557
24526	2	6464	2-31	2-07	2-09	1-97	1-88	1-64	1-37	1-14	53	52	11:49:36	RMP	AC	Exce].	1588
24526	3	9606	3-51	3-12	3-18	3-04	2-84	2-47	2-06	1-72	53	52	11:49:41	RMP	AC	Exce].	1558
24526	4	12835	4-71	4-18	4-32	4-07	3-85	3-34	2-79	2-32	53	52	11:49:48	RMP	AC	Exce].	1550
25767	2	6468	1-63	1-50	1-48	1-38	1-34	1-18	1-00	0-87	53	52	11:52:44	RMP	AC	Exce].	2255
25767	3	9696	2-60	2-29	2-29	2-15	2-03	1-80	1-53	1-32	53	52	11:52:49	RMP	AC	Exce].	2122
25767	4	12905	3-33	3-09	3-12	2-92	2-75	2-45	2-05	1-75	53	52	11:52:56	RMP	AC	Exce].	2080
C Comment at 25767 FT Time: 11:53:05 :JUST PAST BRIDGE																	
26041	2	6464	1-59	1-50	1-46	1-34	1-29	1-14	0-97	0-87	53	52	11:54:25	RMP	AC	Exce].	2309
26041	3	9679	2-51	2-29	2-21	2-11	1-97	1-75	1-50	1-31	53	52	11:54:31	RMP	AC	Exce].	2194
26041	4	12877	3-39	3-07	2-99	2-82	2-67	2-38	2-05	1-77	53	52	11:54:37	RMP	AC	Exce].	2160
26520	2	6486	1-66	1-51	1-47	1-38	1-32	1-19	1-03	0-92	53	52	11:56:14	RMP	AC	Exce].	2227
26520	3	9683	2-33	2-30	2-25	2-16	2-05	1-82	1-62	1-41	53	52	11:56:20	RMP	AC	Exce].	2178
26520	4	12883	3-40	3-07	3-02	2-91	2-75	2-47	2-18	1-91	53	52	11:56:26	RMP	AC	Exce].	2155
27020	2	6469	2-22	2-00	1-86	1-69	1-57	1-38	1-19	1-02	53	52	11:57:46	RMP	AC	Exce].	1659
27020	3	9661	3-38	3-04	2-86	2-64	2-42	2-10	1-80	1-56	53	52	11:57:51	RMP	AC	Exce].	1623
27020	4	12850	4-37	4-09	3-85	3-55	3-30	2-84	2-45	2-12	53	52	11:57:58	RMP	AC	Exce].	1597
27579	2	6462	1-92	1-76	1-73	1-62	1-60	1-45	1-25	1-10	53	52	11:59:23	RMP	AC	Exce].	1917
27579	3	9650	2-81	2-64	2-61	2-46	2-41	2-20	1-90	1-69	53	52	11:59:28	RMP	AC	Exce].	1888
27579	4	12889	3-51	3-55	3-53	3-35	3-23	2-92	2-57	2-26	53	52	11:59:34	RMP	AC	Exce].	1875
28004	2	6433	2-33	2-05	2-09	1-87	1-79	1-56	1-33	1-12	53	52	12:00:56	RMP	AC	Exce].	1569
28004	3	9630	3-56	3-14	3-17	2-96	2-73	2-40	2-04	1-74	53	52	12:01:07	RMP	AC	Exce].	1537
28004	4	12837	4-81	4-22	4-29	4-00	3-74	3-30	2-78	2-36	53	52	12:01:07	RMP	AC	Exce].	1517
28558	2	6447	2-15	2-05	1-91	1-83	1-74	1-58	1-39	1-24	53	52	12:02:31	RMP	AC	Exce].	1702
28558	3	9628	3-24	3-08	2-95	2-79	2-64	2-37	2-10	1-87	53	52	12:02:36	RMP	AC	Exce].	1692
28558	4	12824	4-33	4-07	3-92	3-71	3-52	3-17	2-84	2-49	53	52	12:02:43	RMP	AC	Exce].	1683
29028	2	6426	2-47	2-46	2-13	1-97	1-83	1-56	1-35	1-13	53	52	12:04:05	RMP	AC	Exce].	1482
29028	3	9645	3-78	3-74	3-24	3-03	2-79	2-42	2-05	1-75	53	52	12:04:10	RMP	AC	Exce].	1452
29028	4	12847	4-43	4-27	4-23	4-03	3-74	3-30	2-78	2-36	53	52	12:04:57	RMP	AC	Exce].	1506
29028	3	9661	3-76	3-73	3-24	3-03	2-82	2-40	1-37	1-77	53	52	12:05:02	RMP	AC	Exce].	1461
29028	2	6439	2-37	2-11	2-15	2-06	1-91	1-67	1-40	1-21	53	52	12:05:09	RMP	AC	Exce].	1441
29534	3	9667	3-66	3-25	3-31	3-12	2-92	2-59	2-13	1-83	53	52	12:06:34	RMP	AC	Exce].	1545
29534	4	12882	4-37	4-40	4-50	4-27	3-99	3-54	2-95	2-49	53	52	12:06:39	RMP	AC	Exce].	1501
30038	2	6451	2-66	2-46	2-42	2-28	2-19	1-95	1-70	1-46	53	52	12:08:45	RMP	AC	Exce].	1473
30038	3	9644	4-07	3-73	3-72	3-55	3-35	2-97	2-59	2-23	53	52	12:08:17	RMP	AC	Exce].	1379
30038	4	12804	5-42	4-96	4-99	4-72	4-47	3-97	3-46	2-97	53	52	12:08:22	RMP	AC	Exce].	1348
30039	2	6434	2-60	2-43	2-36	2-28	2-12	1-88	1-69	1-48	54	52	12:08:29	RMP	AC	Exce].	1343
30039	3	9636	3-96	3-69	3-61	3-46	3-24	2-89	2-55	2-25	54	52	12:09:12	RMP	AC	Exce].	1409
30039	4	12797	5-30	4-90	4-82	4-61	4-36	3-87	3-42	3-02	54	52	12:09:17	RMP	AC	Exce].	1384
C Comment at 30038 FT Time: 12:09:33 :LOAD TRANSFER																	
30552	2	6438	2-01	1-86	1-76	1-65	1-57	1-44	1-27	1-13	53	52	12:10:39	RMP	AC	Exce].	1824
30552	3	9647	3-01	2-80	2-67	2-56	2-40	2-15	1-92	1-69	53	52	12:10:44	RMP	AC	Exce].	1824
30552	4	12808	4-02	3-71	3-59	3-39	3-24	2-89	2-59	2-28	53	52	12:10:51	RMP	AC	Exce].	1810
31019	2	6437	2-18	2-03	2-00	1-85	1-83	1-64	1-50	1-32	53	52	12:12:32	RMP	AC	Exce].	1679
31019	3	9615	3-26	3-04	3-00	2-88	2-74	2-51	2-27	2-03	53	52	12:12:37	RMP	AC	Exce].	1675
31019	4	12819	4-33	4-02	3-98	3-83	3-68	3-34	3-04	2-74	53	52	12:12:44	RMP	AC	Exce].	1684
31563	2	6450	2-50	2-23	2-21	2-07	1-99	1-80	1-55	1-31	53	52	12:14:13	RMP	AC	Exce].	1469
31563	3	9643	3-81	3-37	3-39	3-04	2-74	2-36	2-06	1-72	53	52	12:14:18	RMP	AC	Exce].	1439
31563	4	12844	5-11	4-54	4-58	4-34	4-10	3-69	3-17	2-71	53	52	12:14:24	RMP	AC	Exce].	1430
32023	2	6419	2-62	2-39	2-30	2-12	2-00	1-69	1-44	1-22	53	52	12:15:37	RMP	AC	Exce].	1394

D	41025	4	12849	4.30	4.06	3.87	3.71	3.50	3.12	2.73	2.43	53	Sibley HWY9 EB NOV13	AC	Exce].	None	1698
D	41515	2	6418	1.61	1.46	1.44	1.32	1.31	1.19	1.11	0.98	53	52 12:44:24 RWP	AC	Exce].	None	2262
D	41515	3	9631	2.42	2.21	2.14	2.08	1.99	1.82	1.66	1.51	53	52 12:45:41 RWP	AC	Exce].	None	2260
D	41515	4	12854	3.22	2.93	2.87	2.74	2.65	2.46	2.23	2.03	53	52 12:45:46 RWP	AC	Exce].	None	2267
D	42026	2	6413	2.02	1.86	1.79	1.62	1.55	1.29	1.10	0.93	53	52 12:45:52 RWP	AC	Exce].	None	1807
D	42026	3	9613	3.09	2.81	2.76	2.59	2.39	1.99	1.66	1.41	53	52 12:47:09 RWP	AC	Exce].	None	1768
D	42026	4	12824	4.16	3.77	3.72	3.50	3.22	2.70	2.26	1.90	53	52 12:47:14 RWP	AC	Exce].	None	1751
D	42551	2	6398	2.29	2.10	2.03	1.86	1.78	1.49	1.29	1.11	53	52 12:47:21 RWP	AC	Exce].	None	1587
D	42551	3	9581	3.54	3.23	3.12	2.96	2.72	2.33	2.00	1.70	53	53 12:48:46 RWP	AC	Exce].	None	1541
D	43020	2	6443	4.77	4.36	4.26	3.97	3.71	3.18	2.70	2.29	53	53 12:48:57 RWP	AC	Exce].	None	1525
D	43020	3	9648	3.38	3.15	3.11	2.99	2.81	2.49	2.15	1.85	53	53 12:50:10 RWP	AC	Exce].	None	1647
D	43020	4	12856	4.59	4.26	4.23	4.03	3.84	3.41	2.96	2.56	53	53 12:50:15 RWP	AC	Exce].	None	1623
D	43540	2	6437	2.35	2.18	2.07	1.89	1.75	1.52	1.33	1.13	54	53 12:50:22 RWP	AC	Exce].	None	1592
D	43540	3	9627	3.60	3.30	3.13	2.92	2.69	2.33	2.02	1.76	54	52 12:51:55 RWP	AC	Exce].	None	1556
D	43540	4	12818	4.85	4.42	4.24	3.89	3.64	3.12	2.74	2.38	54	52 12:52:00 RWP	AC	Exce].	None	1520
D	44012	2	6411	2.04	1.87	1.77	1.64	1.55	1.34	1.17	0.99	54	52 12:52:06 RWP	AC	Exce].	None	1504
D	44012	3	9614	3.06	2.82	2.68	2.55	2.37	2.05	1.76	1.50	54	52 12:53:30 RWP	AC	Exce].	None	1791
D	44012	4	12806	4.08	3.75	3.61	3.37	3.17	2.76	2.37	2.04	54	52 12:53:36 RWP	AC	Exce].	None	1786
D	44625	2	6410	3.14	2.98	2.93	2.78	2.64	2.31	2.01	1.65	53	52 12:53:42 RWP	AC	Exce].	None	1785
D	44625	3	9591	4.68	4.42	4.35	4.20	3.96	3.48	2.99	2.50	53	52 12:55:33 RWP	AC	Exce].	None	1159
D	44625	4	12800	6.17	5.82	5.77	5.54	5.27	4.64	3.99	3.34	53	52 12:55:39 RWP	AC	Exce].	None	1166
C	Comment at 44625 ft																1179
C	Comment at 44799 ft																
C									LARGE PANEL								
C									END OF SECTION								
C									END OF SECTION								
C																	

Iowa Highway 9 Westbound, Sibley, IA, November 2009

Sibley HWY9 WB NOV13

SIBLEY HWY9 WB NOV13.fwd

IKUAB FWD FILE
 HProject No. : Cable Overlay
 HLocation : HWY 9 WB
 HClient : Cable
 HStart Station :
 HDirection :
 HEnd Station :
 HWeather : CLOUDY DRIZZLE
 HOperator : hg

IDate Created : 11/13/2009
 IVersion : 2.3.11
 ILoad Mode : 1 (SHRP 8+8 buffers, 0 plates)
 IPlate Radius : 5.91 (in)
 IExtra Field Set : Example Road
 IDrop Sequence : 2123
 INO Of drops : 1111
 IRecord Drop? :
 IDrop Height :
 IImpact Load :
 ISensor Number :
 ISensor Distance : 0.00 12.00 12.00 18.00 24.00 36.00 48.00 60.00 0.00 (in)
 ISensor Position : CENTER FRONT BEHIND BEHIND BEHIND BEHIND BEHIND BEHIND BEHIND

ID	Distance	Imp	Load	D0	D1	D2	D3	D4	D5	D6	D7	Air	Pave	Time	Location	Pavement	Pavement	Surface	
J	ft	Num	lbf	mils	°F	F	°F		Type	Condition	Modulus								
D	93	2	6429	2.94	2.77	2.75	2.61	2.51	2.17	1.88	1.57	53	53	13:00:18	RWP	AC	Excel.	None	1242
D	93	3	9637	4.37	4.15	4.12	3.97	3.75	3.28	2.85	2.37	53	53	13:00:24	RWP	AC	Excel.	None	1254
D	93	4	12839	5.80	5.47	5.47	5.24	4.97	4.40	3.79	3.18	53	53	13:00:31	RWP	AC	Excel.	None	1260
C	Comment at 93 ft Time: 13:00:41 : LARGE PANEL																		
D	595	2	6422	2.03	1.84	1.84	1.72	1.66	1.45	1.25	1.10	52	52	13:02:17	RWP	AC	Excel.	None	1797
D	595	3	9651	3.09	2.81	2.83	2.69	2.53	2.24	1.92	1.67	52	52	13:02:22	RWP	AC	Excel.	None	1779
D	595	4	12844	4.16	3.78	3.83	3.63	3.42	3.02	2.60	2.24	52	52	13:02:28	RWP	AC	Excel.	None	1755
D	597	2	6425	1.99	1.84	1.81	1.69	1.64	1.42	1.24	1.08	52	52	13:03:12	RWP	AC	Excel.	None	1832
D	597	3	9623	3.03	2.78	2.75	2.65	2.51	2.17	1.90	1.64	52	52	13:03:17	RWP	AC	Excel.	None	1807
D	597	4	12839	4.08	3.74	3.73	3.56	3.37	2.96	2.58	2.20	52	52	13:03:23	RWP	AC	Excel.	None	1790
C	Comment at 595 ft Time: 13:03:33 : LOAD TRANSFER																		
D	1102	2	6413	1.68	1.58	1.51	1.44	1.41	1.29	1.18	1.07	52	52	13:04:42	RWP	AC	Excel.	None	2174
D	1102	3	9613	2.51	2.35	2.26	2.22	2.12	1.96	1.79	1.63	52	52	13:04:47	RWP	AC	Excel.	None	2176
D	1102	4	12823	3.33	3.11	3.03	2.93	2.81	2.61	2.39	2.19	52	52	13:04:53	RWP	AC	Excel.	None	2188
D	1581	2	6422	1.75	1.61	1.59	1.53	1.51	1.38	1.27	1.14	52	52	13:06:12	RWP	AC	Excel.	None	2088
D	1581	3	9621	2.61	2.37	2.39	2.34	2.26	2.14	1.92	1.72	52	52	13:06:18	RWP	AC	Excel.	None	2095
D	1581	4	12816	3.45	3.14	3.18	3.10	3.03	2.85	2.58	2.30	52	52	13:06:24	RWP	AC	Excel.	None	2113
D	2094	2	6400	1.86	1.78	1.68	1.58	1.55	1.41	1.30	1.18	52	52	13:07:51	RWP	AC	Excel.	None	1958
D	2094	3	9603	2.78	2.64	2.51	2.45	2.35	2.14	1.96	1.76	52	52	13:07:56	RWP	AC	Excel.	None	1961
D	2094	4	12806	3.67	3.49	3.35	3.24	3.12	2.86	2.60	2.37	52	52	13:08:02	RWP	AC	Excel.	None	1986
D	2605	2	6386	2.45	2.32	2.17	2.04	1.97	1.74	1.53	1.36	52	52	13:09:19	RWP	AC	Excel.	None	1480
D	2605	3	9598	3.73	3.32	3.34	3.17	2.99	2.66	2.37	2.09	52	52	13:09:24	RWP	AC	Excel.	None	1465
D	2605	4	12790	4.98	4.69	4.49	4.23	4.04	3.58	3.19	2.83	52	52	13:09:31	RWP	AC	Excel.	None	1461
D	3122	2	6421	2.43	2.24	2.05	1.90	1.81	1.60	1.38	1.22	52	52	13:10:51	RWP	AC	Excel.	None	1503
D	3122	3	9609	3.65	3.37	3.13	2.94	2.73	2.42	2.11	1.84	52	52	13:10:57	RWP	AC	Excel.	None	1499
D	3122	4	12820	4.87	4.51	4.22	3.95	3.70	3.27	2.87	2.50	52	52	13:11:03	RWP	AC	Excel.	None	1496
D	3602	2	6417	2.00	1.85	1.79	1.65	1.56	1.37	1.22	1.05	52	52	13:11:37	RWP	AC	Excel.	None	1822
D	3602	3	9642	3.03	2.81	2.70	2.55	2.39	2.10	1.85	1.63	52	52	13:11:42	RWP	AC	Excel.	None	1810
D	3602	4	12830	4.06	3.76	3.65	3.44	3.22	2.85	2.49	2.21	52	52	13:11:48	RWP	AC	Excel.	None	1795
D	4092	2	6395	2.53	2.40	2.28	2.13	2.02	1.76	1.52	1.30	52	52	13:14:06	RWP	AC	Excel.	None	1435
D	4092	3	9592	3.89	3.67	3.47	3.31	3.13	2.70	2.32	1.98	52	52	13:14:12	RWP	AC	Excel.	None	1401
D	4092	4	12778	5.23	4.94	4.70	4.48	4.21	3.68	3.15	2.69	52	52	13:14:18	RWP	AC	Excel.	None	1389
D	4599	2	6379	2.38	2.23	2.12	1.98	1.84	1.57	1.32	1.11	52	52	13:15:46	RWP	AC	Excel.	None	1523

D	-52108	2	6114	117	102	95	88	82	69	57	47	57	57	09:30:43	RWP	IA65	NE	Cable	None	756
D	-52108	3	9252	178	157	147	138	127	107	89	72	57	57	09:30:49	RWP	AC		Poor	None	749
D	-52108	4	12426	240	210	198	185	170	145	120	99	57	57	09:30:55	RWP	AC		Poor	None	749

TA65 SB Cable

-10492	4	12415	240	212	202	193	183	159	137	114	57	57	10:01:35	RMP	Name	746
-11518	2	6110	146	118	103	94	81	72	60	47	57	57	10:03:42	RMP	Name	505
-11518	3	9241	224	184	164	149	130	115	97	77	57	57	10:03:47	RMP	Name	507
-11518	4	12420	301	251	224	205	179	159	134	108	57	57	10:03:53	RMP	Name	535
-12609	3	6095	140	124	119	113	110	95	79	66	57	57	10:05:30	RMP	Name	627
-12609	3	9221	216	193	186	179	171	150	127	106	57	57	10:05:36	RMP	Name	617
-12609	4	12374	291	261	252	242	232	213	173	145	57	57	10:05:44	RMP	Name	614
-13544	2	6076	117	109	104	97	93	80	67	58	57	57	10:07:13	RMP	Name	751
-13544	3	9220	182	170	163	155	146	126	106	83	57	57	10:07:21	RMP	Name	731
-13544	4	12369	249	231	222	210	199	173	146	117	57	57	10:07:30	RMP	Name	718
-14529	2	6059	84	75	72	67	65	56	47	38	57	57	10:09:10	RMP	Name	1047
-14529	3	9195	133	121	115	111	105	91	76	63	57	57	10:09:15	RMP	Name	996
-14529	4	12343	186	169	162	156	148	139	108	90	57	57	10:09:22	RMP	Name	960
-15367	2	6037	121	114	119	110	106	92	78	65	57	57	10:11:15	RMP	Name	722
-15367	3	9181	188	188	184	173	163	142	121	103	57	57	10:11:20	RMP	Name	707
-15367	4	12335	255	234	250	234	221	193	165	140	57	57	10:11:27	RMP	Name	699
-16488	2	6084	102	95	91	86	81	70	57	44	57	57	10:13:17	RMP	Name	864
-16488	3	9204	160	149	143	136	128	111	90	71	57	57	10:13:23	RMP	Name	829
-16488	4	12370	219	204	196	187	176	152	125	98	57	57	10:13:31	RMP	Name	814
-20005	2	5973	176	152	136	121	115	100	86	71	58	58	10:16:46	RMP	Name	490
-20005	3	9073	269	235	213	195	181	159	136	114	58	58	10:16:52	RMP	Name	486
-20005	4	12177	362	318	290	265	248	218	188	158	58	58	10:16:58	RMP	Name	486
Comment at -17505				Time: 10:20:24			Note dmi error to 17505									
-18536	2	6040	115	109	106	102	101	92	81	68	58	58	10:22:06	RMP	Name	738
-18536	3	9137	177	169	165	162	157	144	126	107	58	58	10:22:12	RMP	Name	744
-18536	4	12312	241	229	224	219	212	196	172	145	58	58	10:22:20	RMP	Name	737
Comment at -18536				Time: 10:22:30			Dmi error should be 17505									
-19526	2	6038	130	118	110	102	98	86	73	60	58	58	10:23:53	RMP	Name	673
-19526	3	9140	203	185	173	165	154	135	115	96	58	58	10:23:59	RMP	Name	650
-19526	4	12253	279	255	240	226	213	186	159	132	58	58	10:24:07	RMP	Name	633
-20595	2	5992	202	198	195	190	188	161	139	119	58	58	10:25:52	RMP	Name	428
-20595	3	9084	309	303	298	297	288	248	214	185	58	58	10:25:58	RMP	Name	424
-20595	4	12201	410	403	396	391	381	329	286	248	58	58	10:26:06	RMP	Name	429
-21518	2	6053	100	97	94	93	91	83	73	64	58	58	10:27:51	RMP	Name	823
-21518	3	9180	157	151	148	148	143	130	115	100	58	58	10:27:56	RMP	Name	844
-21518	4	12386	213	205	201	200	195	177	157	138	58	58	10:28:03	RMP	Name	840
-22530	2	6055	98	96	93	90	82	66	55	46	58	58	10:29:41	RMP	Name	894
-22530	3	9158	153	150	146	140	128	104	86	72	58	58	10:29:48	RMP	Name	865
-22530	4	12316	208	203	198	188	172	140	118	98	58	58	10:29:55	RMP	Name	856
-23519	2	6088	71	65	62	59	56	49	41	34	58	58	10:31:28	RMP	Name	1236
-23519	3	9226	113	102	98	94	88	77	65	54	58	58	10:31:33	RMP	Name	1174
-23519	4	12426	154	139	134	128	121	106	90	75	58	58	10:31:40	RMP	Name	1164
-24504	2	6101	75	69	67	63	60	51	42	34	58	58	10:33:36	RMP	Name	1159
-24504	3	9243	118	109	105	101	95	81	68	56	58	58	10:33:42	RMP	Name	1129
-24504	4	12463	161	149	145	138	130	112	95	78	58	58	10:33:48	RMP	Name	1116
-25373	2	6093	103	94	90	85	81	71	61	51	58	58	10:35:36	RMP	Name	856
-25373	3	9280	163	150	143	139	130	114	98	82	58	58	10:35:42	RMP	Name	824
-25373	4	12493	224	206	199	191	180	158	137	115	58	58	10:35:49	RMP	Name	806
-26542	2	6068	105	97	93	89	86	76	65	55	58	58	10:37:20	RMP	Name	836
-26542	3	9164	162	151	145	141	133	118	102	86	58	58	10:37:26	RMP	Name	815
-26542	4	12350	220	205	198	191	181	161	140	118	58	58	10:37:34	RMP	Name	810
-27562	2	6087	84	75	70	64	62	52	43	36	58	58	10:39:17	RMP	Name	1051
-27562	3	9232	131	117	110	104	97	83	70	58	58	58	10:39:22	RMP	Name	1019
-27562	4	12416	182	161	153	142	134	115	97	81	58	58	10:39:29	RMP	Name	987
-28503	2	6029	198	172	161	146	132	115	98	81	58	58	10:41:40	RMP	Name	411
-28503	3	9113	302	266	249	233	205	179	154	127	58	58	10:41:46	RMP	Name	436
-28503	4	12201	406	358	336	315	279	244	210	175	58	58	10:41:53	RMP	Name	434
-29571	2	6111	127	114	108	104	97	86	74	62	58	58	10:44:38	RMP	Name	695
-29571	3	9229	199	179	169	165	153	137	118	100	59	58	10:44:44	RMP	Name	670
-29571	4	12405	272	244	231	225	210	188	164	138	59	58	10:44:51	RMP	Name	659
-30529	2	6094	81	77	75	71	69	63	56	50	59	59	10:46:48	RMP	Name	1091
-30529	3	9213	124	118	116	110	105	93	87	78	59	59	10:46:54	RMP	Name	1071
-30529	4	12423	167	159	157	151	145	133	119	106	59	59	10:47:01	RMP	Name	1072
-31530	2	6063	139	133	128	122	118	104	89	75	59	59	10:49:06	RMP	Name	628
-31530	3	9159	217	207	201	193	185	163	141	119	59	59	10:49:11	RMP	Name	609
-31530	4	12316	295	279	272	261	250	222	191	163	59	59	10:49:18	RMP	Name	603

TA65 SB Cable

D	-32522	2	6020	262	223	134	103	89	74	61	49	59	60	10:50:50	RMP	None	331
D	-32522	3	9126	390	332	209	164	141	118	98	79	59	60	10:50:56	RMP	None	338
D	-32522	4	12229	515	438	285	225	195	164	136	110	59	60	10:51:04	RMP	None	343
D	-33572	2	6025	127	112	107	103	99	87	75	63	59	60	10:52:51	RMP	None	687
D	-33572	3	9138	200	178	171	167	158	139	119	102	59	60	10:52:56	RMP	None	691
D	-33572	4	12253	273	244	235	228	217	192	166	142	59	60	10:53:03	RMP	None	698
D	-34507	3	6043	158	127	114	103	96	81	67	55	59	60	10:54:35	RMP	None	551
D	-34507	4	12318	243	200	181	165	152	128	107	87	59	60	10:54:41	RMP	None	544
D	-34507	5	9180	330	274	249	226	208	177	150	121	59	60	10:54:48	RMP	None	539
D	-35539	2	6052	147	109	91	74	65	51	42	33	59	60	10:56:28	RMP	None	595
D	-35539	3	9163	220	165	140	117	102	80	65	52	59	60	10:56:35	RMP	None	603
D	-35539	4	12554	293	221	189	158	138	110	89	72	59	60	10:56:43	RMP	None	609
D	-36578	2	6033	169	149	138	128	123	111	96	81	59	61	10:58:29	RMP	None	514
D	-36578	3	9144	263	211	204	193	173	150	128	108	59	61	10:58:35	RMP	None	503
D	-36578	4	12307	357	313	293	278	262	235	206	176	59	61	10:58:42	RMP	None	499
D	-37517	3	6048	141	123	115	107	102	88	74	60	59	60	11:00:25	RMP	None	620
D	-37517	4	9170	219	192	180	171	161	140	117	96	59	60	11:00:31	RMP	None	604
D	-37517	5	12323	296	261	246	232	220	191	161	133	59	60	11:00:38	RMP	None	601
D	-38521	2	6042	137	125	112	102	90	81	69	59	59	60	11:02:23	RMP	None	638
D	-38521	3	9180	208	190	169	164	156	139	125	107	59	60	11:02:29	RMP	None	637
D	-38521	4	12306	280	235	230	221	211	189	172	147	59	60	11:02:37	RMP	None	634
D	-39647	2	6016	116	109	104	98	96	86	75	64	59	61	11:04:31	RMP	None	751
D	-39647	3	9147	180	169	162	156	149	135	116	100	59	61	11:04:37	RMP	None	733
D	-39647	4	12270	245	219	211	201	183	159	136	98	59	61	11:04:45	RMP	None	722
D	-40495	2	6018	180	158	142	135	130	119	104	88	59	60	11:06:25	RMP	None	483
D	-40495	3	9112	277	243	222	214	202	186	163	138	59	60	11:06:31	RMP	None	474
D	-40495	4	12202	372	327	300	288	274	251	221	186	59	60	11:06:38	RMP	None	474
D	-41565	2	6004	175	166	160	154	150	136	121	107	59	60	11:08:29	RMP	None	495
D	-41565	3	9073	270	233	244	239	230	208	186	165	59	60	11:08:35	RMP	None	485
D	-41565	4	12241	361	338	327	320	309	279	251	225	59	60	11:08:43	RMP	None	490
D	-42508	2	6036	161	150	138	130	122	107	94	79	59	60	11:10:08	RMP	None	542
D	-42508	3	9165	249	231	214	204	191	168	147	124	59	60	11:10:14	RMP	None	531
D	-42508	4	12310	337	310	290	276	258	228	202	171	59	60	11:10:22	RMP	None	528
D	-43491	2	5986	151	138	130	126	123	110	95	83	59	60	11:11:49	RMP	None	572
D	-43491	3	9038	232	210	199	194	188	169	147	127	59	60	11:11:56	RMP	None	564
D	-43491	4	12204	314	283	270	263	253	229	200	172	59	61	11:12:04	RMP	None	562
D	-44545	2	6054	95	85	81	76	74	64	54	46	59	61	11:13:50	RMP	None	920
D	-44545	3	9191	148	132	127	121	115	100	86	72	59	61	11:13:55	RMP	None	895
D	-44545	4	12372	203	180	173	165	156	137	117	99	59	61	11:14:02	RMP	None	882
D	-45649	2	6048	151	139	126	113	100	82	69	55	59	61	11:15:42	RMP	None	578
D	-45649	3	9171	233	214	196	178	156	130	110	87	59	61	11:15:48	RMP	None	568
D	-45649	4	12338	317	289	266	241	212	178	150	121	59	61	11:15:56	RMP	None	562
D	-46559	2	6005	177	154	141	126	119	104	88	73	59	61	11:17:23	RMP	None	480
D	-46559	3	9075	269	234	215	198	185	160	137	115	59	61	11:17:28	RMP	None	488
D	-46559	4	12264	361	314	290	267	249	216	187	157	59	61	11:17:36	RMP	None	491
D	-47514	2	6010	156	140	134	124	118	106	95	81	59	60	11:18:24	RMP	None	535
D	-47514	3	9146	245	234	226	219	213	197	177	154	60	62	11:18:29	RMP	None	539
D	-47514	4	12284	333	314	304	295	285	267	240	209	60	62	11:18:36	RMP	None	533
D	-48650	2	6097	92	88	85	81	78	69	59	51	60	62	11:21:13	RMP	None	962
D	-48650	3	9253	142	136	133	128	122	107	94	79	60	62	11:21:20	RMP	None	939
D	-48650	4	12446	194	185	181	173	166	147	128	109	60	62	11:21:28	RMP	None	924
D	-49594	2	6034	87	81	77	73	70	63	52	43	60	62	11:23:07	RMP	None	938
D	-49594	3	9199	137	128	122	117	111	100	83	69	60	62	11:23:13	RMP	None	972
D	-49594	4	12424	188	175	168	160	152	138	116	96	60	62	11:23:21	RMP	None	955
D	-50569	2	5995	199	199	192	184	172	158	141	124	60	62	11:24:46	RMP	None	434
D	-50569	3	9121	298	294	292	282	272	258	245	230	60	62	11:24:52	RMP	None	442
D	-50569	4	12293	395	385	370	359	348	328	306	278	60	62	11:24:59	RMP	None	450
D	-51489	2	6033	227	157	135	117	108	89	71	56	60	63	11:26:14	RMP	None	384
D	-51489	3	9135	341	242	211	186	169	140	113	90	60	63	11:26:20	RMP	None	387
D	-51489	4	12279	453	323	287	252	230	191	155	123	60	63	11:26:27	RMP	None	391
D	-52528	2	6118	95	87	84	79	76	66	55	45	60	64	11:27:59	RMP	None	926
D	-52528	3	9229	148	134	129	124	118	102	86	71	60	64	11:28:04	RMP	None	900
D	-52528	4	12451	203	184	177	169	161	140	118	97	60	64	11:28:11	RMP	None	886
C	Comment at -52986	Time: 11:29:40	Deflection is not decreasing														
C	-52986	2	5943	290	257	193	182	193	161	129	93	61	64	11:29:48	RMP	None	296
C	Comment at -52986	Time: 11:29:53	Deflection is not decreasing														

Northwood 65 Cable NB

D	3104	4	12972	5.58	5.16	5.68	5.76	5.79	5.32	4.56	3.97	28	28	07:32:12	CTR	None	1322
D	3608	2	6497	3.31	2.84	2.72	2.45	2.23	1.84	1.54	1.33	28	28	07:33:53	CTR	None	1117
D	3608	3	9758	5.03	4.30	4.17	3.77	3.42	2.83	2.40	2.03	28	28	07:33:58	CTR	None	1103
D	3608	4	12942	6.68	5.69	5.57	5.01	4.57	3.80	3.22	2.72	28	28	07:34:06	CTR	None	1101
Comment: Testing in this file was continued again on 11/17/2009 at 7:36:19 AM																	
D	5000	2	6519	2.76	2.38	2.45	2.20	2.05	1.44	1.24	1.06	28	29	07:37:15	CTR	None	1343
D	5000	3	9785	4.18	3.57	3.71	3.42	3.12	2.14	1.85	1.58	28	29	07:37:21	CTR	None	1332
D	5000	4	13007	5.50	4.66	4.88	4.47	4.11	2.78	2.39	2.07	28	29	07:37:28	CTR	None	1345
Comment at 4999 ft Time: 07:37:41 :DVI ERROR, LOCATION ESTIMATE																	
D	5508	2	6453	1.85	1.70	1.85	1.80	1.76	1.59	1.42	1.30	28	28	07:39:15	CTR	None	1987
Comment at 5508 ft Time: 07:39:21 :Deflection is not decreasing																	
D	5508	3	9757	2.78	2.55	2.80	2.77	2.65	2.40	2.16	1.95	28	28	07:39:25	CTR	None	1994
Comment at 5508 ft Time: 07:39:32 :Deflection is not decreasing																	
D	5508	4	12950	3.69	3.34	3.72	3.65	3.52	3.19	2.87	2.59	28	28	07:39:35	CTR	None	1996
D	6483	2	6524	1.87	1.76	1.70	1.64	1.60	1.49	1.34	1.21	28	28	07:42:58	CTR	None	1983
D	6483	3	9804	2.84	2.65	2.59	2.52	2.43	2.25	2.04	1.85	28	28	07:43:04	CTR	None	1965
D	6483	2	6533	1.87	1.75	1.69	1.63	1.60	1.48	1.32	1.21	28	28	07:43:26	CTR	None	1989
D	6483	3	9755	2.82	2.63	2.58	2.50	2.41	2.24	2.03	1.83	28	28	07:43:31	CTR	None	1984
D	6483	4	13026	3.74	3.49	3.45	3.33	3.22	3.01	2.71	2.45	28	28	07:43:59	CTR	None	1979
D	7029	2	6467	4.25	4.03	3.65	3.31	3.06	2.21	1.88	1.88	28	28	07:45:31	CTR	None	865
D	7029	3	9711	6.51	6.16	5.60	5.13	4.72	4.00	3.41	2.88	28	28	07:45:37	CTR	None	848
D	7029	4	12898	8.66	8.20	7.47	6.84	6.28	5.35	4.57	3.84	28	28	07:45:44	CTR	None	847
D	7030	2	6442	4.59	4.23	3.87	3.46	3.22	2.68	2.25	1.94	28	29	07:47:18	CTR	None	798
D	7030	3	9658	7.07	6.49	5.97	5.45	4.97	4.15	3.54	2.99	28	29	07:47:24	CTR	None	778
D	7030	4	12888	9.38	8.65	7.95	7.24	6.64	5.36	4.73	3.98	28	29	07:47:52	CTR	None	760
Comment at 7030 ft Time: 07:47:57 :LOAD TRANSFER																	
D	7505	2	6466	2.00	1.90	1.88	1.82	1.84	1.70	1.52	1.39	28	28	07:49:35	CTR	None	1838
D	7505	3	9761	3.02	2.86	2.89	2.82	2.77	2.58	2.36	2.11	28	28	07:49:41	CTR	None	1836
D	7505	4	12985	4.01	3.78	3.84	3.74	3.67	3.44	3.13	2.80	28	28	07:49:48	CTR	None	1841
D	8031	2	6460	3.11	2.73	2.81	2.55	2.42	2.06	1.80	1.58	28	28	07:51:44	CTR	None	1181
D	8031	3	9738	4.76	4.17	4.33	4.03	3.69	3.37	2.76	2.41	28	28	07:51:49	CTR	None	1163
D	8031	4	12948	6.54	5.52	5.80	5.36	4.94	4.23	3.68	3.21	28	28	07:51:57	CTR	None	1162
D	8518	2	6482	1.85	1.76	1.74	1.68	1.70	1.55	1.40	1.29	29	29	07:53:35	CTR	None	1989
D	8518	3	9742	2.79	2.64	2.64	2.62	2.54	2.35	2.15	1.96	29	29	07:53:42	CTR	None	1986
D	8518	4	12970	3.67	3.47	3.51	3.42	3.37	3.13	2.85	2.59	29	29	07:53:50	CTR	None	2010
Comment: Testing in this file was continued again on 11/17/2009 at 7:59:02 AM																	
D	9008	2	6510	2.27	2.16	2.17	2.15	2.12	1.91	1.61	1.40	30	31	07:59:40	CTR	None	1631
D	9008	3	9788	3.43	3.22	3.35	3.31	3.22	2.90	2.47	2.13	30	31	07:59:46	CTR	None	1621
D	9008	4	12994	4.56	4.23	4.46	4.38	4.28	3.86	3.28	2.83	30	31	07:59:54	CTR	None	1620
Comment at 9524 ft Time: 08:01:47 :Deflection is not decreasing																	
D	9524	2	6485	2.77	2.41	2.80	2.74	2.70	2.19	1.76	1.43	29	29	08:01:52	CTR	None	1331
Comment at 9524 ft Time: 08:01:58 :Deflection is not decreasing																	
D	9524	3	9700	4.21	3.63	4.29	4.21	4.14	3.35	2.70	2.19	29	29	08:02:06	CTR	None	1309
Comment at 9524 ft Time: 08:02:13 :Deflection is not decreasing																	
D	9524	4	12906	5.65	4.86	5.78	5.62	5.53	4.49	3.61	2.93	29	29	08:02:15	CTR	None	1299
Comment: Testing in this file was continued again on 11/17/2009 at 8:06:08 AM																	
D	10012	2	6475	2.50	2.43	2.20	2.13	2.08	1.89	1.69	1.52	31	31	08:06:35	RWP	None	1473
D	10012	3	9749	3.74	3.64	3.41	3.29	3.15	2.88	2.62	2.33	31	31	08:06:40	RWP	None	1484
D	10012	4	12981	4.93	4.78	4.51	4.34	4.17	3.83	3.48	3.11	31	31	08:06:48	RWP	None	1499
D	10524	2	6515	2.21	2.09	2.05	1.98	1.98	1.83	1.66	1.54	30	30	08:08:34	RWP	None	1675
D	10524	3	9737	3.54	3.15	3.13	3.08	3.00	2.80	2.56	2.34	30	30	08:08:40	RWP	None	1660
D	10524	4	12933	4.39	4.14	4.16	4.07	3.98	3.72	3.41	3.11	30	30	08:08:47	RWP	None	1676
D	11013	2	6512	1.17	1.03	1.06	1.02	1.02	0.93	0.81	0.72	30	29	08:10:24	RWP	None	3169
D	11013	3	9776	1.76	1.55	1.61	1.62	1.56	1.41	1.26	1.10	30	29	08:10:31	RWP	None	3158
D	11013	4	12981	2.36	2.09	2.20	2.14	2.08	1.91	1.68	1.49	30	29	08:10:40	RWP	None	3127
D	11517	2	6484	1.72	1.61	1.61	1.60	1.58	1.45	1.28	1.12	30	30	08:12:16	RWP	None	2139
D	11517	3	9738	2.61	2.42	2.49	2.46	2.40	2.20	1.95	1.71	30	30	08:12:22	RWP	None	2120
D	11517	4	12932	3.45	3.18	3.30	3.27	3.22	2.94	2.59	2.28	30	30	08:12:29	RWP	None	2132
D	12007	2	6505	3.80	3.35	3.00	2.69	2.43	2.04	1.72	1.50	30	30	08:14:00	RWP	None	974
D	12007	3	9695	5.72	5.04	4.58	4.11	3.73	3.11	2.66	2.28	30	30	08:14:05	RWP	None	965
D	12506	2	6463	2.90	2.50	2.57	2.37	2.22	1.84	1.58	1.39	30	31	08:15:46	RWP	None	1268
D	12506	3	9708	4.41	3.76	3.95	3.69	3.39	2.81	2.40	2.08	30	31	08:15:52	RWP	None	1252

		Northwood 65 Cable NB															
D	12506	4	12902	5.86	4.99	5.28	4.89	4.54	3.78	3.19	2.74	30	31	08:15:59 RWP	AC	Excel.	1252
D	12971	2	6504	1.90	1.82	1.73	1.66	1.64	1.52	1.39	1.28	31	08:17:36 RWP	AC	Excel.	1945	
D	12971	3	9776	2.88	2.75	2.65	2.58	2.51	2.33	2.12	1.94	30	08:17:42 RWP	AC	Excel.	1933	
D	12971	4	12976	3.83	3.65	3.54	3.45	3.35	3.12	2.84	2.58	30	08:17:49 RWP	AC	Excel.	1927	
D	13526	2	6502	2.96	2.13	2.14	2.02	1.96	1.76	1.58	1.46	30	08:19:35 RWP	PCC	Excel.	1569	
D	13526	3	9736	3.56	3.20	3.22	3.12	2.96	2.67	2.41	2.18	30	08:19:40 RWP	PCC	Excel.	1556	
D	13526	4	12948	4.68	4.20	4.28	4.09	3.91	3.52	3.19	2.88	30	08:19:47 RWP	PCC	Excel.	1573	
D	14015	2	6506	2.79	2.54	2.77	2.68	2.62	2.23	1.97	1.88	31	08:21:24 RWP	PCC	Excel.	1328	
D	14015	3	9738	4.17	3.76	4.11	3.99	3.79	3.35	2.95	2.65	31	08:21:30 RWP	PCC	Excel.	1329	
C	Comment	at	14015	ft	Time: 08:21:37: Deflection is not decreasing												
D	14015	4	12940	5.51	4.96	5.46	5.27	5.01	4.44	3.94	3.48	31	08:21:40 RWP	PCC	Excel.	1336	
D	14547	2	6532	2.26	2.06	2.22	2.22	2.19	1.91	1.64	1.40	31	08:23:27 RWP	PCC	Excel.	1641	
C	Comment	at	14547	ft	Time: 08:23:32: Deflection is not decreasing												
C	Comment	at	9764	3.37	3.07	3.36	3.39	3.27	2.88	2.50	2.12	31	08:23:34 RWP	PCC	Excel.	1649	
C	Comment	at	14547	ft	Time: 08:23:42: Deflection is not decreasing												
D	15043	2	6500	4.45	4.04	4.48	4.48	4.36	3.82	3.31	2.80	31	08:23:43 RWP	PCC	Excel.	1652	
D	15043	3	9689	2.80	2.37	2.40	2.18	2.07	1.78	1.51	1.29	31	08:25:27 RWP	PCC	Excel.	1318	
D	15043	4	12919	4.27	3.59	3.68	3.39	3.16	2.70	2.32	1.99	31	08:25:32 RWP	PCC	Excel.	1291	
D	15043	4	12919	5.72	4.81	4.95	4.52	4.25	3.65	3.13	2.67	31	08:25:39 RWP	PCC	Excel.	1284	
D	15508	2	6538	1.89	1.85	1.75	1.70	1.65	1.50	1.34	1.26	31	08:27:12 RWP	PCC	Excel.	1966	
D	15508	3	9719	2.84	2.78	2.65	2.52	2.47	2.28	2.07	1.89	31	08:27:17 RWP	PCC	Excel.	1944	
D	15508	4	12936	3.80	3.70	3.55	3.44	3.33	3.05	2.80	2.50	31	08:27:25 RWP	PCC	Excel.	1938	
D	16070	2	6596	1.41	1.33	1.27	1.25	1.20	1.11	1.04	0.94	31	08:29:20 RWP	PCC	Excel.	2669	
D	16070	3	9745	2.11	1.98	1.92	1.86	1.80	1.66	1.57	1.41	31	08:29:25 RWP	PCC	Excel.	2625	
D	16070	4	13004	2.80	2.61	2.58	2.50	2.41	2.23	2.05	1.90	31	08:29:32 RWP	PCC	Excel.	2641	
D	16072	2	6541	1.43	1.34	1.27	1.22	1.21	1.12	1.03	0.94	32	08:30:16 RWP	PCC	Excel.	2604	
D	16072	3	9743	2.16	2.01	1.95	1.90	1.82	1.73	1.56	1.42	32	08:30:21 RWP	PCC	Excel.	2569	
D	16072	4	13004	2.89	2.67	2.62	2.51	2.44	2.27	2.07	1.90	32	08:30:28 RWP	PCC	Excel.	2561	
C	Comment	at	16072	ft	Time: 08:30:39: Load transfer												
D	16490	2	6521	1.43	1.32	1.33	1.23	1.25	1.17	1.07	1.02	32	08:31:42 RWP	PCC	Excel.	2600	
D	16490	3	9721	2.11	1.96	1.94	1.88	1.82	1.77	1.63	1.52	32	08:31:47 RWP	PCC	Excel.	2619	
D	16490	4	12946	2.76	2.58	2.58	2.52	2.45	2.34	2.13	1.96	32	08:31:54 RWP	PCC	Excel.	2664	
D	17021	2	6514	2.26	1.98	2.13	2.01	1.89	1.59	1.38	1.18	32	08:33:46 RWP	PCC	Excel.	1637	
D	17021	3	9663	3.39	2.96	3.20	3.06	2.87	2.78	2.63	2.34	32	08:33:52 RWP	PCC	Excel.	1622	
D	17021	4	12917	4.48	3.89	4.21	4.05	3.80	3.16	2.73	2.34	32	08:33:59 RWP	PCC	Excel.	1641	
D	17523	2	6540	1.65	1.53	1.53	1.48	1.48	1.40	1.28	1.15	32	08:35:38 RWP	PCC	Excel.	2257	
D	17523	3	9741	2.31	2.31	2.33	2.32	2.27	2.13	1.95	1.74	32	08:35:43 RWP	PCC	Excel.	2206	
D	17523	4	12931	3.32	3.04	3.12	3.05	3.01	2.85	2.62	2.50	32	08:35:51 RWP	PCC	Excel.	2217	
D	18021	2	6541	1.67	1.48	1.43	1.35	1.32	1.18	1.08	0.99	32	08:37:26 RWP	PCC	Excel.	2233	
D	18021	3	9723	2.54	2.25	2.21	2.10	2.01	1.81	1.63	1.48	32	08:37:32 RWP	PCC	Excel.	2179	
D	18021	4	12976	3.43	3.03	3.00	2.86	2.72	2.45	2.20	1.98	32	08:37:39 RWP	PCC	Excel.	2150	
D	18513	2	6489	1.75	1.72	1.49	1.40	1.36	1.19	1.05	0.94	32	08:39:10 RWP	PCC	Excel.	2114	
D	18513	3	9719	2.64	2.58	2.28	2.05	1.83	1.62	1.42	1.24	32	08:39:15 RWP	PCC	Excel.	2090	
D	18513	4	12937	3.53	3.44	3.07	2.89	2.75	2.45	2.14	1.90	32	08:39:22 RWP	PCC	Excel.	2082	
D	19031	2	6554	1.44	1.31	1.27	1.23	1.19	1.10	1.01	0.94	32	08:40:55 CTR	PCC	Excel.	2573	
D	19031	3	9727	2.17	1.97	1.96	1.88	1.82	1.67	1.54	1.41	32	08:41:00 CTR	PCC	Excel.	2546	
D	19031	4	12941	2.91	2.64	2.63	2.51	2.45	2.26	2.05	1.91	32	08:41:07 CTR	PCC	Excel.	2527	
D	19672	2	6573	1.15	1.05	1.01	0.97	0.94	0.89	0.84	0.78	32	08:42:58 CTR	PCC	Excel.	3240	
D	19672	3	9800	1.71	1.55	1.53	1.48	1.44	1.35	1.27	1.16	32	08:43:03 CTR	PCC	Excel.	3262	
D	19672	4	13031	2.28	2.04	2.04	1.96	1.91	1.81	1.67	1.52	32	08:43:10 CTR	PCC	Excel.	3255	
D	20014	2	6500	1.49	1.37	1.36	1.31	1.30	1.17	1.05	0.99	34	08:44:56 CTR	PCC	Excel.	2475	
D	20014	3	9738	2.24	2.03	2.06	2.01	1.94	1.78	1.62	1.46	34	08:45:02 CTR	PCC	Excel.	2479	
D	20014	4	12960	2.97	2.67	2.73	2.64	2.59	2.39	2.15	1.95	34	08:45:09 CTR	PCC	Excel.	2485	
D	20531	2	6544	2.03	1.87	1.84	1.77	1.73	1.59	1.45	1.32	35	08:46:55 CTR	PCC	Excel.	1836	
D	20531	3	9748	3.02	2.80	2.78	2.69	2.59	2.38	2.17	2.00	35	08:47:00 CTR	PCC	Excel.	1834	
D	20531	4	12971	4.04	3.71	3.72	3.58	3.44	3.17	2.89	2.65	35	08:47:07 CTR	PCC	Excel.	1827	
D	20946	2	6559	2.58	2.34	2.44	2.35	2.34	2.00	1.75	1.57	34	08:48:40 CTR	PCC	Excel.	1447	
D	20946	3	9748	3.85	3.47	3.64	3.55	3.50	3.00	2.64	2.34	34	08:48:45 CTR	PCC	Excel.	1438	
D	20946	4	12975	5.10	4.60	4.84	4.72	4.64	4.01	3.53	3.12	34	08:48:52 CTR	PCC	Excel.	1448	
D	21518	2	6543	1.91	1.80	1.79	1.77	1.79	1.69	1.56	1.42	35	08:50:42 CTR	PCC	Excel.	1948	
D	21518	3	9760	2.85	2.67	2.72	2.70	2.68	2.54	2.35	2.15	35	08:50:47 CTR	PCC	Excel.	1948	
D	21518	4	12955	3.75	3.52	3.63	3.59	3.56	3.41	3.17	2.89	35	08:50:54 CTR	PCC	Excel.	1962	
D	22078	2	6544	1.74	1.62	1.58	1.51	1.48	1.34	1.20	1.10	35	08:53:08 CTR	PCC	Excel.	2141	

		Northwood 65 Cable NB																	
D	22078	3	9753	2.63	2.43	2.39	2.32	2.23	2.04	1.84	1.67	35	34	08:53:13	CTR	PCC	Excel.	None	2105
D	22078	4	12969	3.52	3.23	3.22	3.11	3.00	2.75	2.49	2.23	35	34	08:53:20	CTR	PCC	Excel.	None	2095
D	22483	2	6512	4.14	3.74	3.61	3.26	2.98	2.41	1.97	1.69	35	34	08:55:17	CTR	PCC	Excel.	None	895
D	22483	3	9694	6.24	5.63	5.45	4.98	4.52	3.65	3.02	2.52	35	34	08:55:23	CTR	PCC	Excel.	None	883
D	22483	4	12896	8.51	7.46	7.29	6.63	6.04	4.89	4.05	3.58	35	34	08:55:30	CTR	PCC	Excel.	None	883
D	22993	2	6537	1.88	1.79	1.87	1.84	1.88	1.71	1.52	1.37	35	34	08:57:14	CTR	PCC	Excel.	None	1935
D	22993	3	9730	2.88	2.64	2.81	2.80	2.81	2.59	2.30	2.04	35	34	08:57:20	CTR	PCC	Excel.	None	1924
C	Comment	at	22993	ft	Time: 08:57:27	Deflection is not decreasing													
D	22993	4	12952	3.77	3.45	3.71	3.69	3.70	3.45	3.08	2.70	35	34	08:57:29	CTR	PCC	Excel.	None	1951
D	23522	2	6545	2.69	2.76	2.45	2.34	2.25	2.06	1.85	1.64	35	34	08:59:19	CTR	PCC	Excel.	None	1383
D	23522	3	9737	4.00	4.06	3.65	3.50	3.36	3.05	2.74	2.44	35	34	08:59:24	CTR	PCC	Excel.	None	1385
D	23522	4	12944	5.27	5.35	4.80	4.62	4.43	4.03	3.65	3.23	35	34	08:59:32	CTR	PCC	Excel.	None	1397
C	Comment	at	24020	ft	Time: 09:03:00	Deflection is not decreasing													
D	24020	2	6570	3.12	2.81	3.32	3.41	2.94	1.81	1.59	1.45	36	35	09:03:04	CTR	PCC	Excel.	None	1197
C	Comment	at	24020	ft	Time: 09:03:10	Deflection is not decreasing													
D	24020	3	9758	4.69	4.20	5.02	5.22	4.38	2.67	2.38	2.13	36	35	09:03:16	CTR	PCC	Excel.	None	1182
C	Comment	at	24020	ft	Time: 09:03:24	Deflection is not decreasing													
D	24020	4	12946	6.26	5.58	6.68	6.92	5.80	3.50	3.11	2.79	36	35	09:03:26	CTR	PCC	Excel.	None	1177
D	24514	2	6518	2.71	2.54	2.60	2.50	2.39	2.09	1.85	1.65	37	35	09:05:24	CTR	PCC	Excel.	None	1366
D	24514	3	9752	4.08	3.82	3.94	3.84	3.63	3.17	2.81	2.47	37	35	09:05:30	CTR	PCC	Excel.	None	1358
D	24514	4	12959	5.40	5.05	5.27	5.06	4.82	4.22	3.73	3.30	37	35	09:05:38	CTR	PCC	Excel.	None	1363
D	24520	2	6558	2.94	2.64	2.58	2.41	2.30	2.00	1.71	1.46	36	36	09:08:58	CTR	PCC	Excel.	None	1269
D	24520	3	9742	4.41	3.95	3.90	3.68	3.46	3.02	2.61	2.19	36	36	09:09:03	CTR	PCC	Excel.	None	1256
D	24520	4	12952	5.85	5.22	5.16	4.86	4.60	3.98	3.47	2.90	36	36	09:09:10	CTR	PCC	Excel.	None	1259
C	Comment	at	24519	ft	Time: 09:09:21	LOAD TRANSFER													
D	24981	2	6555	1.57	1.46	1.45	1.42	1.37	1.29	1.22	1.14	36	36	09:11:11	CTR	PCC	Excel.	None	2374
D	24981	3	9751	2.32	2.17	2.16	2.10	2.06	1.95	1.85	1.70	36	36	09:11:17	CTR	PCC	Excel.	None	2387
D	24981	4	12964	3.04	2.82	2.85	2.78	2.71	2.58	2.42	2.25	36	36	09:11:24	CTR	PCC	Excel.	None	2421
D	25502	2	6570	1.79	1.71	1.58	1.50	1.43	1.26	1.14	1.01	37	35	09:13:16	CTR	PCC	Excel.	None	2087
D	25502	3	9753	2.70	2.60	2.38	2.31	2.17	1.94	1.72	1.52	37	35	09:13:22	CTR	PCC	Excel.	None	2050
D	25502	4	12985	3.61	3.48	3.21	3.05	2.92	2.61	2.35	2.05	37	35	09:13:29	CTR	PCC	Excel.	None	2044
D	26016	2	6588	2.38	2.15	1.95	1.77	1.64	1.37	1.16	1.00	36	36	09:15:04	CTR	PCC	Excel.	None	1570
D	26016	3	9747	3.63	3.26	3.00	2.72	2.49	2.10	1.79	1.53	36	36	09:15:09	CTR	PCC	Excel.	None	1526
D	26016	4	12970	4.86	4.36	4.03	3.65	3.37	2.86	2.42	2.03	36	36	09:15:16	CTR	PCC	Excel.	None	1517
D	26516	2	6550	1.68	1.54	1.59	1.53	1.54	1.44	1.24	1.11	38	37	09:16:58	CTR	PCC	Excel.	None	2213
D	26516	3	9784	2.52	2.29	2.38	2.35	2.32	2.19	1.92	1.67	38	37	09:17:03	CTR	PCC	Excel.	None	2204
D	26516	4	13006	3.35	3.05	3.19	3.14	3.15	2.95	2.37	2.25	38	37	09:17:10	CTR	PCC	Excel.	None	2210
D	27011	2	6536	2.65	2.44	2.37	2.22	2.17	1.92	1.69	1.54	37	37	09:18:45	CTR	PCC	Excel.	None	1402
D	27011	3	9737	4.01	3.67	3.57	3.44	3.28	2.92	2.60	2.33	37	37	09:18:50	CTR	PCC	Excel.	None	1382
D	27011	4	12948	5.32	4.86	4.76	4.53	4.37	3.88	3.47	3.08	37	37	09:18:57	CTR	PCC	Excel.	None	1384
C	Comment	at	28192	ft	Time: 09:21:48	Deflection is not decreasing													
D	28192	2	6593	1.79	1.59	1.80	1.81	1.61	1.37	1.15	1.00	36	37	09:21:50	CTR	PCC	Excel.	None	2092
C	Comment	at	28192	ft	Time: 09:21:56	Deflection is not decreasing													
D	28192	3	9818	2.71	2.37	2.75	2.78	2.44	2.08	1.78	1.51	36	37	09:21:58	CTR	PCC	Excel.	None	2058
C	Comment	at	28192	ft	Time: 09:22:44	Deflection is not decreasing													
D	28192	2	6574	1.78	1.57	1.79	1.79	1.60	1.35	1.14	1.01	37	38	09:22:46	CTR	PCC	Excel.	None	2096
C	Comment	at	28192	ft	Time: 09:22:52	Deflection is not decreasing													
D	28192	3	9806	2.71	2.37	2.73	2.79	2.42	2.07	1.77	1.51	37	38	09:22:52	CTR	PCC	Excel.	None	2060
C	Comment	at	28192	ft	Time: 09:22:59	Deflection is not decreasing													
D	28192	4	13031	3.61	3.17	3.69	3.73	3.26	2.79	2.38	2.04	37	38	09:23:01	CTR	PCC	Excel.	None	2051
D	28518	2	6583	1.59	1.28	1.29	1.26	1.25	1.11	1.00	0.96	38	37	09:24:12	CTR	PCC	Excel.	None	2689
D	28518	3	9792	2.10	1.92	1.95	1.93	1.86	1.69	1.53	1.39	38	37	09:24:18	CTR	PCC	Excel.	None	2650
D	28518	4	13022	2.77	2.50	2.59	2.52	2.46	2.23	2.03	1.85	38	37	09:24:25	CTR	PCC	Excel.	None	2668
D	29529	2	6519	1.60	1.52	1.47	1.42	1.39	1.30	1.22	1.09	38	36	09:26:43	CTR	PCC	Excel.	None	2312
D	29529	3	9730	2.41	2.25	2.23	2.18	2.09	1.94	1.62	1.38	38	36	09:26:48	CTR	PCC	Excel.	None	2295
D	29529	4	12972	3.21	2.99	2.98	2.87	2.79	2.60	2.40	2.16	38	36	09:26:55	CTR	PCC	Excel.	None	2299
D	29532	2	6512	1.66	1.61	1.50	1.42	1.37	1.27	1.15	1.04	38	37	09:27:48	CTR	PCC	Excel.	None	2237
D	29532	3	9727	2.51	2.43	2.26	2.19	2.10	1.90	1.74	1.57	38	37	09:27:54	CTR	PCC	Excel.	None	2208
D	29532	4	12946	3.34	3.21	3.02	2.91	2.79	2.53	2.31	2.07	38	37	09:28:01	CTR	PCC	Excel.	None	2207
C	Comment	at	29532	ft	Time: 09:28:11	LOAD TRANSFER													
D	30010	2	6473	1.51	1.46	1.40	1.32	1.29	1.18	1.09	1.02	38	36	09:29:29	CTR	PCC	Excel.	None	2432
D	30010	3	9687	2.27	2.15	2.08	2.01	1.94	1.78	1.64	1.51	38	36	09:29:34	CTR	PCC	Excel.	None	2425
D	30010	4	12909	2.99	2.86	2.77	2.67	2.58	2.38	2.19	2.00	38	36	09:29:41	CTR	PCC	Excel.	None	2453

Northwood 65 Cable NB														
D	30517	2	6439	2.43	2.26	2.06	1.90	1.81	1.60	1.41	1.28	1.06	None	1509
D	30517	3	9638	3.69	3.41	3.16	2.94	2.76	2.45	2.16	1.93	1.06	None	1486
D	30517	4	12846	4.91	4.55	4.24	3.93	3.71	3.28	2.90	2.57	1.06	None	1486
D	31022	2	6445	1.78	1.71	1.52	1.45	1.40	1.20	1.12	1.03	1.03	None	2063
D	31022	3	9655	2.68	2.56	2.33	2.22	2.12	1.90	1.70	1.54	1.03	None	2063
D	31022	4	12860	3.53	3.40	3.12	2.94	2.82	2.27	2.04	1.88	1.03	None	2070
D	31518	2	6466	2.13	1.83	1.76	1.58	1.47	1.24	1.04	0.88	0.88	None	1726
D	31518	3	9666	3.25	2.77	2.68	2.43	2.23	1.89	1.60	1.34	0.88	None	1694
D	31518	4	12849	4.34	3.68	3.60	3.25	3.01	2.53	2.14	1.82	0.88	None	1685
D	32009	2	6478	1.60	1.53	1.40	1.33	1.26	1.14	1.00	0.90	0.90	None	2304
D	32009	3	9657	2.45	2.36	2.15	2.07	1.96	1.73	1.57	1.38	0.90	None	2250
D	32009	4	12927	3.30	3.19	2.93	2.79	2.65	2.38	2.10	1.87	0.90	None	2226
D	32523	2	6497	1.49	1.42	1.32	1.26	1.21	1.07	0.99	0.89	0.89	None	2473
D	32523	3	9673	2.22	2.12	1.99	1.95	1.84	1.67	1.50	1.34	0.89	None	2473
D	32523	4	12907	2.95	2.81	2.66	2.56	2.46	2.00	1.73	1.50	0.89	None	2473
D	33019	2	6447	1.22	1.12	1.07	1.04	0.98	0.90	0.83	0.76	0.76	None	2490
D	33019	3	9641	1.83	1.69	1.63	1.55	1.51	1.38	1.25	1.13	0.76	None	3011
D	33019	4	12885	2.42	2.24	2.17	2.07	2.00	1.82	1.65	1.51	0.76	None	2995
D	33514	2	6502	1.81	1.75	1.53	1.42	1.34	1.19	1.02	0.93	0.93	None	3022
D	33514	3	9679	2.71	2.62	2.31	2.17	2.04	1.77	1.55	1.36	0.93	None	2039
D	33514	4	12913	3.62	3.49	3.10	2.89	2.73	2.37	2.07	1.82	0.93	None	2028
D	34016	2	6458	1.58	1.49	1.43	1.36	1.33	1.21	1.10	0.99	0.99	None	2030
D	34016	3	9679	2.38	2.23	2.18	2.11	2.01	1.82	1.64	1.49	0.99	None	2322
D	34016	4	12899	3.17	2.98	2.92	2.80	2.70	2.46	2.22	1.98	0.99	None	2314
D	34523	2	6496	1.88	1.78	1.81	1.77	1.77	1.69	1.54	1.41	1.41	None	2311
D	34523	3	9656	2.82	2.64	2.71	2.69	2.66	2.57	2.38	2.11	1.41	None	1960
D	34523	4	12902	3.76	3.50	3.63	3.61	3.57	3.45	3.21	2.84	1.41	None	1949
C	Comment	at	35059	ft	Time: 09:49:21	Deflection is not decreasing								1952
D	35059	2	6459	2.40	2.08	2.44	2.48	1.84	1.54	1.31	1.12	1.12	None	1530
C	Comment	at	35059	ft	Time: 09:49:29	Deflection is not decreasing								1488
D	35059	3	9670	3.69	3.19	3.76	3.85	2.81	2.35	2.00	1.71	1.71	None	1488
C	Comment	at	35059	ft	Time: 09:49:38	Deflection is not decreasing								1465
D	35059	4	12876	5.00	4.34	5.12	5.17	3.80	3.21	2.71	2.31	2.31	None	1465
D	35502	2	6480	1.77	1.64	1.59	1.52	1.47	1.37	1.25	1.14	1.14	None	2082
D	35502	3	9658	2.71	2.54	2.42	2.35	2.28	2.08	1.89	1.76	1.14	None	2082
D	35502	4	12894	3.55	3.29	3.22	3.12	2.99	2.76	2.53	2.30	1.14	None	2068
D	35504	2	6507	1.04	1.85	1.73	1.64	1.58	1.43	1.26	1.15	1.15	None	1912
D	35504	3	9648	2.88	2.77	2.56	2.45	2.35	2.14	1.94	1.72	1.15	None	1903
D	35504	4	12925	3.83	3.69	3.42	3.29	3.15	2.84	2.58	2.31	1.15	None	1918
C	Comment	at	35504	ft	Time: 09:53:44	LOAD TRANSFER								2242
D	36020	2	6459	1.64	1.55	1.50	1.45	1.41	1.28	1.14	1.06	1.06	None	2242
D	36020	3	9637	2.44	2.33	2.26	2.19	2.11	1.94	1.76	1.62	1.06	None	2244
D	36020	4	12894	3.24	3.12	2.98	2.89	2.80	2.56	2.33	2.13	1.06	None	2264
D	36516	2	6507	1.41	1.26	1.30	1.20	1.17	1.05	0.92	0.86	0.86	None	2616
D	36516	3	9700	2.14	1.90	1.97	1.87	1.76	1.59	1.44	1.31	0.86	None	2581
D	36516	4	12930	2.85	2.53	2.65	2.50	2.36	2.15	1.93	1.73	0.86	None	2581
C	Comment	at	37628	ft	Time: 10:01:04	END								2582

Northwood 65_CABLE_SB

D	16634	4	12824	3.82	3.57	3.71	3.70	3.59	3.27	2.98	2.70	43	44	11:04:05	CTR	PCC	Excel.	Nore	1910	
D	17010	2	6421	2.13	2.19	1.91	1.80	1.77	1.58	1.42	1.27	44	44	11:05:33	CTR	PCC	Excel.	Nore	1715	
D	17010	3	9619	3.19	3.27	2.89	2.80	2.65	2.39	2.16	1.89	43	44	11:05:38	CTR	PCC	Excel.	Nore	1716	
D	17010	4	12852	4.23	4.35	3.85	3.67	3.53	3.19	2.87	2.52	43	44	11:05:45	CTR	PCC	Excel.	Nore	1726	
D	17506	2	6420	1.85	1.84	1.72	1.63	1.57	1.44	1.32	1.20	44	45	11:07:17	CTR	PCC	Excel.	Nore	1974	
D	17506	3	9594	2.75	2.75	2.52	2.47	2.36	2.17	1.97	1.78	44	45	11:07:22	CTR	PCC	Excel.	Nore	1981	
D	17506	4	12847	3.64	3.63	3.34	3.26	3.13	2.88	2.62	2.38	44	45	11:07:29	CTR	PCC	Excel.	Nore	2007	
D	18011	2	6427	1.75	1.68	1.65	1.59	1.60	1.47	1.34	1.20	44	44	11:09:03	CTR	PCC	Excel.	Nore	2083	
D	18011	3	9577	2.61	2.52	2.30	2.24	2.39	2.23	2.03	1.82	44	44	11:09:08	CTR	PCC	Excel.	Nore	2083	
D	18011	4	12825	3.47	3.32	3.20	3.14	3.18	2.96	2.70	2.43	44	44	11:09:15	CTR	PCC	Excel.	Nore	2104	
D	18515	2	6400	1.75	1.57	1.52	1.46	1.38	1.26	1.06	0.93	44	45	11:10:54	CTR	PCC	Excel.	Nore	2084	
D	18515	3	9530	2.68	2.38	2.33	2.24	2.12	1.87	1.67	1.46	44	45	11:10:59	CTR	PCC	Excel.	Nore	2027	
D	18513	4	12805	3.62	3.20	3.16	3.00	2.85	2.54	2.23	1.98	44	45	11:11:06	CTR	PCC	Excel.	Nore	2011	
D	18515	2	6396	1.54	1.38	1.45	1.45	1.34	1.22	1.09	0.95	44	44	11:11:50	CTR	PCC	Excel.	Nore	2356	
D	18515	3	9560	2.37	2.14	2.25	2.20	2.09	1.87	1.66	1.47	44	44	11:11:55	CTR	PCC	Excel.	Nore	2297	
D	18515	4	12850	3.19	2.86	3.05	2.96	2.82	2.51	2.24	2.00	44	44	11:12:02	CTR	PCC	Excel.	Nore	2287	
C	Comment: Testing in this file was continued again on 11/17/2009 at 11:15:27 AM																			
D	19124	2	6441	1.64	1.52	1.55	1.50	1.49	1.43	1.38	1.25	44	42	11:16:11	RWP	AC	Excel.	Nore	2233	
D	19124	3	9637	2.48	2.31	2.37	2.35	2.29	2.19	2.08	1.90	44	42	11:16:16	RWP	AC	Excel.	Nore	2207	
D	19124	4	12809	3.35	3.08	3.17	3.12	3.07	2.97	2.82	2.57	44	42	11:16:23	RWP	AC	Excel.	Nore	2192	
C	Comment at 19648 Ft Time: 11:18:03 :Deflection is not decreasing																			
D	19648	2	6428	2.31	2.07	2.38	2.42	2.28	1.93	1.63	1.39	45	43	11:18:10	RWP	AC	Excel.	Nore	1583	
C	Comment at 19648 Ft Time: 11:18:16 :Deflection is not decreasing																			
D	19648	3	9583	3.46	3.09	3.57	3.61	3.44	2.94	2.51	2.08	45	43	11:18:18	RWP	AC	Excel.	Nore	1577	
C	Comment at 19648 Ft Time: 11:18:26 :Deflection is not decreasing																			
D	19648	4	12845	4.63	4.13	4.81	4.87	4.64	3.97	3.38	2.79	45	43	11:18:28	RWP	AC	Excel.	Nore	1577	
D	20099	2	6432	2.55	2.34	2.50	2.45	2.32	1.94	1.65	1.42	45	44	11:19:59	RWP	AC	Excel.	Nore	1435	
C	Comment at 20099 Ft Time: 11:20:05 :Deflection is not decreasing																			
D	20099	3	9586	3.84	3.51	3.78	3.76	3.48	2.98	2.56	2.15	45	44	11:20:07	RWP	AC	Excel.	Nore	1421	
C	Comment at 20099 Ft Time: 11:20:14 :Deflection is not decreasing																			
D	20099	4	12854	5.09	4.62	5.07	4.99	4.67	4.01	3.44	2.90	45	44	11:20:16	RWP	AC	Excel.	Nore	1435	
D	20615	2	6421	2.46	2.25	2.47	2.40	2.32	2.05	1.80	1.59	45	43	11:23:22	RWP	AC	Excel.	Nore	1483	
C	Comment at 20615 Ft Time: 11:23:27 :Deflection is not decreasing																			
D	20615	3	9608	3.69	3.38	3.73	3.72	3.51	3.13	2.77	2.42	45	43	11:23:29	RWP	AC	Excel.	Nore	1479	
C	Comment at 20615 Ft Time: 11:23:36 :Deflection is not decreasing																			
D	20615	4	12857	4.91	4.48	4.97	4.92	4.69	4.18	3.70	3.23	45	43	11:23:37	RWP	AC	Excel.	Nore	1489	
D	21129	2	6409	2.37	2.47	2.15	2.00	1.94	1.73	1.57	1.39	45	44	11:25:15	RWP	AC	Excel.	Nore	1536	
D	21129	3	9557	3.57	3.69	3.21	3.09	2.94	2.63	2.37	2.10	45	44	11:25:20	RWP	AC	Excel.	Nore	1524	
D	21726	4	12826	4.77	4.89	4.27	4.09	3.91	3.53	3.17	2.81	45	45	11:25:27	RWP	AC	Excel.	Nore	1530	
D	21726	3	9576	2.91	2.62	2.72	2.60	2.49	2.28	2.11	1.91	45	45	11:27:08	RWP	AC	Excel.	Nore	1881	
D	21726	2	6287	3.89	3.46	3.63	3.45	3.33	3.05	2.81	2.55	45	45	11:27:15	RWP	AC	Excel.	Nore	1872	
D	22141	2	6415	1.64	1.56	1.50	1.45	1.41	1.29	1.17	1.09	45	46	11:28:33	RWP	AC	Excel.	Nore	1883	
D	22141	3	9610	2.47	2.34	2.26	2.20	2.14	1.98	1.80	1.64	45	46	11:28:40	RWP	AC	Excel.	Nore	2209	
D	23132	2	6422	2.20	3.10	3.03	2.93	2.85	2.64	2.41	2.19	45	46	11:28:47	RWP	AC	Excel.	Nore	2224	
D	23132	3	9568	3.29	3.11	3.18	3.17	3.08	2.87	2.61	2.34	45	47	11:31:08	RWP	AC	Excel.	Nore	1659	
D	23132	4	12834	4.35	4.11	4.27	4.21	4.10	3.85	3.48	3.11	45	47	11:31:13	RWP	AC	Excel.	Nore	1652	
D	23628	2	6466	1.51	1.34	1.33	1.23	1.19	1.05	0.91	0.83	45	46	11:33:10	RWP	AC	Excel.	Nore	1676	
D	23628	3	9646	2.30	2.04	2.04	1.92	1.82	1.61	1.45	1.26	45	46	11:33:17	RWP	AC	Excel.	Nore	2431	
D	24121	4	12907	3.09	2.72	2.77	2.59	2.45	2.17	1.92	1.70	45	46	11:33:25	RWP	AC	Excel.	Nore	2383	
D	24121	3	9587	2.18	2.01	1.97	1.91	1.84	1.69	1.54	1.40	45	46	11:35:06	RWP	AC	Excel.	Nore	2537	
D	24121	2	6413	1.44	1.34	1.29	1.27	1.23	1.10	0.99	0.93	45	46	11:35:12	RWP	AC	Excel.	Nore	2503	
D	24121	1	9587	2.91	2.69	2.63	2.56	2.48	2.27	2.07	1.89	45	46	11:35:19	RWP	AC	Excel.	Nore	2508	
D	24122	4	12849	2.91	2.69	2.63	2.56	2.48	2.27	2.07	1.89	45	46	11:35:19	RWP	AC	Excel.	Nore	2508	
D	24122	3	9587	1.45	1.38	1.51	1.26	1.23	1.10	0.98	0.90	45	45	11:36:10	RWP	AC	Excel.	Nore	2511	
D	24122	2	6401	2.20	2.08	2.01	1.93	1.85	1.69	1.53	1.36	45	45	11:36:16	RWP	AC	Excel.	Nore	2479	
D	24122	1	12854	2.96	2.77	2.70	2.59	2.48	2.26	2.05	1.84	45	45	11:36:23	RWP	AC	Excel.	Nore	2471	
C	Comment at 24122 Ft Time: 11:36:33 :LOAD TRANSFER																			
D	24628	2	6417	2.40	2.03	2.03	1.87	1.76	1.52	1.30	1.11	45	46	11:38:05	RWP	AC	Excel.	Nore	1519	
D	24628	3	9627	3.65	3.08	3.10	2.88	2.68	2.28	1.98	1.69	45	46	11:38:11	RWP	AC	Excel.	Nore	1500	
D	24628	4	12863	4.89	4.12	4.19	3.85	3.57	3.10	2.67	2.27	45	46	11:38:18	RWP	AC	Excel.	Nore	1495	
D	25133	2	6414	1.56	1.52	1.39	1.34	1.29	1.16	1.04	0.94	46	46	11:39:57	RWP	AC	Excel.	Nore	2337	
D	25133	3	9636	2.39	2.32	2.15	2.08	1.98	1.80	1.63	1.45	46	46	11:40:03	RWP	AC	Excel.	Nore	2289	
D	25133	4	12868	3.19	3.11	2.88	2.77	2.65	2.41	2.18	1.95	46	46	11:40:10	RWP	AC	Excel.	Nore	2296	
D	25682	2	6361	2.17	1.91	1.82	1.68	1.61	1.41	1.24	1.10	48	48	11:41:51	RWP	AC	Excel.	Nore	1664	

		Northwood 65_CABLE_SB																	
D	35108	3	9641	2.50	2.31	2.40	2.39	2.33	2.18	1.95	1.72	47	50	12:22:28	CTR	PCC	Excel.	None	2189
D	35108	4	12919	3.34	3.06	3.23	3.18	3.13	2.91	2.61	2.30	47	50	12:22:35	CTR	PCC	Excel.	None	2200
D	35609	2	6400	1.92	1.73	1.86	1.82	1.81	1.61	1.41	1.21	47	50	12:24:15	CTR	PCC	Excel.	None	1894
D	35609	3	9607	2.88	2.58	2.77	2.78	2.69	2.44	2.13	1.81	47	50	12:24:21	CTR	PCC	Excel.	None	1897
C	Comment	at 35609 Ft. Time: 12:24:28 : Deflection is not decreasing																	
D	35609	4	12866	3.81	3.40	3.71	3.68	3.61	3.30	2.86	2.42	47	50	12:24:30	CTR	PCC	Excel.	None	1919
D	36121	2	6421	1.51	1.40	1.34	1.28	1.25	1.11	1.00	0.90	47	50	12:26:07	CTR	PCC	Excel.	None	2415
D	36121	3	9602	2.29	2.12	2.04	1.99	1.89	1.70	1.54	1.36	47	50	12:26:12	CTR	PCC	Excel.	None	2382
D	36121	4	12908	3.04	2.80	2.72	2.63	2.51	2.27	1.95	1.82	47	50	12:26:19	CTR	PCC	Excel.	None	2411
D	36630	2	6404	2.99	2.84	2.71	2.54	2.42	2.05	1.75	1.53	47	51	12:27:58	CTR	PCC	Excel.	None	1217
D	36630	3	9597	4.53	4.31	4.10	3.94	3.67	3.14	2.68	2.31	47	51	12:28:04	CTR	PCC	Excel.	None	1204
D	36630	4	12837	6.09	5.77	5.53	5.26	4.92	4.21	3.63	3.13	47	51	12:28:11	CTR	PCC	Excel.	None	1208
D	37253	2	6434	2.22	2.10	2.11	2.03	2.05	1.93	1.79	1.61	47	50	12:28:54	CTR	PCC	Excel.	None	1648
D	37253	3	9602	3.33	3.15	3.15	3.10	3.07	2.94	2.76	2.42	47	50	12:30:00	CTR	PCC	Excel.	None	1641
D	37253	4	12832	4.38	4.15	4.18	4.14	4.07	3.93	3.69	3.24	47	50	12:30:07	CTR	PCC	Excel.	None	1666
D	37625	2	6424	2.28	1.99	2.22	2.11	1.94	1.54	1.28	1.13	47	50	12:31:26	CTR	PCC	Excel.	None	1599
C	Comment	at 37625 Ft. Time: 12:31:31 : Deflection is not decreasing																	
D	37625	3	9619	3.46	2.99	3.36	3.22	2.94	2.34	1.98	1.72	47	50	12:31:33	CTR	PCC	Excel.	None	1583
C	Comment	at 37625 Ft. Time: 12:31:39 : Deflection is not decreasing																	
D	37625	4	12853	4.62	3.99	4.52	4.32	3.94	2.68	2.30	2.30	47	50	12:31:40	CTR	PCC	Excel.	None	1581
D	38097	2	6410	2.53	2.46	2.11	1.92	1.87	1.58	1.36	1.20	47	50	12:33:57	CTR	PCC	Excel.	None	1438
D	38097	3	9598	3.88	3.76	3.24	3.05	2.85	2.45	2.15	1.83	47	50	12:34:02	CTR	PCC	Excel.	None	1405
D	38097	4	12830	5.23	5.05	4.59	4.10	3.82	3.51	2.88	2.45	47	50	12:34:09	CTR	PCC	Excel.	None	1396
D	38646	2	6401	1.83	1.72	1.70	1.61	1.60	1.46	1.35	1.25	47	50	12:36:05	CTR	PCC	Excel.	None	1988
D	38646	3	9592	2.74	2.58	2.52	2.47	2.39	2.22	2.06	1.88	47	50	12:36:10	CTR	PCC	Excel.	None	1988
D	38646	4	12891	3.60	3.40	3.33	3.26	3.16	2.94	2.74	2.50	47	50	12:36:17	CTR	PCC	Excel.	None	2039
D	38696	2	6338	2.96	2.82	2.51	2.29	2.15	1.88	1.63	1.43	47	49	12:39:59	CTR	PCC	Excel.	None	1218
D	38696	3	9540	4.51	4.32	3.80	3.57	3.30	2.89	2.54	2.30	47	49	12:40:05	CTR	PCC	Excel.	None	1203
D	38696	4	12760	6.04	5.76	5.13	4.77	4.46	3.90	3.42	2.93	47	49	12:40:12	CTR	PCC	Excel.	None	1201
D	39134	2	6399	2.18	1.98	2.06	1.99	1.93	1.73	1.53	1.37	47	50	12:41:44	CTR	PCC	Excel.	None	1669
D	39134	3	9633	3.31	3.02	3.15	3.10	2.95	2.66	2.39	2.09	47	50	12:41:49	CTR	PCC	Excel.	None	1655
D	39134	4	12899	4.44	4.02	4.28	4.16	4.00	3.60	3.23	2.81	47	50	12:41:56	CTR	PCC	Excel.	None	1652
D	39633	2	6396	2.08	2.03	1.88	1.78	1.73	1.58	1.42	1.31	47	50	12:43:42	CTR	PCC	Excel.	None	1746
D	39633	3	9599	3.14	3.06	2.81	2.74	2.62	2.39	2.19	1.97	47	50	12:43:47	CTR	PCC	Excel.	None	1740
D	39633	4	12863	4.16	4.04	3.77	3.63	3.48	3.19	2.92	2.62	47	50	12:43:54	CTR	PCC	Excel.	None	1757
D	40108	2	6396	3.53	3.36	3.05	2.78	2.58	2.15	1.84	1.62	47	49	12:45:38	CTR	PCC	Excel.	None	1029
D	40108	3	9571	5.35	5.07	4.61	4.25	3.90	3.27	2.83	2.45	47	49	12:45:43	CTR	PCC	Excel.	None	1017
D	40108	4	12797	7.13	6.75	6.17	5.68	5.23	4.41	3.81	3.29	47	49	12:45:51	CTR	PCC	Excel.	None	1020
D	40624	2	6395	2.30	2.15	2.10	2.03	2.02	1.84	1.67	1.53	47	50	12:47:37	CTR	PCC	Excel.	None	1583
D	40624	3	9633	3.47	3.25	3.19	3.14	3.04	2.80	2.56	2.34	47	50	12:47:43	CTR	PCC	Excel.	None	1577
D	40624	4	12878	4.60	4.31	4.26	4.15	4.03	3.75	3.45	3.13	47	50	12:47:49	CTR	PCC	Excel.	None	1593
D	41130	2	6396	2.30	2.13	2.14	2.05	1.99	1.83	1.65	1.51	47	49	12:48:39	CTR	PCC	Excel.	None	1589
D	41130	3	9591	3.43	3.18	3.20	3.13	3.00	2.76	2.53	2.27	47	49	12:48:45	CTR	PCC	Excel.	None	1589
D	41130	4	12825	4.56	4.19	4.28	4.14	4.00	3.69	3.39	3.05	47	49	12:49:52	CTR	PCC	Excel.	None	1601
D	41601	2	6396	2.13	2.13	1.96	1.88	1.82	1.67	1.53	1.42	47	50	12:51:29	CTR	PCC	Excel.	None	1708
D	41601	3	9566	3.19	3.17	2.93	2.87	2.75	2.53	2.35	2.13	47	50	12:51:34	CTR	PCC	Excel.	None	1703
D	41601	4	12835	4.27	4.20	3.92	3.79	3.67	3.41	3.15	2.85	47	50	12:51:42	CTR	PCC	Excel.	None	1709
D	41603	2	6369	2.39	2.17	2.20	2.05	1.98	1.79	1.61	1.48	48	49	12:52:38	CTR	PCC	Excel.	None	1518
D	41603	3	9537	3.62	3.26	3.29	3.18	3.00	2.72	2.49	2.24	48	49	12:52:43	CTR	PCC	Excel.	None	1500
D	41603	4	12803	4.78	4.33	4.41	4.19	3.99	3.64	3.32	2.99	48	49	12:52:51	CTR	PCC	Excel.	None	1522
C	Comment	at 41603 Ft. Time: 12:53:01 : LOAD TRANSFER																	
D	42105	2	6396	2.77	2.66	2.43	2.28	2.21	1.98	1.74	1.56	47	49	12:54:32	CTR	PCC	Excel.	None	1312
D	42105	3	9589	4.18	4.01	3.68	3.54	3.37	3.07	2.69	2.35	47	49	12:54:38	CTR	PCC	Excel.	None	1303
D	43105	4	12848	5.58	5.36	4.95	4.71	4.49	4.02	3.60	3.14	47	49	12:54:45	CTR	PCC	Excel.	None	1308
D	43623	2	6379	2.24	2.09	2.03	1.94	1.89	1.73	1.55	1.42	48	50	12:56:40	CTR	PCC	Excel.	None	1621
D	43623	3	9566	3.37	3.15	3.07	2.99	2.87	2.62	2.38	2.16	48	50	12:56:52	CTR	PCC	Excel.	None	1612
D	43623	4	12812	4.48	4.17	4.11	3.98	3.83	3.49	3.20	2.89	48	50	12:56:58	CTR	PCC	Excel.	None	1627
D	44593	2	6438	2.25	2.06	2.14	2.11	2.03	1.83	1.64	1.47	48	50	13:01:26	CTR	PCC	Excel.	None	1625
D	44593	3	9609	3.38	3.10	3.26	3.23	3.08	2.81	2.52	2.21	48	50	13:01:26	CTR	PCC	Excel.	None	1619
D	44593	4	12803	4.24	4.06	4.14	4.10	4.03	3.82	3.63	3.46	48	50	13:02:10	CTR	PCC	Excel.	None	1616
D	44593	2	6367	2.24	2.06	2.14	2.10	2.03	1.82	1.63	1.46	48	50	13:02:16	CTR	PCC	Excel.	None	1617
D	44593	3	9583	3.37	3.10	3.25	3.22	3.08	2.81	2.52	2.21	48	50	13:02:23	CTR	PCC	Excel.	None	1617
D	44593	4	12865	4.49	4.13	4.39	4.33	4.15	3.76	3.36	2.96	48	50	13:03:58	CTR	PCC	Excel.	None	1629
D	45124	2	6379	1.84	1.64	1.62	1.51	1.43	1.28	1.12	1.02	47	50	13:04:03	CTR	PCC	Excel.	None	1970
D	45124	3	9582	2.78	2.49	2.44	2.29	2.17	1.92	1.73	1.53	47	50	13:04:10	CTR	PCC	Excel.	None	1958
D	45124	4	12864	3.72	3.29	3.25	3.05	2.92	2.59	2.30	2.04	47	50	13:04:10	CTR	PCC	Excel.	None	1968
D	45896	2	6422	1.75	1.67	1.64	1.57	1.55	1.42	1.32	1.23	48	53	13:06:06	CTR	PCC	Excel.	None	2082

Northwood 65: CABLE SB															
D	45896	3	9622	2.59	2.48	2.43	2.37	2.28	2.15	1.99	1.83	1.83	1.83	1.83	2108
D	45896	4	12877	3.42	3.25	3.23	3.14	3.05	2.87	2.65	2.42	2.42	2.42	2.42	2139
D	46618	3	9508	3.11	2.66	2.66	2.56	2.40	1.91	1.56	1.36	1.36	1.36	1.36	1156
D	46618	4	12758	4.74	4.07	4.30	3.97	3.67	2.83	2.40	2.06	2.06	2.06	2.06	1140
D	46619	4	12758	6.36	5.44	5.80	5.33	4.94	3.75	3.24	2.76	2.76	2.76	2.76	1141
D	46619	3	9539	3.81	2.18	2.41	2.26	2.16	1.87	1.58	1.30	1.30	1.30	1.30	1448
C	Comment at	46619	ft	Time: 13:10:03	Deflection is not decreasing										1425
C	46619	4	12785	5.11	4.46	4.91	4.72	4.46	3.84	3.28	2.61	2.61	2.61	2.61	1422
C	Comment at	46619	ft	Time: 13:10:18	LOAD TRANSFER										1786
D	47112	3	9396	2.03	1.90	1.87	1.80	1.80	1.68	1.56	1.47	1.47	1.47	1.47	1808
D	47112	4	12906	3.98	2.82	2.80	2.76	2.68	2.51	2.37	2.20	2.20	2.20	2.20	1843
D	47651	3	9518	4.40	3.89	4.11	3.86	3.68	3.34	3.16	2.92	2.92	2.92	2.92	821
D	47651	4	12775	6.69	5.91	6.27	5.98	5.62	4.82	4.09	3.40	3.40	3.40	3.40	810
D	48122	4	12775	8.93	7.88	8.40	7.99	7.51	6.48	5.48	4.55	4.55	4.55	4.55	814
D	48122	3	9622	3.01	2.82	2.90	2.79	2.69	2.38	2.06	1.87	1.87	1.87	1.87	1216
D	48122	4	12861	4.55	4.24	4.37	4.29	4.11	3.62	3.17	2.71	2.71	2.71	2.71	1209
D	48607	3	9399	2.71	2.61	2.61	2.58	2.58	2.46	2.33	2.21	2.21	2.21	2.21	1345
D	48607	4	12845	4.16	3.98	4.09	3.98	3.79	3.52	3.33	3.16	3.16	3.16	3.16	1312
D	49194	3	9601	3.50	2.09	2.07	1.96	1.91	1.67	1.46	1.28	1.28	1.28	1.28	1567
D	49194	4	12844	4.67	3.15	3.14	3.02	2.84	2.52	2.22	1.91	1.91	1.91	1.91	1560
D	49639	2	6395	1.69	1.61	1.58	1.54	1.51	1.38	1.26	1.17	1.17	1.17	1.17	1565
D	49639	3	9604	2.58	2.45	2.40	2.36	2.29	2.11	1.95	1.78	1.78	1.78	1.78	2121
D	49639	4	12885	3.44	3.25	3.22	3.14	3.05	2.84	2.62	2.40	2.40	2.40	2.40	2129
D	50181	2	6336	1.47	1.37	1.31	1.23	1.27	1.13	1.03	0.94	0.94	0.94	0.94	2453
D	50181	4	12857	3.00	2.79	2.75	2.63	2.55	2.33	2.14	1.93	1.93	1.93	1.93	2440
D	50620	2	6398	1.81	1.66	1.76	1.74	1.78	1.65	1.44	1.25	1.25	1.25	1.25	2009
C	Comment at	50620	ft	Time: 13:25:27	Deflection is not decreasing										1978
C	50620	3	9612	2.76	2.52	2.72	2.74	2.74	2.54	2.22	1.90	1.90	1.90	1.90	1978
C	Comment at	50620	ft	Time: 13:25:38	Deflection is not decreasing										1930
D	51129	2	6406	3.79	3.44	3.75	3.77	3.75	3.52	3.06	2.59	2.59	2.59	2.59	1803
D	51129	3	9618	3.08	2.72	2.85	2.73	2.55	2.18	1.90	1.65	1.65	1.65	1.65	1775
D	51129	4	12910	4.11	3.61	3.82	3.63	3.41	2.95	2.55	2.23	2.23	2.23	2.23	1787
D	51602	2	6418	1.25	1.15	1.13	1.06	1.06	0.95	0.88	0.83	0.83	0.83	0.83	2909
D	51602	3	9618	1.86	1.73	1.68	1.64	1.57	1.47	1.35	1.24	1.24	1.24	1.24	2936
D	51602	4	12920	2.47	2.29	2.24	2.15	2.08	1.95	1.79	1.65	1.65	1.65	1.65	2979
D	51605	3	9593	2.21	1.35	1.27	1.17	1.15	1.02	0.92	0.84	0.84	0.84	0.84	2507
D	51605	4	12879	2.95	2.72	2.61	2.56	2.41	2.14	1.91	1.67	1.67	1.67	1.67	2471
C	Comment at	51605	ft	Time: 13:32:17	LOAD TRANSFER										2484
D	52118	2	6428	1.46	1.37	1.34	1.28	1.25	1.14	1.03	0.93	0.93	0.93	0.93	2503
D	52118	3	9635	2.19	2.06	1.99	1.95	1.89	1.73	1.55	1.38	1.38	1.38	1.38	2496
D	52604	2	6416	1.65	1.59	1.51	1.44	1.42	1.29	1.20	1.14	1.14	1.14	1.14	2540
D	52604	3	9634	2.48	2.38	2.27	2.22	2.12	1.95	1.81	1.67	1.67	1.67	1.67	2213
D	52604	4	12873	3.24	3.12	2.99	2.89	2.80	2.60	2.40	2.22	2.22	2.22	2.22	2256
D	53108	3	9637	1.51	1.45	1.39	1.34	1.33	1.24	1.13	1.02	1.02	1.02	1.02	2417
D	53108	4	12919	2.24	2.16	2.07	2.06	1.98	1.86	1.69	1.53	1.53	1.53	1.53	2439
D	53626	3	9626	2.05	2.82	2.75	2.68	2.62	2.47	2.24	2.02	2.02	2.02	2.02	2479
D	53626	4	12938	3.14	3.00	2.82	2.78	2.67	2.50	2.31	2.19	2.19	2.19	2.19	1784
C	Comment at	53626	ft	Time: 13:40:04	END										1736
D	53626	4	12938	4.24	4.14	3.74	3.60	3.44	3.11	2.79	2.46	2.46	2.46	2.46	1736

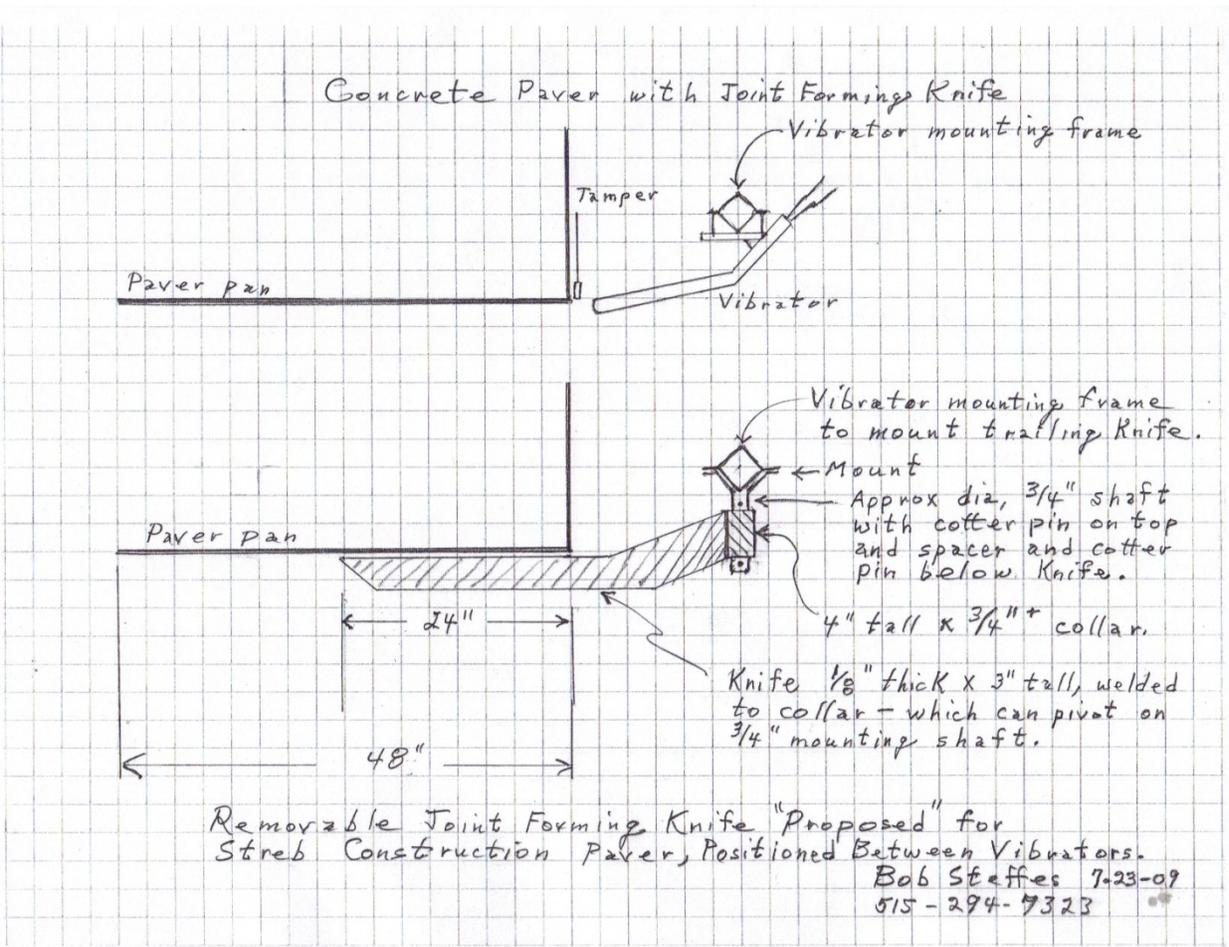
APPENDIX F. KNIFE PHOTOS AND PRELIMINARY DESIGN



The removable joint-forming knife used for creating longitudinal joints



The removable joint-forming knife



A schematic for the concrete paver with the joint-forming knife



Pavement where a crack had formed along the longitudinal joint



Pavement where a crack has formed along the longitudinal joint, but remains tight



Pavement where a crack has formed along the longitudinal joint and is barely visible